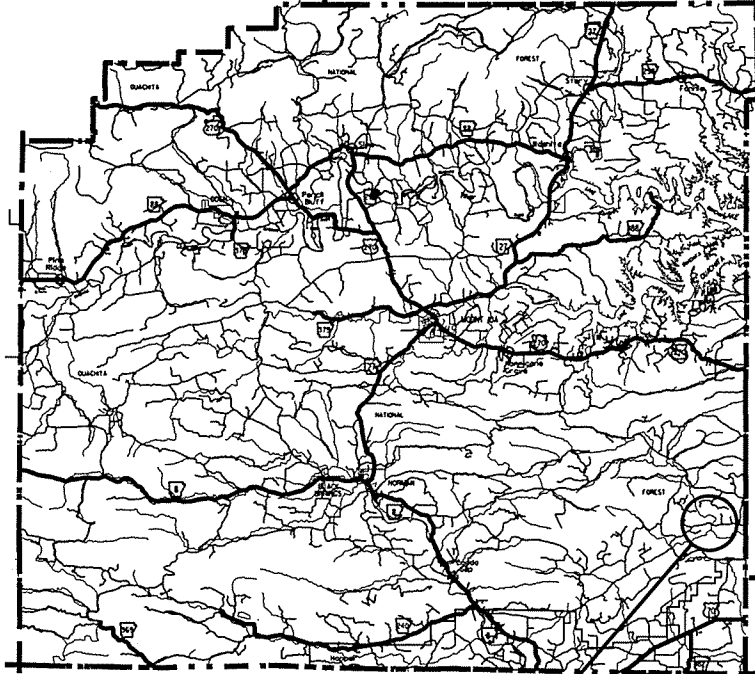


MONTGOMERY



PROJECT LOCATION

VICINITY MAP



ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT
CONSTRUCTION PLANS FOR PROPOSED COUNTY ROAD



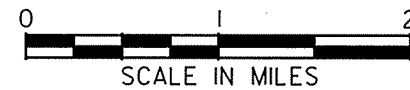
MAZARN CREEK STR. & APPRS. (S)

CO. RD. 34

MONTGOMERY COUNTY

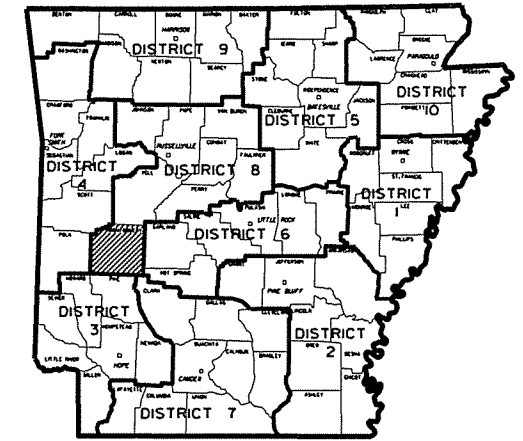
FED. AID PROJ. STPB-NBIL(16)

JOB BR4906



R23W

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		1	85
				4 MAZARN CREEK STR. & APPRS. (S)				



ARKANSAS HIGHWAY DISTRICT 8

STRUCTURES OVER 20'-0" SPAN

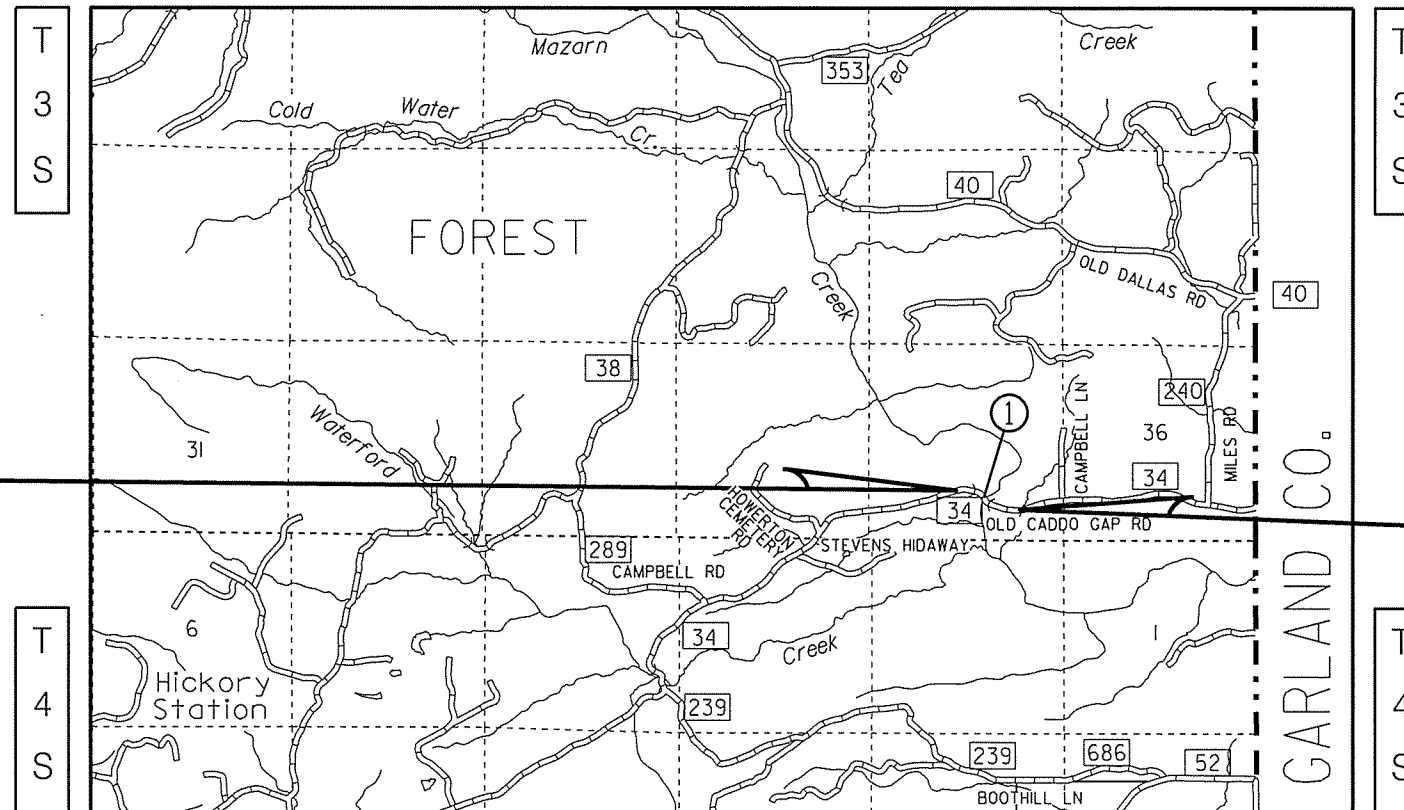
- ① STA. 108+61.92 BRIDGE END
BRIDGE NO. 04930
CONT. COMP. W-BEAM UNIT
24' - 0" CLEAR ROADWAY
BRIDGE LENGTH = 194' - 2"
STA. 110+56.08 BRIDGE END

STA. 97+00.00 - BEGIN JOB BR4906

FED. AID PROJ. STPB-NBIL(16)

STA. 119+00.00 - END JOB BR4906

FED. AID PROJ. STPB-NBIL(16)



R23W

GARLAND CO.

DESIGN TRAFFIC DATA

DESIGN YEAR	2035
2015 ADT	60
2035 ADT	80
2035 DHV	12
DIRECTIONAL DISTRIBUTION	0.60
TRUCKS	3%
DESIGN SPEED	40 MPH

PROJECT COORDINATES:

	BEGIN	MID-POINT	END
LAT.	N 34° 25' 51.1"	N 34° 25' 48.1"	N 34° 25' 45.4"
LONG.	W 93° 26' 1.7"	W 93° 25' 52.4"	W 93° 25' 41.2"

GROSS LENGTH OF PROJECT	2200.00 FEET OR 0.417 MILES
NET " " ROADWAY	2005.84 " " 0.380 "
NET " " BRIDGE	194.16 " " 0.037 "
NET " " PROJECT	2200.00 " " 0.417 "

APPROVED



6-4-15

DEPUTY DIRECTOR
AND CHIEF ENGINEER

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
10/17/15				6	ARK.			
						JOB NO.	BR4906	2
						85		

4 INDEX OF SHEETS, GOV. SPECS, AND GEN. NOTES

SHEET NO.	INDEX OF SHEETS TITLE	BRIDGE NO.	DRWG. NO.	DATE
1.	TITLE SHEET			
2.	INDEX OF SHEETS, GOVERNING SPECIFICATIONS, AND GENERAL NOTES			
3.	TYPICAL SECTIONS OF IMPROVEMENT			
4.	GUARDRAIL WIDENING DETAILS			
5.	DRIVEWAY DETAILS			
6 - 7.	TEMPORARY EROSION CONTROL DETAILS			
8 - 11.	QUANTITIES			
12.	SCHEDULE OF BRIDGE QUANTITIES	04930	56451	
13.	SUMMARY OF QUANTITIES AND REVISIONS			
14 - 15.	SURVEY CONTROL DETAILS			
16 - 22.	PLAN AND PROFILE SHEETS			
23.	LAYOUT OF BRIDGE OVER MAZARN CREEK (SHEET 1 OF 2)	04930	56452	
24.	LAYOUT OF BRIDGE OVER MAZARN CREEK (SHEET 2 OF 2)	04930	56453	
25.	DETAILS OF END BENTS	04930	56454	
26.	DETAILS OF INTERMEDIATE BENTS	04930	56455	
27.	DETAILS OF 192'-0" CONTINUOUS COMPOSITE W-BEAM UNIT (SHEET 1 OF 5)	04930	56456	
28.	DETAILS OF 192'-0" CONTINUOUS COMPOSITE W-BEAM UNIT (SHEET 2 OF 5)	04930	56457	
29.	DETAILS OF 192'-0" CONTINUOUS COMPOSITE W-BEAM UNIT (SHEET 3 OF 5)	04930	56458	
30.	DETAILS OF 192'-0" CONTINUOUS COMPOSITE W-BEAM UNIT (SHEET 4 OF 5)	04930	56459	
31.	DETAILS OF 192'-0" CONTINUOUS COMPOSITE W-BEAM UNIT (SHEET 5 OF 5)	04930	56460	
32.	DETAILS OF JOINTS	04930	56461	
33.	DETAILS OF ELASTOMERIC BEARINGS	04930	56462	
34.	STANDARD DETAILS FOR EMBANKMENT CONSTRUCTION AND BACKFILL AT BRIDGE ENDS		55000	02-27-14
35.	STANDARD DETAILS FOR DUMPED RIPRAP AND FILTER BLANKET AND COMPUTING EXCAVATION FOR STRUCTURES		55001	02-27-14
36.	STANDARD DETAILS FOR PERMANENT STEEL BRIDGE DECK FORMS FOR STEEL & CONCRETE GIRDER SPANS		55005	02-27-14
37.	STANDARD DETAILS FOR TYPE C BRIDGE NAME PLATES		55011	02-27-14
38.	STANDARD DETAILS FOR STEEL H-PILES AND PILE ENCASEMENTS		55020	02-27-14
39.	STANDARD DETAILS FOR TYPE A APPROACH GUTTERS		55030A	02-27-14
40.	GUARD RAIL DETAILS		GR-8	07-14-10
41.	GUARD RAIL DETAILS		GR-9	04-17-08
42.	GUARD RAIL DETAILS		GR-10	07-14-10
43.	GUARD RAIL DETAILS		GR-10A	07-14-10
44.	GUARD RAIL DETAILS		GRT-1	07-14-10
45.	MAILBOX DETAILS		MB-1	11-18-04
46.	CONCRETE PIPE CULVERT FILL HEIGHTS & BEDDING		PCC-1	02-27-14
47.	METAL PIPE CULVERT FILL HEIGHTS & BEDDING		PCM-1	02-27-14
48.	PLASTIC PIPE CULVERT (HIGH DENSITY POLYETHYLENE)		PCP-1	02-27-14
49.	PLASTIC PIPE CULVERT (PVC F949)		PCP-2	02-27-14
50.	PAVEMENT MARKING DETAILS		PM-1	09-12-13
51.	DETAILS OF PIPE UNDERDRAIN		PU-1	04-10-03
52.	TABLES AND METHOD OF SUPERELEVATION FOR TWO-WAY TRAFFIC		SE-2	10-18-96
53.	STANDARD HIGHWAY SIGNS AND SUPPORT ASSEMBLIES		SHS-1	09-12-13
54.	U-CHANNEL POST ASSEMBLIES		SHS-2	02-27-14
55.	STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION		TC-1	09-02-15
56.	STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION		TC-2	09-02-15
57.	STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION		TC-3	09-02-15
58.	TEMPORARY EROSION CONTROL DEVICES		TEC-1	12-15-11
59.	TEMPORARY EROSION CONTROL DEVICES		TEC-2	06-02-94
60.	TEMPORARY EROSION CONTROL DEVICES		TEC-3	11-03-94
61.	WIRE FENCE TYPE C AND D		WF-4	08-22-02
62 - 85.	CROSS SECTIONS			

GENERAL NOTES

- GRADE LINE DENOTES FINISHED GRADE WHERE SHOWN ON PLANS.
- UTILITIES INTERFERING WITH CONSTRUCTION SHALL BE MOVED BY THE OWNERS.
- ALL LAND MONUMENTS LOCATED WITHIN THE CONSTRUCTION AREA SHALL BE PROTECTED IN ACCORDANCE WITH SECTION 107.12 OF THE STANDARD SPECIFICATIONS.
- ALL TREES THAT DO NOT DIRECTLY INTERFERE WITH THE PROPOSED CONSTRUCTION SHALL BE SPARED AS DIRECTED BY THE ENGINEER. CARE AND DISCRETION SHALL BE USED TO INSURE THAT ALL TREES NOT TO BE REMOVED SHALL BE HARMED AS LITTLE AS POSSIBLE DURING THE CONSTRUCTION OPERATIONS.
- SUPERELEVATION SHALL BE COMPUTED IN ACCORDANCE WITH STD. DRWG. SE-2 USING 40 M.P.H. DESIGN VALUES AND REVOLVE ABOUT THE INNER EDGE OF TRAVEL LANE UNLESS OTHERWISE SHOWN.
- ALL SALVAGEABLE PIPE CULVERTS SHALL BE STORED ON THE RIGHT-OF-WAY AND REMAIN THE PROPERTY OF MONTGOMERY COUNTY.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING A FENCE TO CONTROL LIVESTOCK IN AREAS WHERE PASTURES ARE SEVERED. WIRE FENCE MAY BE CONSTRUCTED INITIALLY, OR IN LIEU THEREOF, THE CONTRACTOR AT HIS OWN EXPENSE, MAY ELECT TO PROVIDE TEMPORARY FENCING SUITABLE TO CONTAIN LIVESTOCK.
- THE ROAD SHALL BE MAINTAINED AND REMAIN OPEN TO TRAFFIC THROUGHOUT THE PROJECT. THE BRIDGE SHALL BE OPENED TO TRAFFIC AS SOON AS PRACTICABLE.
- THE CONTRACTOR WILL BE REQUIRED TO PROTECT THE BRIDGE DECK DURING PRIME AND SEAL OPERATIONS.
- EXISTING BRIDGE NUMBER 20738 SHALL BE REMOVED IN ACCORDANCE WITH SECTION 205 OF THE STANDARD SPECIFICATIONS AFTER CONSTRUCTION OF THE NEW BRIDGE. ALL MATERIAL FROM THE EXISTING BRIDGE SHALL BECOME THE PROPERTY OF THE CONTRACTOR.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING U.S. MAILBOXES WITHIN THE PROJECT LIMITS IN SUCH A MANNER THAT THE PUBLIC MAY RECEIVE CONTINUED MAIL SERVICE. PAYMENT WILL BE CONSIDERED INCLUDED IN THE PRICE BID FOR THE VARIOUS BID ITEMS.

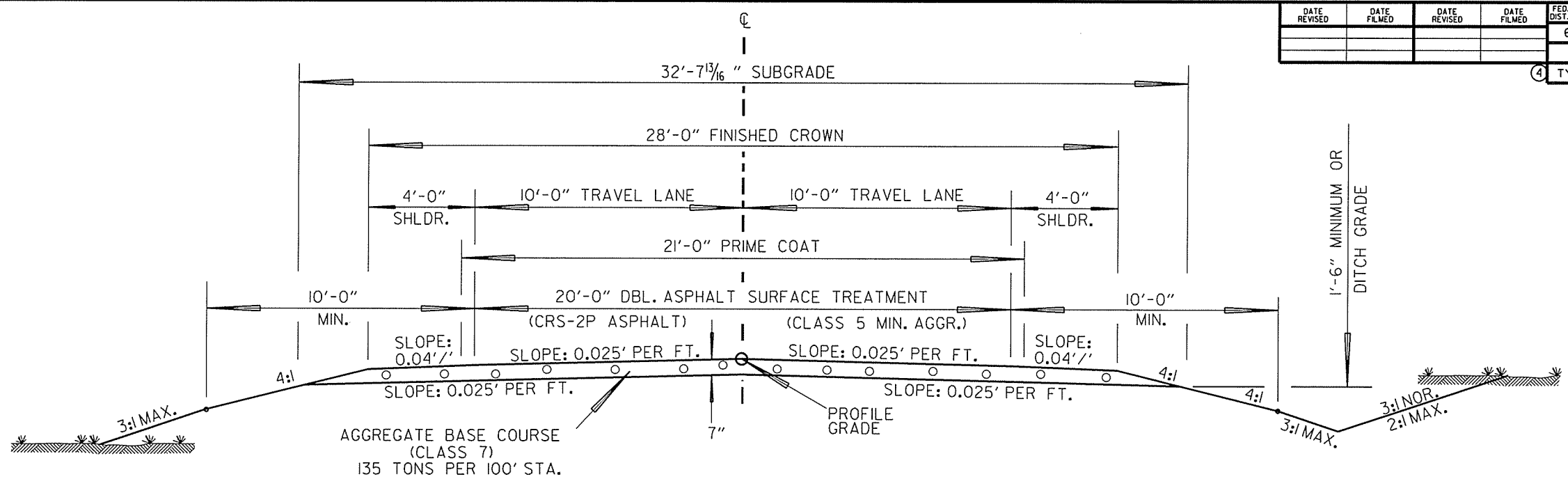
GOVERNING SPECIFICATIONS

ARKANSAS STATE HIGHWAY COMMISSION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, EDITION OF 2014, AND THE FOLLOWING SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS.

NUMBER	TITLE
ERRATA	ERRATA FOR THE BOOK OF STANDARD SPECIFICATIONS
FHWA-1273	REQUIRED CONTRACT PROVISIONS FEDERAL-AID CONSTRUCTION CONTRACTS
FHWA-1273	SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - NOTICE TO CONTRACTORS
FHWA-1273	SUPPLEMENT - SPECIFIC EQUAL EMPLOYMENT OPPORTUNITY RESPONSIBILITIES (23 U.S.C. 140)
FHWA-1273	SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - GOALS AND TIMETABLES
FHWA-1273	SUPPLEMENT - EQUAL EMPLOYMENT OPPORTUNITY - FEDERAL STANDARDS
FHWA-1273	SUPPLEMENT - POSTERS AND NOTICES REQUIRED FOR FEDERAL-AID PROJECTS
FHWA-1273	SUPPLEMENT - REVISIONS OF FHWA-1273 FOR OFF-SYSTEM PROJECTS
108-1	LIQUIDATED DAMAGES
604-1	RETROREFLECTIVE SHEETING FOR TRAFFIC CONTROL DEVICES IN CONSTRUCTION ZONES
606-1	PIPE CULVERTS FOR SIDE DRAINS
620-1	MULCH COVER
JOB BR4906	BIDDING REQUIREMENTS AND CONDITIONS
JOB BR4906	BROADBAND INTERNET SERVICE FOR FIELD OFFICE
JOB BR4906	DISADVANTAGED BUSINESS ENTERPRISE BIDDER'S RESPONSIBILITIES
JOB BR4906	GOALS FOR DISADVANTAGED BUSINESS ENTERPRISE PARTICIPATION
JOB BR4906	MANDATORY ELECTRONIC CONTRACT
JOB BR4906	PLASTIC PIPE
JOB BR4906	SECTION 404 NATIONWIDE 14 PERMIT REQUIREMENTS
JOB BR4906	SPECIAL CLEARING REQUIREMENTS
JOB BR4906	STORM WATER POLLUTION PREVENTION PLAN
JOB BR4906	UTILITY ADJUSTMENTS



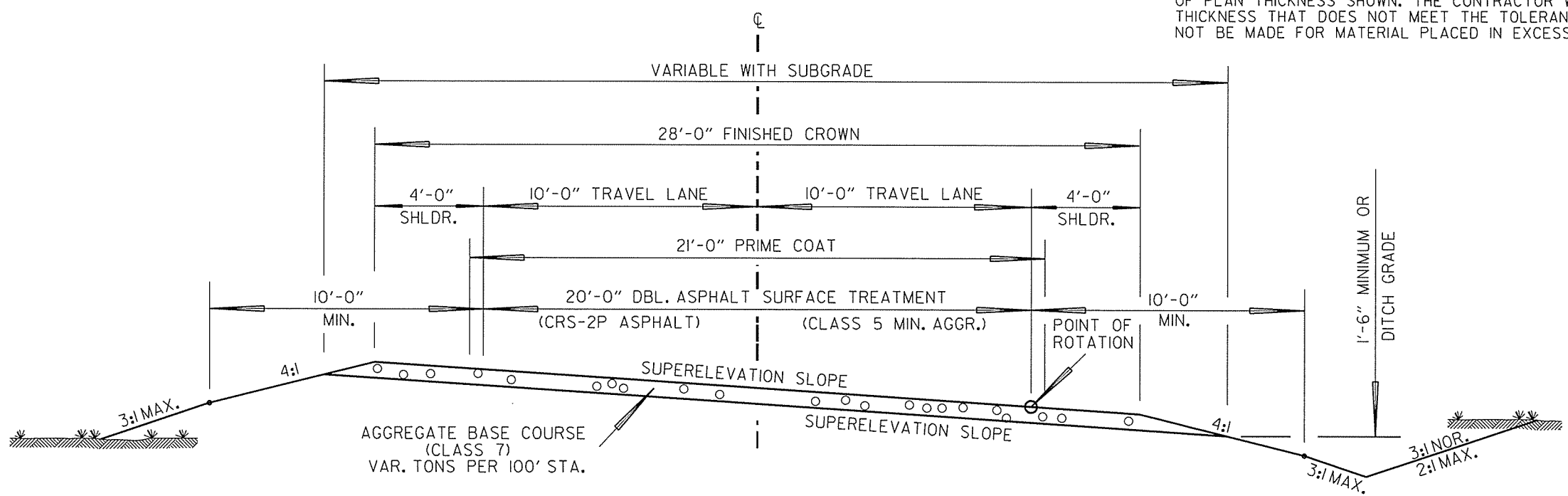
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	3	85	



TYPICAL SECTION OF IMPROVEMENT

NOT TO SCALE
 REFER TO CROSS SECTIONS FOR DEVIATION FROM THE NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER.

THE THICKNESS OF BASE COURSE SHALL BE WITHIN PLUS OR MINUS ONE INCH OF PLAN THICKNESS SHOWN. THE CONTRACTOR WILL CORRECT ANY DEFICIENT THICKNESS THAT DOES NOT MEET THE TOLERANCE INDICATED. PAYMENT WILL NOT BE MADE FOR MATERIAL PLACED IN EXCESS OF THE TOLERANCE INDICATED.



SUPERELEVATED SECTION OF IMPROVEMENT

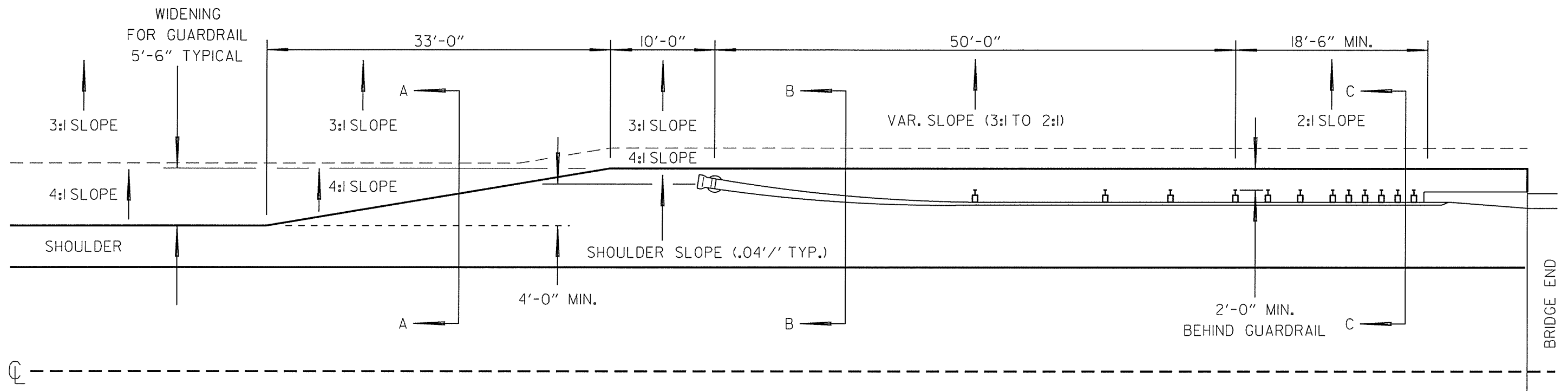
NOT TO SCALE

TYPICAL SECTIONS OF IMPROVEMENT

NOTE: THE ABOVE DETAILS MAY BE MODIFIED TO MEET LOCAL CONDITIONS AS DIRECTED BY THE ENGINEER.

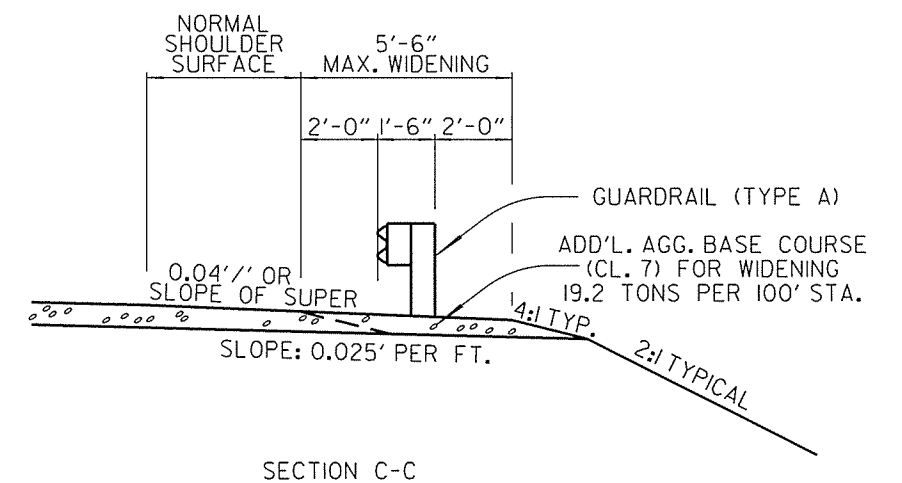
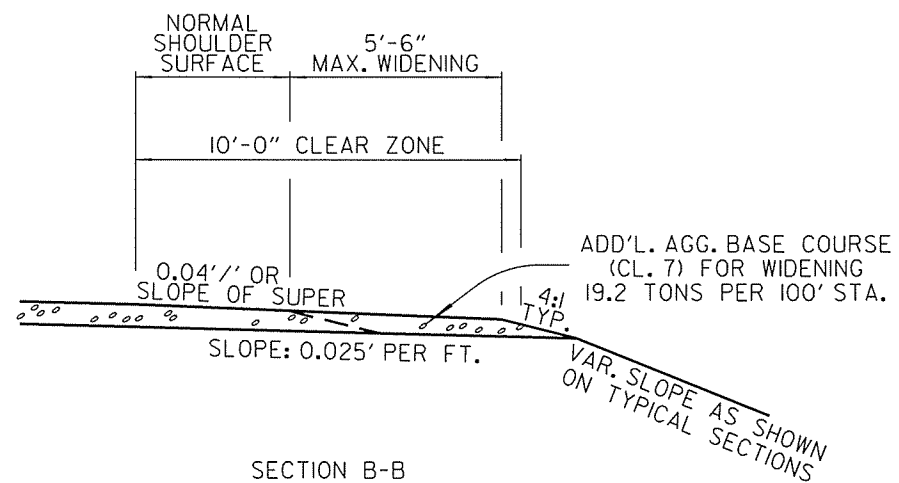
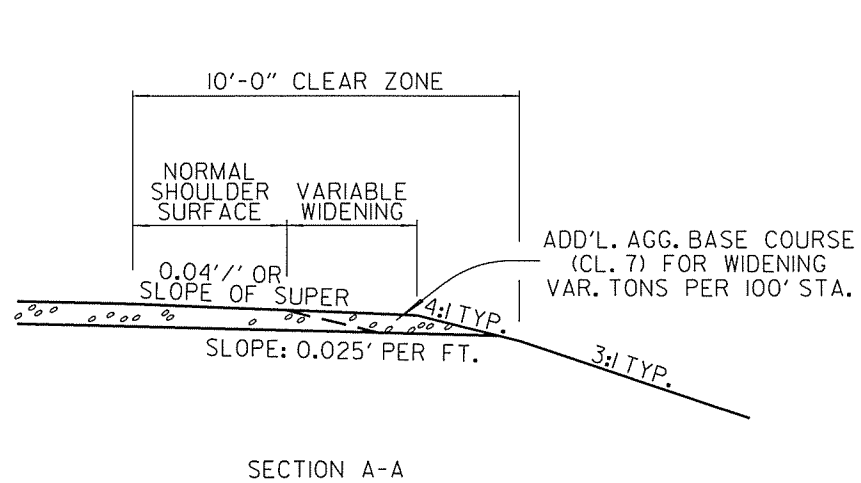


DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		4	85
				④ GUARDRAIL WIDENING DETAILS				



DETAILS OF ROADWAY WIDENING FOR GUARDRAIL

NOT TO SCALE



GUARDRAIL WIDENING SECTIONS

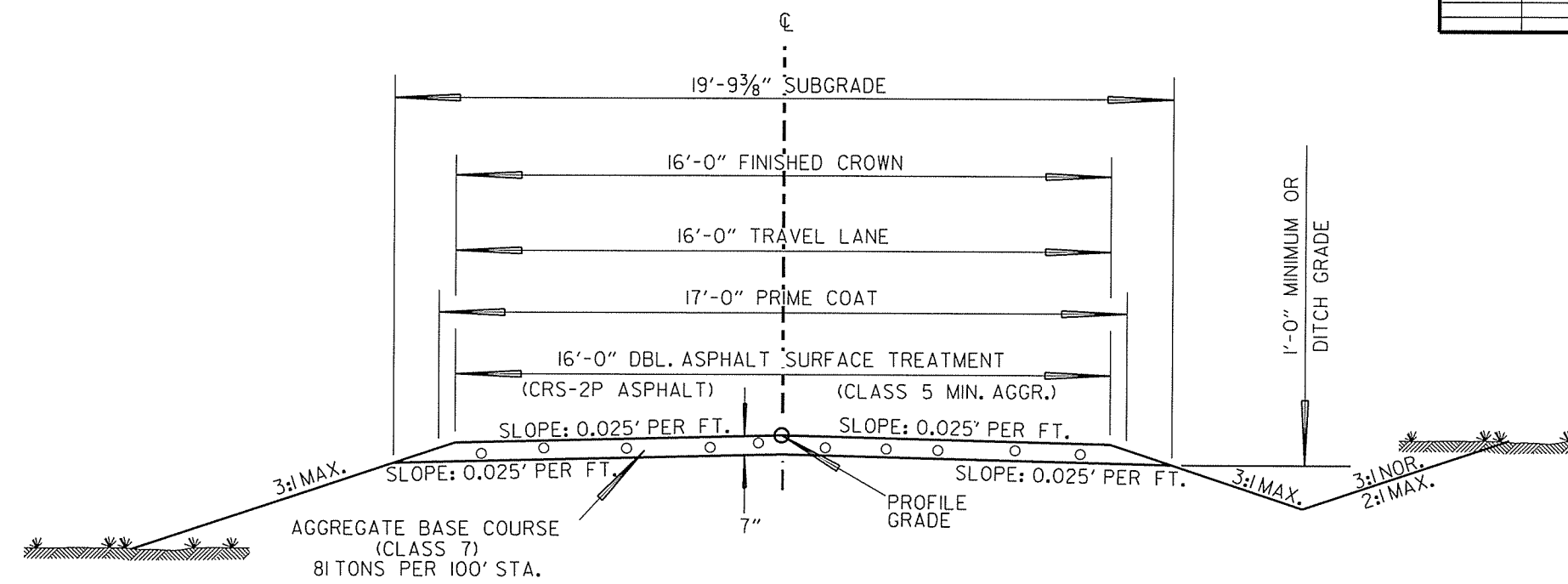
NOT TO SCALE

GUARDRAIL WIDENING DETAILS



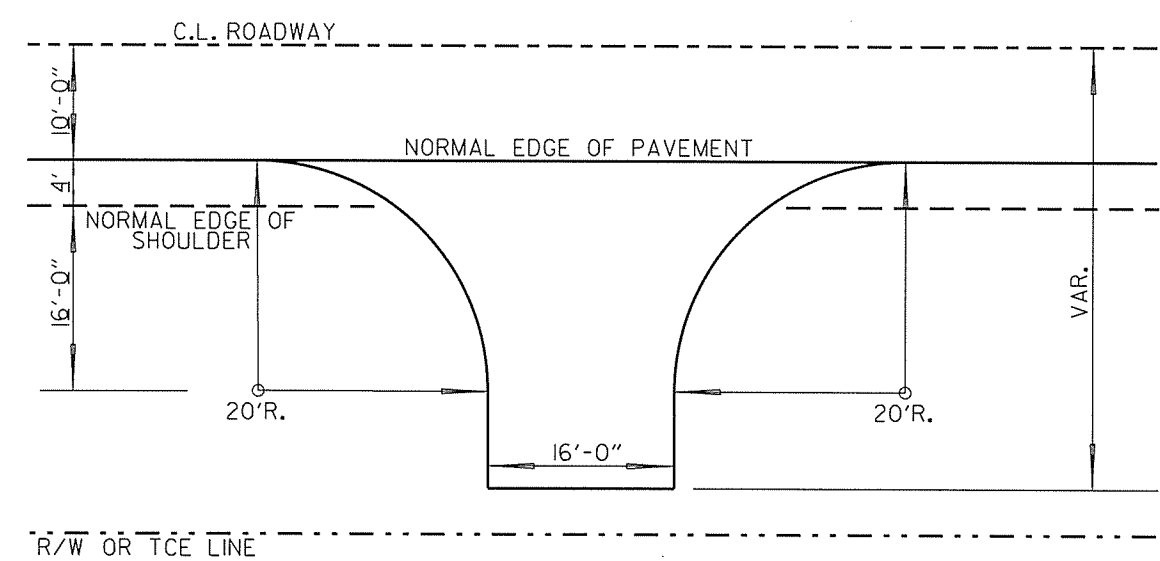
6/3/2015

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	5	85	



TYPICAL SECTION OF PRIVATE ENTRANCE

NOT TO SCALE
 REFER TO CROSS SECTIONS FOR DEVIATION FROM THE NORMAL SLOPES. NO CHANGES SHALL BE MADE FROM THE PLANNED SLOPES WITHOUT THE APPROVAL OF THE ENGINEER.



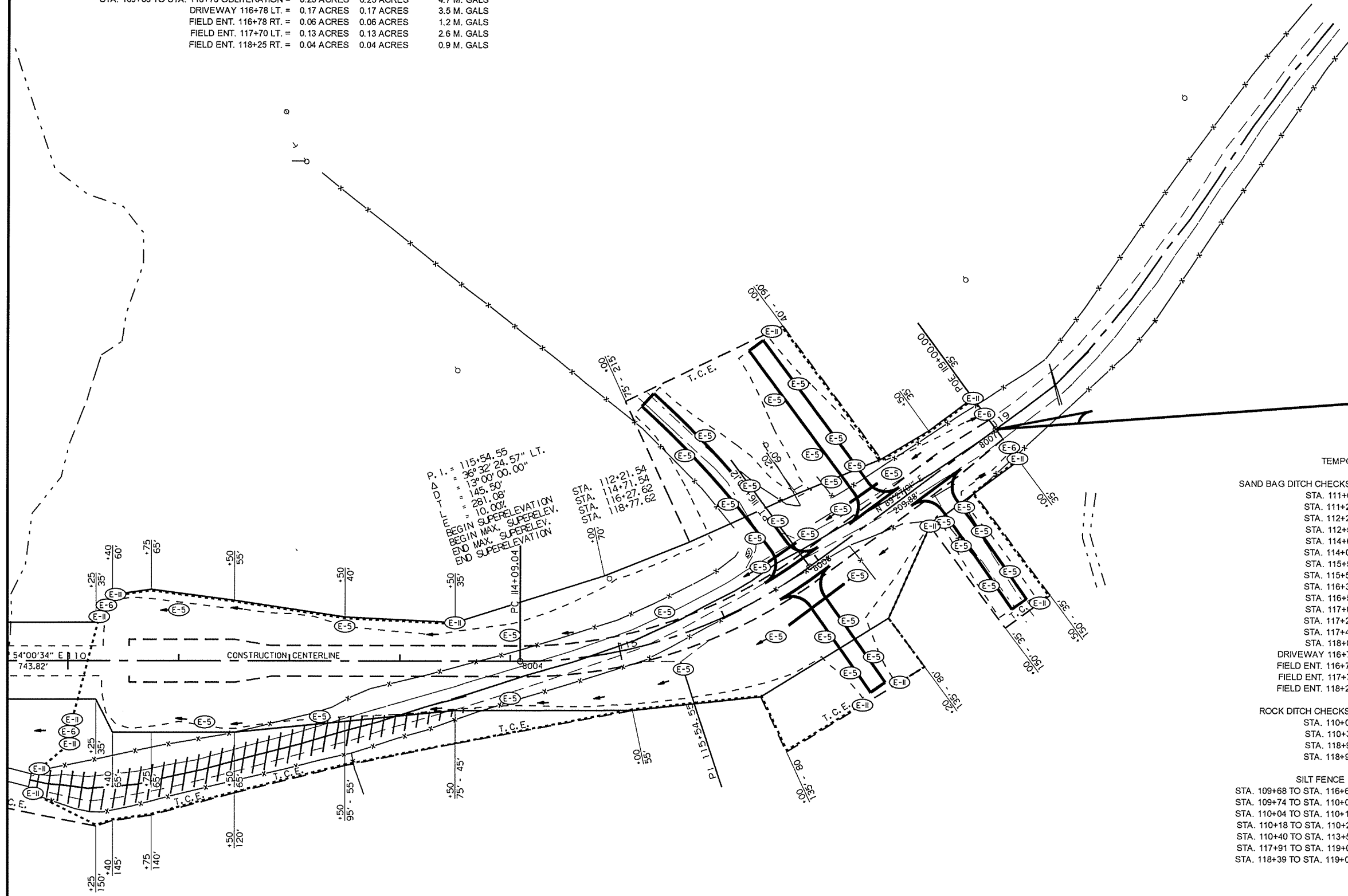
DETAIL OF PRIVATE ENTRANCE

DRIVEWAY DETAILS

STATE OF ARKANSAS
 REGISTERED PROFESSIONAL ENGINEER
 No. 5368
 DAVID B. MANO JR.
 5/29/2015

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		7	85
				JOB NO. BR4906				
(4) TEMPORARY EROSION CONTROL DETAILS								

	TEMPORARY SEEDING	MULCH COVER	WATER
STA. 109+59 TO STA. 119+00 MAIN LANES =	1.31 ACRES	1.31 ACRES	26.7 M. GALS
STA. 109+65 TO STA. 113+73 OBLITERATION =	0.23 ACRES	0.23 ACRES	4.7 M. GALS
DRIVEWAY 116+78 LT. =	0.17 ACRES	0.17 ACRES	3.5 M. GALS
FIELD ENT. 116+78 RT. =	0.06 ACRES	0.06 ACRES	1.2 M. GALS
FIELD ENT. 117+70 LT. =	0.13 ACRES	0.13 ACRES	2.6 M. GALS
FIELD ENT. 118+25 RT. =	0.04 ACRES	0.04 ACRES	0.9 M. GALS

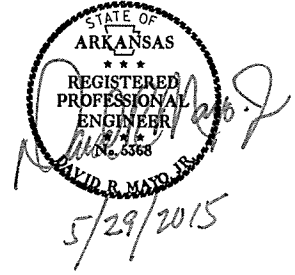


STA. 119+00.00 - END JOB BR4906
FED. AID PROJ. STPB-NBIL(16)

TEMPORARY EROSION CONTROL

SAND BAG DITCH CHECKS (E-5)	SEDIMENT REMOVAL AND DISPOSAL
STA. 111+00 LT. = 20 BAGS	3 CU. YDS
STA. 111+25 RT. = 20 BAGS	3 CU. YDS
STA. 112+25 RT. = 20 BAGS	3 CU. YDS
STA. 112+50 LT. = 20 BAGS	3 CU. YDS
STA. 114+00 LT. = 20 BAGS	3 CU. YDS
STA. 114+00 RT. = 20 BAGS	3 CU. YDS
STA. 115+50 LT. = 20 BAGS	3 CU. YDS
STA. 115+50 RT. = 20 BAGS	3 CU. YDS
STA. 116+33 RT. = 20 BAGS	3 CU. YDS
STA. 116+52 LT. = 20 BAGS	3 CU. YDS
STA. 117+07 LT. = 20 BAGS	3 CU. YDS
STA. 117+20 RT. = 20 BAGS	3 CU. YDS
STA. 117+44 RT. = 20 BAGS	3 CU. YDS
STA. 118+00 LT. = 20 BAGS	3 CU. YDS
DRIVEWAY 116+78 LT. = 120 BAGS	20 CU. YDS
FIELD ENT. 116+78 RT. = 80 BAGS	13 CU. YDS
FIELD ENT. 117+70 LT. = 120 BAGS	20 CU. YDS
FIELD ENT. 118+25 RT. = 120 BAGS	20 CU. YDS
ROCK DITCH CHECKS (E-6)	SEDIMENT REMOVAL AND DISPOSAL
STA. 110+00 RT. = 5 CU. YDS.	5 CU. YDS
STA. 110+35 LT. = 5 CU. YDS.	5 CU. YDS
STA. 118+90 LT. = 5 CU. YDS.	5 CU. YDS
STA. 118+90 RT. = 5 CU. YDS.	5 CU. YDS
SILT FENCE (E-11)	SEDIMENT REMOVAL AND DISPOSAL
STA. 109+68 TO STA. 116+65 RT. = 813 LIN. FT.	24 CU. YDS
STA. 109+74 TO STA. 110+02 RT. = 35 LIN. FT.	1 CU. YDS
STA. 110+04 TO STA. 110+18 RT. = 55 LIN. FT.	2 CU. YDS
STA. 110+18 TO STA. 110+28 LT. = 41 LIN. FT.	1 CU. YDS
STA. 110+40 TO STA. 113+50 LT. = 312 LIN. FT.	9 CU. YDS
STA. 117+91 TO STA. 119+00 LT. = 262 LIN. FT.	8 CU. YDS
STA. 118+39 TO STA. 119+00 RT. = 176 LIN. FT.	5 CU. YDS

TEMPORARY EROSION CONTROL DETAILS



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS	
				6	ARK.				
				JOB NO.	BR4906	8	85		
								4	QUANTITIES

BENCH MARK CAPS

LOCATION / DESCRIPTION	EACH
STA. 108+54 TRANSITION RAIL	1
TOTAL:	1

NOTE: BENCH MARK CAPS SHALL BE FURNISHED AND INSTALLED BY STATE FORCES.

CLEARING AND GRUBBING

STATION	STATION	CLEARING	GRUBBING
		STATION	STATION
97+00	119+00	22	22
TOTALS:		22	22

FENCING

STATION	STATION	SIDE	REMOVAL AND DISPOSAL OF FENCE	WIRE FENCE (TYPE D-1)	18'-0" GATES
			LIN. FT.	LIN. FT.	EACH
97+00	98+26	LT.	138	137	
97+00	98+76	RT.	179	177	
98+76	98+94	RT.			1
98+84	109+03	RT.	600	950	
99+46	106+91	LT.		785	
99+46	108+31	LT.	900		
106+91	107+09	LT.			1
107+09	108+63	LT.		205	
108+48	109+09	RT.	110		
109+67	118+16	RT.	966		
109+72	113+72	RT.	360		
110+55	116+00	LT.		686	
110+55	116+70	RT.		704	
113+72	116+41	LT.	259		
116+00	116+50	LT.	132		
116+00	116+66	LT.		56	
116+66	116+84	LT.			1
116+70	116+86	RT.			1
116+84	117+61	LT.		223	
116+86	118+16	RT.		44	
116+89	117+29	LT.	145		
117+44	119+00	LT.	180		
117+61	117+79	LT.			1
117+79	119+00	LT.		130	
118+16	118+34	RT.			1
118+32	119+00	RT.	115		
118+34	119+00	RT.		72	
TOTALS:			4084	4169	6

REMOVAL AND DISPOSAL OF PIPE CULVERTS

STATION	DESCRIPTION	PIPE CULVERTS
		EACH
117+32	12" X 30' CMP LT. SIDE DRAIN	1
118+29	12" X 24' CMP RT. SIDE DRAIN	1
TOTAL:		2

NOTE: ALL SALVAGEABLE PIPE CULVERTS SHALL BECOME THE PROPERTY OF MONTGOMERY COUNTY.

REMOVAL AND DISPOSAL OF SIGNS

STATION	SIDE	DESCRIPTION	SIGNS
			EACH
105+58	RT.	ONE LANE BRIDGE	1
112+27	RT.	ONE LANE BRIDGE	1
TOTAL:			2

EARTHWORK

STATION	STATION	LOCATION / DESCRIPTION	UNCLASSIFIED EXCAVATION			COMPACTED EMBANKMENT		
			NORMAL	ADD'L	TOTAL	NORMAL	ADD'L	TOTAL
CUBIC YARDS								
97+00	109+00	MAIN LANES	2695		2695	6289		6289
110+50	119+00	MAIN LANES	14017		14017	6928		6928
99+05	99+22	OBLITERATION OF EXISTING DRIVEWAY, LT.		2	2			
101+34	109+12	OBLITERATION OF EXISTING ROADWAY		159	159		400	400
109+65	113+52	OBLITERATION OF EXISTING ROADWAY		125	125		246	246
98+85		FIELD ENT, RT.		155	155		12	12
98+89		DRIVEWAY, LT.		41	41		23	23
107+00		FIELD ENT, LT.		24	24		402	402
116+78		DRIVEWAY, LT.		2269	2269		33	33
116+78		FIELD ENT, RT.		300	300		224	224
117+70		FIELD ENT, LT.		1418	1418		35	35
118+25		FIELD ENT, RT.		91	91		71	71
ENTIRE PROJECT		FILTER BLANKET / DUMPED RIPRAP		500	500			
TOTALS:			16712	5084	21796	13217	1446	14663

NOTE: EARTHWORK QUANTITIES SHOWN ABOVE SHALL BE PAID AS PLAN QUANTITY.



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	9	85	
								QUANTITIES

BASE AND SURFACING

STARTING STATION	ENDING STATION	LOCATION / DESCRIPTION	LENGTH	AGGREGATE BASE COURSE (CLASS 7)	PRIME COAT			DOUBLE ASPHALT SURFACE TREATMENT			
					TOTAL WIDTH	SQUARE YARDS	GALLONS	TOTAL WIDTH	SQUARE YARDS	MINERAL AGGREGATE (CLASS 5)	ASPHALT CEMENT (CRS-2P)
97+00.	98+83.02	MAIN LANES	183.02	247.08	21.00	427.05	170.82	20.00	406.71	16.27	305.03
98+83.02	101+33.02	MAIN LANES	250.00	345.00	21.00	583.33	233.33	20.00	555.56	22.22	416.67
101+33.02	106+02.73	MAIN LANES	469.71	657.59	21.00	1095.99	438.40	20.00	1043.80	41.75	782.85
106+02.73	107+34.52	MAIN LANES	131.79	181.87	21.00	307.51	123.00	20.00	292.87	11.71	219.65
107+34.52	107+67.52	MAIN LANES	33.00	53.79	21.00	77.00	30.80	20.00	73.33	2.93	55.00
107+67.52	108+31.92	MAIN LANES	64.40	115.92	21.00	150.27	60.11	20.00	143.11	5.72	107.33
108+31.92	108+52.52	MAIN LANES	20.60	36.05	21.00	48.07	19.23	20.00	45.78	1.83	34.34
108+52.52	108+61.92	MAIN LANES	9.40	15.79	21.00	21.93	8.77	20.00	20.89	0.84	15.67
108+61.92	110+56.08	BRIDGE NO. 04930	194.16	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
110+56.08	110+86.08	MAIN LANES	30.00	51.00	21.00	70.00	28.00	20.00	66.67	2.67	50.00
110+86.08	111+50.48	MAIN LANES	64.40	111.41	21.00	150.27	60.11	20.00	143.11	5.72	107.33
111+50.48	111+83.48	MAIN LANES	33.00	51.15	21.00	77.00	30.80	20.00	73.33	2.93	55.00
111+83.48	112+21.54	MAIN LANES	38.06	51.38	21.00	88.81	35.52	20.00	84.58	3.38	63.44
112+21.54	114+71.54	MAIN LANES	250.00	345.00	21.00	583.33	233.33	20.00	555.56	22.22	416.67
114+71.54	116+27.62	MAIN LANES	156.08	218.51	21.00	364.19	145.68	20.00	346.84	13.87	260.13
116+27.62	118+77.62	MAIN LANES	250.00	345.00	21.00	583.33	233.33	20.00	555.56	22.22	416.67
118+77.62	119+00.	MAIN LANES	22.38	28.42	21.00	52.22	20.89	20.00	49.73	1.99	37.30
98+85.		FIELD ENT. RT.	90.00	78.68	17.00	186.85	74.74	16.00	179.07	7.16	134.30
98+89.		DRIVEWAY LT.	50.00	46.47	17.00	111.30	44.52	16.00	107.96	4.32	80.97
107+00.		FIELD ENT. LT.	140.00	118.95	17.00	281.30	112.52	16.00	267.96	10.72	200.97
116+78.		DRIVEWAY LT.	199.69	167.02	17.00	394.04	157.62	16.00	374.08	14.96	280.56
116+78.		FIELD ENT. RT.	115.00	98.81	17.00	234.07	93.63	16.00	223.52	8.94	167.64
117+70.		FIELD ENT. LT.	180.00	151.16	17.00	356.85	142.74	16.00	339.07	13.56	254.30
118+25.		FIELD ENT. RT.	130.00	110.89	17.00	262.41	104.96	16.00	250.19	10.01	187.64
MAINTENANCE OF TRAFFIC ENTIRE PROJECT				400.00							
TOTALS:				4026.94		2602.85			247.94	4649.46	

USE: 4027 2603 248 4649

BASIS OF ESTIMATE:

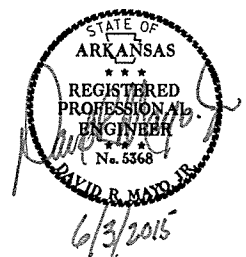
AGGREGATE BASE COURSE (VARIABLE) _____ 135 TONS / STA. (TYPICAL)
 PRIME COAT _____ 0.40 GAL. / SQ. YD.
 POLYMER MODIFIED CATIONIC EMULSIFIED ASPHALT (CRS-2P) (1ST APPLICATION) _____ 0.40 GALS. PER SQ. YD.
 POLYMER MODIFIED CATIONIC EMULSIFIED ASPHALT (CRS-2P) (2ND APPLICATION) _____ 0.35 GALS. PER SQ. YD.
 MINERAL AGGREGATE IN ASPHALT SURFACE TREATMENT (CLASS 5) (1ST APPLICATION) _____ 40 LBS. PER SQ. YD.
 MINERAL AGGREGATE IN ASPHALT SURFACE TREATMENT (CLASS 5) (2ND APPLICATION) _____ 40 LBS. PER SQ. YD.

NOTE: RATES MAY BE MODIFIED IF AND WHERE DIRECTED BY THE ENGINEER. SEE SECTION 104.03 OF THE STANDARD SPECIFICATIONS.

APPROACH GUTTERS

STATION	STATION	SIDE	APPROACH GUTTERS (TYPE A)	REINFORCING STEEL - ROADWAY (GRADE 60)
			CU. YDS.	LBS.
108+32	108+62	LT.	3.40	285
108+32	108+62	RT.	3.40	285
110+56	110+86	LT.	3.40	285
110+56	110+86	RT.	3.40	285
TOTALS:			13.60	1140

NOTE: W = 3' - 0"



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	10	85	
								QUANTITIES

TRAFFIC CONTROL DEVICES

DESCRIPTION	W20-1						G20-1 (ROAD WORK NEXT XX MILES)		G20-2 (END ROAD WORK)		*BARRICADES (TYPE III)	*TRAFFIC DRUMS	*VERTICAL PANELS	STANDARD DRAWING NUMBER
	1500 FT.		1000 FT.		500 FT.		NO.	SQ. FT.	NO.	SQ. FT.	LIN. FT.	EACH	EACH	
	NO.	SQ. FT.	NO.	SQ. FT.	NO.	SQ. FT.								
STATION 82+00 RT.	1	16.00												TC-1, TC-2, TC-3
STATION 87+00 RT.			1	16.00										TC-1, TC-2, TC-3
STATION 92+00 RT.					1	16.00								TC-1, TC-2, TC-3
STATION 92+00 LT.								1	8.00					TC-1, TC-2, TC-3
STATION 97+00 RT.							1	10.00						TC-1, TC-2, TC-3
STATION 119+00 LT.							1	10.00						TC-1, TC-2, TC-3
STATION 124+00 RT.									1	8.00				TC-1, TC-2, TC-3
STATION 124+00 LT.					1	16.00								TC-1, TC-2, TC-3
STATION 129+00 LT.			1	16.00										TC-1, TC-2, TC-3
STATION 134+00 LT.	1	16.00												TC-1, TC-2, TC-3
ENTIRE PROJECT										32	10*	40*		TC-1, TC-2, TC-3
TOTALS:	2	32.00	2	32.00	2	32.00	2	20.00	2	16.00	32*	10*	40*	

NOTE: LOCATION OF THE TRAFFIC CONTROL DEVICES TO BE AS DIRECTED BY THE ENGINEER.

*NOTE: ESTIMATED QUANTITY. TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER. SEE SECTION 104.03 OF THE STANDARD SPECIFICATIONS.

STRUCTURES

STATION	DESCRIPTION	SIDE DRAINS			STANDARD DRAWING NUMBERS
		18"	30"	21" X 15"	
		LINEAR FT.			
98+85.	INSTALL PIPE CULVERT RT. SIDE DRAIN			32	PCM-1, PCC-1
98+89.	INSTALL PIPE CULVERT LT. SIDE DRAIN			32	PCM-1, PCC-1
107+00.	INSTALL PIPE CULVERT LT. SIDE DRAIN		62		PCM-1, PCC-1, PCP-1, PCP-2
116+78.	INSTALL PIPE CULVERT LT. SIDE DRAIN			30	PCM-1, PCC-1
116+78.	INSTALL PIPE CULVERT RT. SIDE DRAIN	62			PCM-1, PCC-1
117+70.	INSTALL PIPE CULVERT LT. SIDE DRAIN			30	PCM-1, PCC-2
TOTALS:		62	62	124	

NOTE: FOR C.M. PIPE CULVERT INSTALLATIONS USE TYPE 2 BEDDING UNLESS OTHERWISE SPECIFIED.
NOTE: FOR PLASTIC PIPE CULVERT INSTALLATIONS USE TYPE 2 BEDDING UNLESS OTHERWISE SPECIFIED.

4" PIPE UNDERDRAIN

LOCATION	4' PIPE UNDERDRAIN	UNDERDRAIN OUTLET PROTECTORS
	LINEAR FT.	EACH
ENTIRE PROJECT	200	5
TOTALS:	200	5

NOTE: ESTIMATED QUANTITY. TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER.
SEE SECTION 104.03 OF THE STANDARD SPECIFICATIONS.

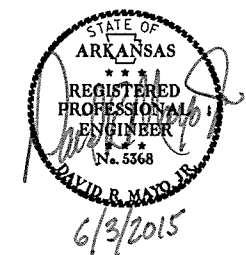
GUARDRAIL

STATION	STATION	SIDE	GUARDRAIL (TYPE A)	TERMINAL ANCHOR POST (TYPE 1)	THREE BEAM GUARDRAIL TERMINAL
			LINEAR FT.	EACH	EACH
107+84	108+53	LT.	50	1	1
107+84	108+53	RT.	50	1	1
110+65	111+34	LT.	50	1	1
110+65	111+34	RT.	50	1	1
TOTALS:			200	4	4

EROSION CONTROL

STATION	STATION	LOCATION	PERMANENT EROSION CONTROL			
			LIME	SEEDING	MULCH COVER	WATER
			TONS	ACRES	ACRES	M. GALS.
97+00	109+00	MAIN LANES	2	0.85	0.85	86.7
110+00	119+00	MAIN LANES	3	1.31	1.31	133.6
98+85		FIELD ENT, RT.	0	0.03	0.03	3.1
98+89		DRIVEWAY, LT.	0	0.01	0.01	1.0
107+00		FIELD ENT, LT.	0	0.06	0.06	6.1
116+78		DRIVEWAY, LT.	0	0.17	0.17	17.3
116+78		FIELD ENT, RT.	0	0.06	0.06	6.1
117+70		FIELD ENT, LT.	0	0.13	0.13	13.3
118+25		FIELD ENT, RT.	0	0.04	0.04	4.1
TOTALS:			5	2.66	2.66	271.3

BASIS OF ESTIMATE:
LIME..... 2 TONS / ACRE OF SEEDING
WATER..... 102.0 M.G. / ACRE OF SEEDING, PERMANENT SEEDING



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	11	85	
								④ QUANTITIES

TEMPORARY EROSION CONTROL

STATION	STATION	LOCATION	TEMPORARY SEEDING	MULCH COVER	WATER	SILT FENCE (E-11)	SAND BAG DITCH CHECKS (E-5)	ROCK DITCH CHECKS (E-6)	SEDIMENT REMOVAL AND DISPOSAL
			ACRES	ACRES	M. GALS	LIN. FT.	BAGS	CU. YDS.	CU. YDS.
97+00	109+41	MAIN LANES	0.85	0.85	17.3	1568	260	20	106
101+34	109+11	OBLITERATION	0.36	0.36	7.3				
109+59	119+00	MAIN LANES	1.31	1.31	26.7	1694	280	20	112
109+65	113+73	OBLITERATION	0.23	0.23	4.7				
FIELD ENT. 98+85		RT.	0.03	0.03	0.6		80		13
DRIVEWAY 98+89		LT.	0.01	0.01	0.3		40		7
FIELD ENT. 107+00		LT.	0.06	0.06	1.3		80		13
DRIVEWAY 116+78		LT.	0.17	0.17	3.5		120		20
FIELD ENT. 116+78		RT.	0.06	0.06	1.2		80		13
FIELD ENT. 117+70		LT.	0.13	0.13	2.6		120		
FIELD ENT. 118+25		RT.	0.04	0.04	0.9		120		
*ENTIRE PROJECT AS DIRECTED BY ENGINEER									
TOTALS:			3.25	3.25	66.4	3262	1180	40	284

BASIS OF ESTIMATE:

WATER..... 20.4 M.G. / ACRE OF SEEDING, TEMPORARY SEEDING

NOTE: TEMPORARY EROSION CONTROL DEVICES SHALL BE INSTALLED IN SUCH A SEQUENCE AS TO DETER EROSION AND SEDIMENTATION ON U.S. WATERWAYS AS EXPLAINED BY THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM PERMIT.

NOTE: EROSION CONTROL ITEMS ARE SUBJECT TO IMMEDIATE PLACEMENT AS DIRECTED BY THE ENGINEER. EXACT LOCATIONS TO BE DETERMINED IN THE FIELD BY THE ENGINEER.

MAILBOXES

LOCATION	SIDE	MAILBOXES	MAILBOX SUPPORTS (SINGLE)
		EACH	EACH
98+59	LT.	1	1
116+50	LT.	1	1
TOTALS:		2	2

REFLECTORIZED PAINT PAVEMENT MARKING

STATION		REFLECTORIZED PAINT PAVEMENT MARKING YELLOW (4") CONTINUOUS
FROM	TO	LINEAR FEET
97+00	119+00	4400
TOTAL:		4400

NOTE: THIS IS A LOW VOLUME ROAD AS DEFINED IN SECTION 604.03 OF THE STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION, EDITION OF 2014.

STANDARD HIGHWAY SIGNS AND SUPPORT ASSEMBLIES

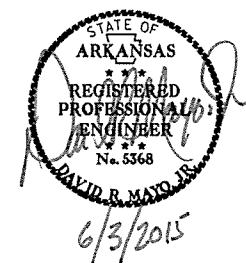
STATION	SIDE	STANDARD SIGN NUMBER						SUPPORT ASSEMBLIES		STANDARD DRAWING NUMBER
		W8-3		OM-3L		OM-3R		TYPE A	TYPE C	
		NO.	SQ. FT.	NO.	SQ. FT.	NO.	SQ. FT.			
103+00	LT.	1	9.00					1		SHS 1&2
108+62	LT.			1	3.00				1	SHS 1&2
108+62	RT.					1	3.00		1	SHS 1&2
110+56	LT.					1	3.00		1	SHS 1&2
110+56	RT.			1	3.00				1	SHS 1&2
116+00	RT.	1	9.00					1		SHS 1&2
TOTALS:		2	18.00	2	6.00	2	6.00	2	4	

NOTE: ALL STANDARD SIGN BLANKS TO BE 0.08" THICK. REFER TO STANDARD DRAWING SHS - 2 FOR CHANNEL POST SPLICING DETAILS.

FILTER BLANKET AND DUMPED RIPRAP

STATION	STATION	SIDE	FILTER BLANKET	DUMPED RIPRAP
			SQ. YDS.	CU. YDS.
ENTIRE PROJECT			1000	500
TOTALS:			1000	500

NOTE: TO BE USED IF AND WHERE DIRECTED BY THE ENGINEER. SEE SECTION 104.03 OF THE STANDARD SPECIFICATIONS.



SUMMARY OF QUANTITIES

ITEM NO.	ITEM	QUANTITY	UNIT
SP&201	CLEARING	22	STA
201	GRUBBING	22	STA
202	REMOVAL AND DISPOSAL OF FENCE	4084	LF
202	REMOVAL AND DISPOSAL OF PIPE CULVERTS	2	EACH
202	REMOVAL AND DISPOSAL OF SIGNS	2	EACH
210	UNCLASSIFIED EXCAVATION	21796	CUYD
210	COMPACTED EMBANKMENT	14663	CUYD
303	AGGREGATE BASE COURSE (CLASS 7)	4027	TON
401	PRIME COAT	2603	GAL
402	MINERAL AGGREGATE IN ASPHALT SURFACE TREATMENT (CLASS 5)	248	TON
402	POLYMER MODIFIED CATIONIC EMULSIFIED ASPHALT (CRS-2P)	4649	GAL
504	APPROACH GUTTERS	13.60	CUYD
601	MOBILIZATION	1.00	L.S.
SP&602	FURNISHING FIELD OFFICE	1	EACH
603	MAINTENANCE OF TRAFFIC	1.00	L.S.
SS&604	SIGNS	132	SQFT
SS&604	BARRICADES	32	LF
SS&604	TRAFFIC DRUMS	10	EACH
SS&604	VERTICAL PANELS	40	EACH
SPSS606	18" SIDE DRAIN	62	LF
SPSS606	30" SIDE DRAIN	62	LF
SS&606	21" X 15" SIDE DRAIN	124	LF
611	UNDERDRAIN OUTLET PROTECTORS	5	EACH
611	4" PIPE UNDERDRAINS	200	LF
617	GUARDRAIL (TYPE A)	200	LF
617	TERMINAL ANCHOR POSTS (TYPE 1)	4	EACH
617	THREE BEAM GUARDRAIL TERMINAL	4	EACH
619	WIRE FENCE (TYPE D-1)	4169	LF
619	18' STEEL GATES (ALTERNATE NO. 1)	6	EACH
619	18' ALUMINUM GATES (ALTERNATE NO. 2)	6	EACH
620	LIME	5	TON
620	SEEDING	2.66	ACRE
SS&620	MULCH COVER	5.91	ACRE
620	WATER	337.7	MGAL
621	TEMPORARY SEEDING	3.25	ACRE
621	SILT FENCE	3262	LF
621	SAND BAG DITCH CHECKS	1180	BAG
621	SEDIMENT REMOVAL AND DISPOSAL	284	CUYD
621	ROCK DITCH CHECKS	40	CUYD
635	ROADWAY CONSTRUCTION CONTROL	1.00	L.S.
637	MAILBOXES	2	EACH
637	MAILBOX SUPPORTS (SINGLE)	2	EACH
718	REFLECTORIZED PAINT PAVEMENT MARKING YELLOW (4")	4400	LF
726	STANDARD SIGN	30.00	SQFT
729	CHANNEL POST SIGN SUPPORT (TYPE A)	2	EACH
729	CHANNEL POST SIGN SUPPORT (TYPE C)	4	EACH
804	REINFORCING STEEL-ROADWAY (GRADE 60)	1140	LB
816	FILTER BLANKET	1000	SQYD
816	DUMPED RIPRAP	500	CUYD
STRUCTURES OVER 20'-0" SPAN			
205	REMOVAL OF EXISTING BRIDGE STRUCTURE (SITE NO. 1)	1.00	L.S.
636	BRIDGE CONSTRUCTION CONTROL	1.00	L.S.
801	UNCLASSIFIED EXCAVATION FOR STRUCTURES-BRIDGE	189	CUYD
802	CLASS S CONCRETE-BRIDGE	116.10	CUYD
802	CLASS S(AE) CONCRETE-BRIDGE	162.80	CUYD
803	CLASS 1 PROTECTIVE SURFACE TREATMENT	13.6	GAL
804	REINFORCING STEEL-BRIDGE (GRADE 60)	53850	LB
805	STEEL PILING (HP 12X53)	156	LF
807	STRUCTURAL STEEL IN BEAM SPANS (M270-GR50W)	122250	LB
808	ELASTOMERIC BEARINGS	4386.0	CUIN
809	SILICONE JOINT SEALANT	54	LF
812	BRIDGE NAME PLATE (TYPE C)	1	EACH
816	FILTER BLANKET	752	SQYD
816	DUMPED RIPRAP	409	CUYD

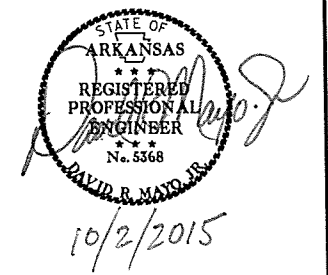
* DENOTES ALTERNATE BID ITEMS.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
10/17/15				6	ARK.		13	85
						JOB NO.	BR4906	

4 SUMMARY OF QUANTITIES AND REVISIONS

REVISIONS

DATE	REVISION	SHEET NUMBER
10/1/2015	ADDED SPECIAL PROVISION: SPECIAL CLEARING REQUIREMENTS	2, 13



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	14	85	

4 SURVEY CONTROL DETAILS

MIDPOINT:
 LT: 34-25-47
 LG: 93-25-52

SURVEY CONTROL COORDINATES

Project Name: sbr4906
 Date: 5/7/2013
 Coordinate System: ARKANSAS STATE PLANE - SOUTH ZONE BASED ON FAST STATIC.
 OBSERVATIONS AT POINTS 2 AND 4 PROJECTED TO GROUND.
 Units: U.S. SURVEY FOOT

Point Name	Northing	Easting	Elev	Feature	Description
1	1957238.8514	879283.2011	633.054	CTL	*5/8" Rebar with 2" Aluminum Cap stamped pnt 1
2	1957491.4536	879968.2798	631.810	CTL	*5/8" Rebar with 2" Aluminum Cap stamped pnt 2
3	1957515.9887	880356.3269	613.831	CTL	*5/8" Rebar with 2" Aluminum Cap stamped pnt 3
4	1957393.3600	88024.9997	610.556	CTL	*5/8" Rebar with 2" Aluminum Cap stamped pnt 4
5	1957106.7918	880897.4770	599.070	CTL	*5/8" Rebar with 2" Aluminum Cap stamped pnt 5
6	1956913.5809	881484.0974	636.484	CTL	*5/8" Rebar with 2" Aluminum Cap stamped pnt 6
7	1956898.4176	881851.0597	624.064	CTL	*5/8" Rebar with 2" Aluminum Cap stamped pnt 7
8	1957066.4943	882308.2444	649.100	CTL	*5/8" Rebar with 2" Aluminum Cap stamped pnt 8
900	-99999.0000	-99999.0000	631.981	TBM	*RR SPIKE IN PP IN FE LINE CR 34 20.0' S OF C/L CR 34
901	1957157.7033	880862.5907	599.832	TBM	*CHIS SQ CUT NE CORN BR CR 34
902	-99999.0000	-99999.0000	650.332	TBM	*RR SPIKE IN PP CR 34 BONNERDALE 9' S OF FE 15.5' N OF C/L CR 34
1500	1957075.0861	882339.2119	650.859	CTL	*RESECT REFERENCE 8" SPIKE NL
1501	1957035.9697	882348.2236	649.919	CTL	*RESECT REFERENCE 8" SPIKE NL
1502	1957024.8567	882295.6660	648.253	CTL	*RESECT REFERENCE 8" SPIKE NL
1503	1957066.6434	882289.8085	649.454	CTL	*RESECT REFERENCE 8" SPIKE NL
1504	1956900.0718	881888.8104	623.871	CTL	*RESECT REFERENCE 8" SPIKE
1505	1956936.2600	881876.5933	623.850	CTL	*RESECT REFERENCE 8" SPIKE
1506	1956925.2476	881808.1253	623.511	CTL	*RESECT REFERENCE 8" SPIKE
1507	1956896.5376	881789.9137	622.035	CTL	*RESECT REFERENCE 8" SPIKE
1508	1956952.7164	881554.9831	637.352	CTL	*RESECT REFERENCE 8" SPIKE
1509	1956910.2687	881510.4202	636.654	CTL	*RESECT REFERENCE 8" SPIKE
1510	1956902.6160	881469.2346	636.845	CTL	*RESECT REFERENCE 8" SPIKE
1511	1956949.5691	881506.5238	640.501	CTL	*RESECT REFERENCE 8" SPIKE
1512	1957123.2694	880953.5784	601.114	CTL	*RESECT REFERENCE 8" SPIKE
1513	1957076.6953	880960.5680	601.434	CTL	*RESECT REFERENCE 8" SPIKE
1514	1957121.4802	880883.7092	599.151	CTL	*RESECT REFERENCE 8" SPIKE
1515	1957143.6101	880908.0454	599.526	CTL	*RESECT REFERENCE 8" SPIKE
1516	1957373.8993	880745.2841	609.016	CTL	*RESECT REFERENCE 8" SPIKE
1517	1957290.6100	880737.8374	604.430	CTL	*RESECT REFERENCE 8" SPIKE
1518	1957413.7558	880617.8179	614.952	CTL	*RESECT REFERENCE 8" SPIKE
1519	1957409.5535	880707.2008	611.641	CTL	*RESECT REFERENCE 8" SPIKE
1520	1957512.7395	880382.7443	614.337	CTL	*RESECT REFERENCE 8" SPIKE
1521	1957486.0792	880376.7298	613.896	CTL	*RESECT REFERENCE 8" SPIKE
1522	1957498.8363	880309.0228	614.281	CTL	*RESECT REFERENCE 8" SPIKE
1523	1957527.3618	880305.9826	615.084	CTL	*RESECT REFERENCE 8" SPIKE
1524	1957489.2664	879993.9532	628.776	CTL	*RESECT REFERENCE 8" SPIKE
1525	1957456.6806	879976.3497	632.314	CTL	*RESECT REFERENCE 8" SPIKE
1526	1957447.2948	879944.8379	632.188	CTL	*RESECT REFERENCE 8" SPIKE
1527	1957487.1893	879949.0733	632.182	CTL	*RESECT REFERENCE 8" SPIKE
1528	1957280.0301	879348.1616	632.830	CTL	*RESECT REFERENCE 8" SPIKE
1529	1957247.1002	879341.9551	632.521	CTL	*RESECT REFERENCE 8" SPIKE
1530	1957220.6329	879221.8195	636.099	CTL	*RESECT REFERENCE 8" SPIKE
1531	1957258.8952	879238.4620	635.271	CTL	*RESECT REFERENCE 8" SPIKE

*Note - Rebar and Cap - Standard - * Rebar with 2" Aluminum Cap stamped
 (Standard markings common to all caps), or as indicated
 (Other markings indicated in the point description of the individual point).
 ALL DISTANCES ARE GROUND.
 USE CAF = 1.0 FOR STAKEOUT FOR THIS PROJECT.
 A PROJECT CAF OF 0.9999123196 HAS BEEN USED TO COMPUTE THE ABOVE LISTED GROUND COORDINATES.
 THIS CAF IS INTENDED FOR USE WITHIN THE PROJECT LIMITS.
 GRID DISTANCE = GROUND DISTANCE X CAF.
 GROUND COORDINATES ARE PROJECTED FROM AR. STATE PLANE GRID COORDINATES BY SCALING ALL X,Y
 COORDINATE VALUES WITH THE INVERSE (1/X) OF THE COMBINED ADJUSTMENT FACTOR (CAF) ABOUT X+0,Y+0.

GRID COORDINATES ARE STORED UNDER FILE NAME: sbr4906gi.ct1
 HORIZONTAL DATUM: NAD 83 (1997)
 VERTICAL DATUM: NAVD 88 ELEVATIONS FOR POINTS 1-8, AND 900-902 WERE ESTABLISHED BY fast static observations.

POSITIONAL ACCURACY:
 HORIZONTAL-GPS(POINTS 1): 1.0 CM 10 PPM, PRIMARY CONTROL(POINTS 1-8): 2.0 CM 20 PPM
 VERTICAL-POSITIONAL ACCURACY IS THIRD ORDER, UNLESS SPECIFIED OTHERWISE AT A SPECIFIC POINT

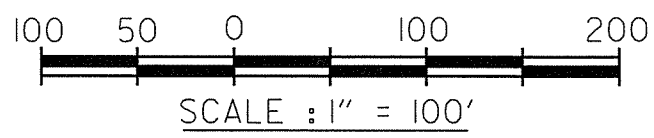
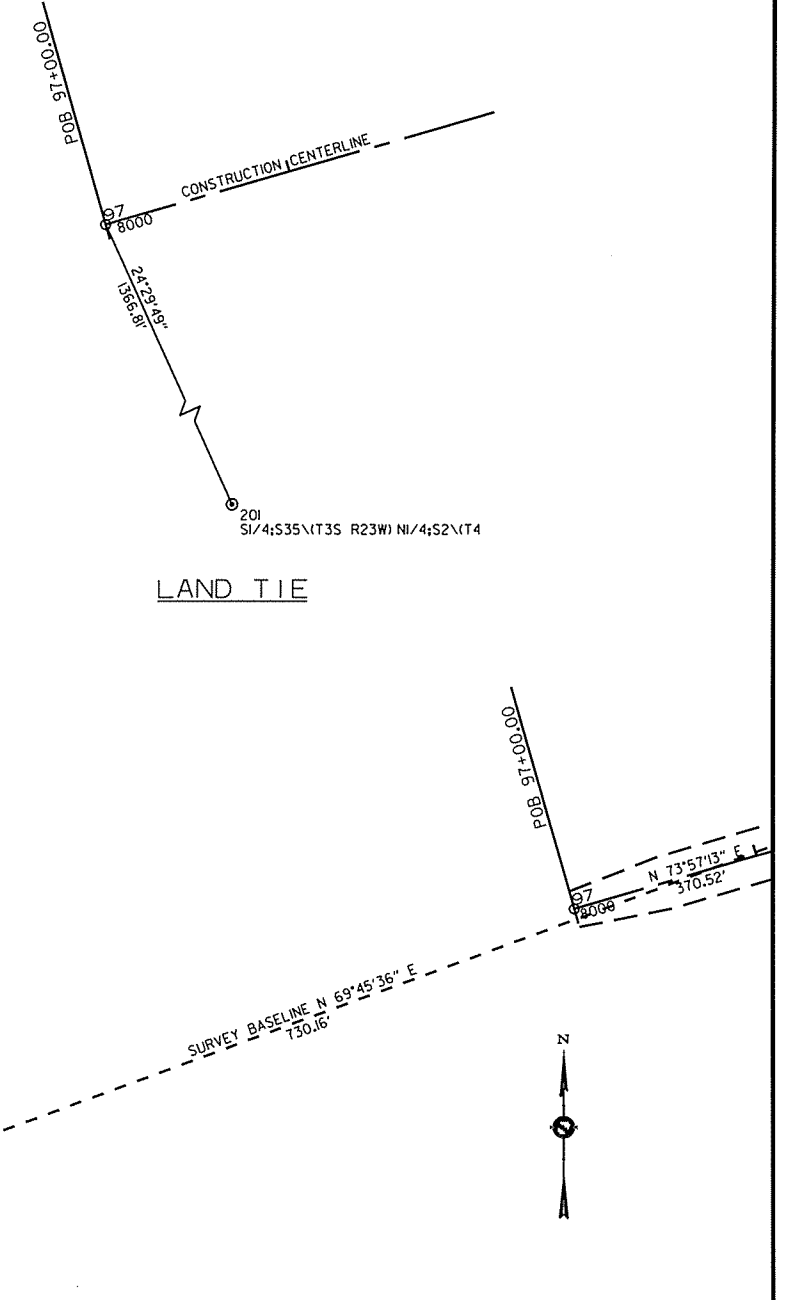
BASIS OF BEARING:
 ARKANSAS STATE PLANE GRID BEARINGS - 0302-SOUTH ZONE
 DETERMINED FROM GPS CONTROL POINTS FAST STATIC PN 2 AND 4
 CONVERGENCE ANGLE: 00 48 03.40 LEFT AT PN 5
 GRID AZIMUTH = ASTRONOMICAL AZIMUTH - CONVERGENCE ANGLE.

LT: 34-25-47.3 LG: 093-25-51.8
 GRID NORTHING: 1956935.1920 GRID EASTING: 880820.2396
 GROUND NORTHING: 1957106.7918 GROUND EASTING: 880897.4770

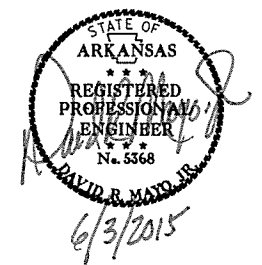
CONSTRUCTION CENTERLINE COORDINATES

Project Name: BR4906
 Date: 10/29/2014
 Coordinate System: ARKANSAS STATE PLANE - SOUTH ZONE
 Units: U.S. SURVEY FOOT

Point	Description	Station	Northing	Easting
8000	POB	97+00.00	1957422.5900	879765.9896
8001	PC	100+70.52	1957525.0068	880122.0741
8002	CC		1956895.7101	880303.0721
8003	PT	106+65.23	1957425.5256	880687.8711
8004	PC	114+09.04	1956988.4218	881289.7032
8005	CC		1957345.0283	881548.7025
8006	PT	116+90.12	1956904.3119	881552.9308
8007	POE	119+00.00	1956906.3254	881762.8023

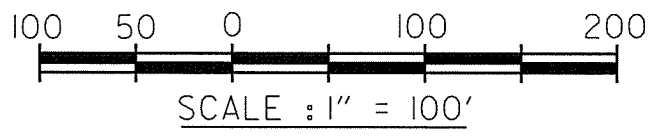
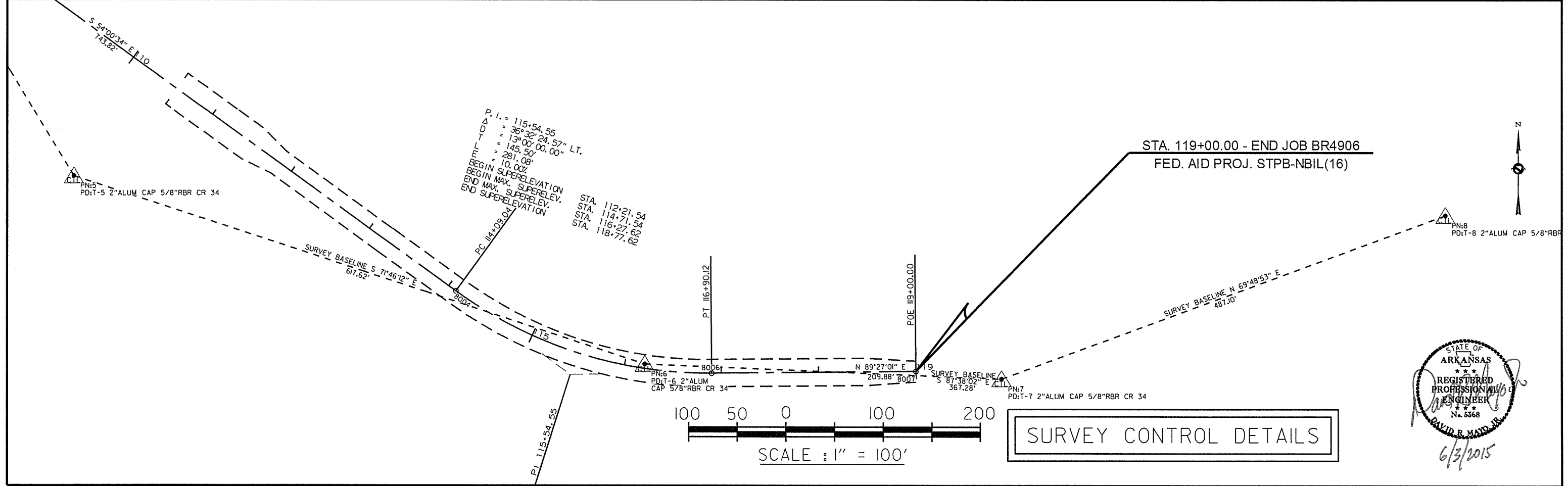
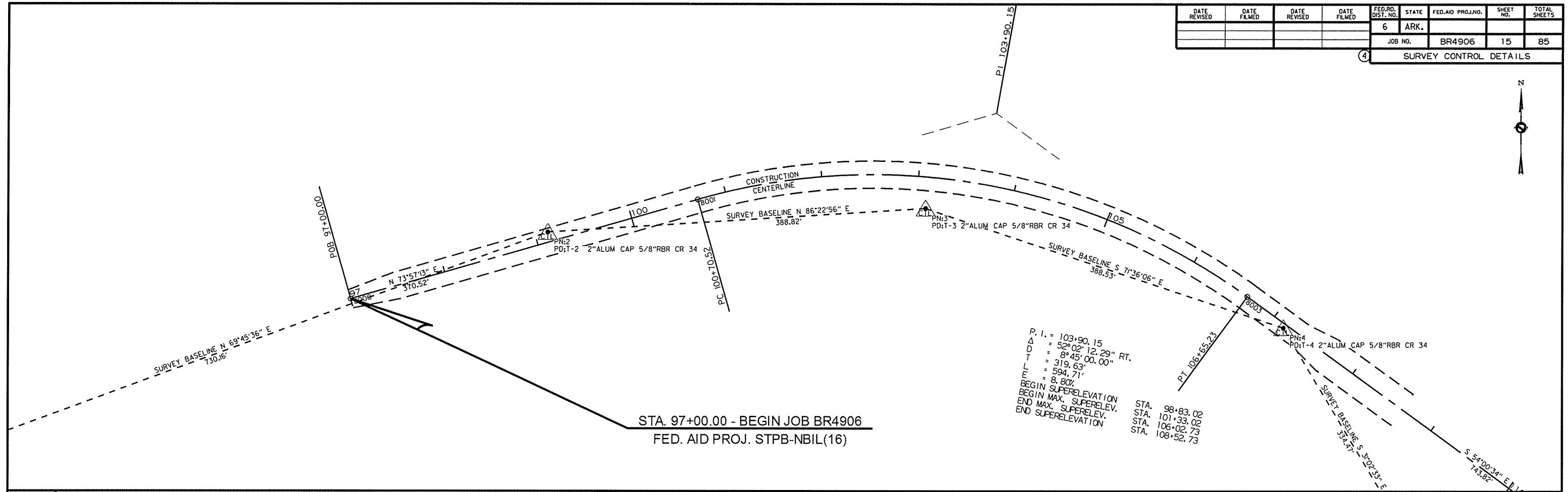


SURVEY CONTROL DETAILS



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	15	85	

4 SURVEY CONTROL DETAILS



SURVEY CONTROL DETAILS



6/3/2015

LEGEND

- --- POWER POLE
- ⊕ --- COMBINATION POLE
- ⊖ --- POLE W/GUY
- ⊞ --- TELEPHONE RISER
- ⊙ --- SIGN
- △ --- UNDERGROUND CABLE
- ⊕ --- MAILBOX

REMOVAL AND DISPOSAL OF FENCE

STA. 97+00 TO STA. 98+26 ON LT.	=	138 LIN. FT.
STA. 97+00 TO STA. 98+76 ON RT.	=	179 LIN. FT.
STA. 98+84 TO STA. 109+03 ON RT.	=	600 LIN. FT.
STA. 99+46 TO STA. 108+31 ON LT.	=	900 LIN. FT.

WIRE FENCE (TYPE D-1)

STA. 97+00 TO STA. 98+26 ON LT.	=	137 LIN. FT.
STA. 97+00 TO STA. 98+76 ON RT.	=	177 LIN. FT.
STA. 98+84 TO STA. 109+03 ON RT.	=	950 LIN. FT.
STA. 99+46 TO STA. 106+91 ON LT.	=	785 LIN. FT.

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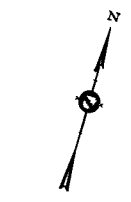
OBLITERATION OF EXISTING DRIVEWAY, LT.
STA. 99+05 TO STA. 99+22 = 2 CU.YDS.

STA. 98+89
INSTALL SIDE DRAIN ON LT.
21" X 15" X 32' ARCH PIPE CULVERT
CONSTRUCT APPROACH
= 23 CU. YDS. COMP. EMBANKMENT
= 41 CU. YDS. UNCL. EXCAVATION
= 46.5 TONS AGGR. BASE COURSE

CLEARING AND GRUBBING
STA. 97+00 TO STA. 119+00 = 22 STA.

NOTE: REMOVE AND DISPOSE OF THE EXISTING BASE FROM THE ABANDONED ROADWAY. THE DITCHES SHALL BE LEVELED AND THE SUBGRADE SCARIFIED BEFORE THE SEEDING OPERATION BEGINS.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		16	85



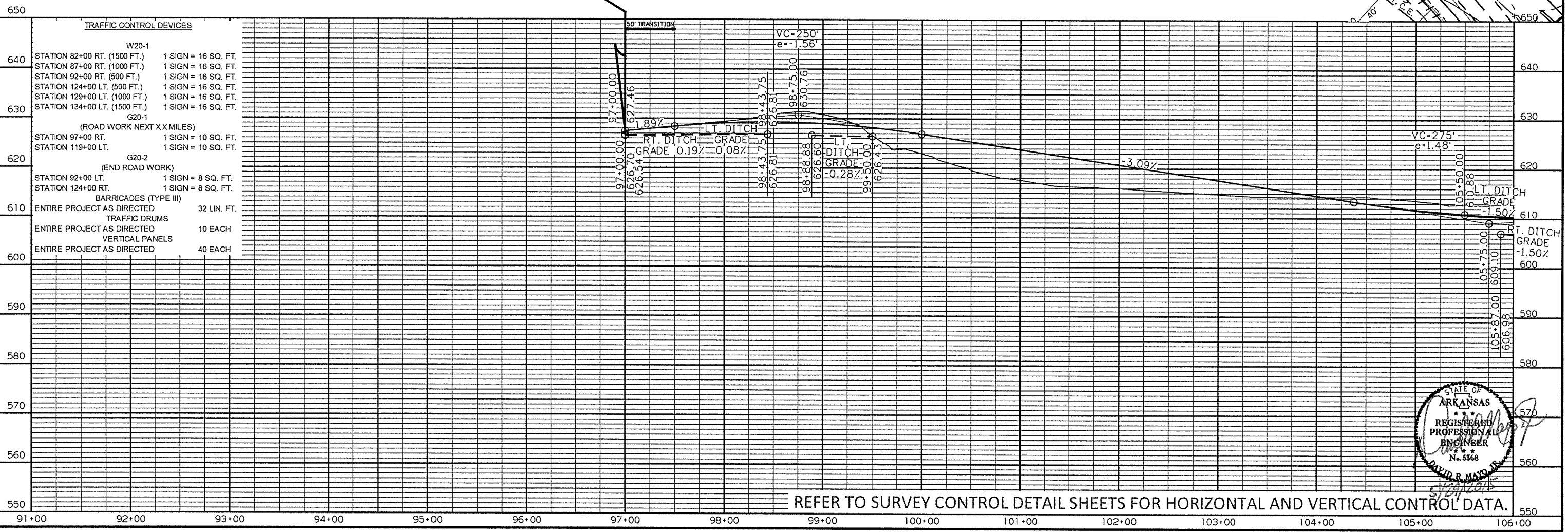
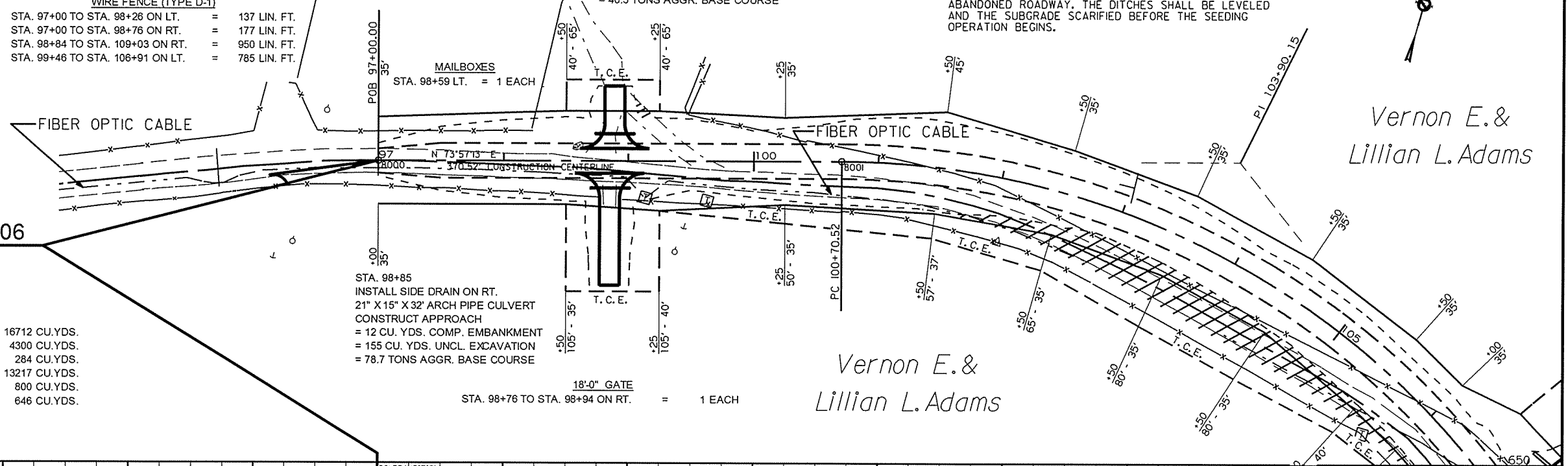
Vernon E. & Lillian L. Adams

STA. 97+00.00 - BEGIN JOB BR4906
FED. AID PROJ. STPB-NBIL(16)

EARTHWORK
STA. 97+00 TO STA. 119+00

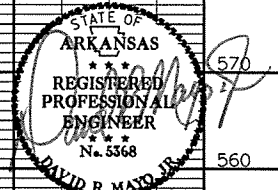
UNCLASSIFIED EXCAVATION (NORMAL)	=	16712 CU.YDS.
UNCLASSIFIED EXCAVATION (DRIVEWAYS / FIELD ENTRANCES)	=	4300 CU.YDS.
UNCLASSIFIED EXCAVATION (OBLITERATION OF EXISTING ROADWAY)	=	284 CU.YDS.
COMPACTED EMBANKMENT (NORMAL)	=	13217 CU.YDS.
COMPACTED EMBANKMENT (DRIVEWAYS / FIELD ENTRANCES)	=	800 CU.YDS.
COMPACTED EMBANKMENT (OBLITERATION OF EXISTING ROADWAY)	=	646 CU.YDS.

NOTE: EARTHWORK SHALL BE PAID AS PLAN QUANTITY.

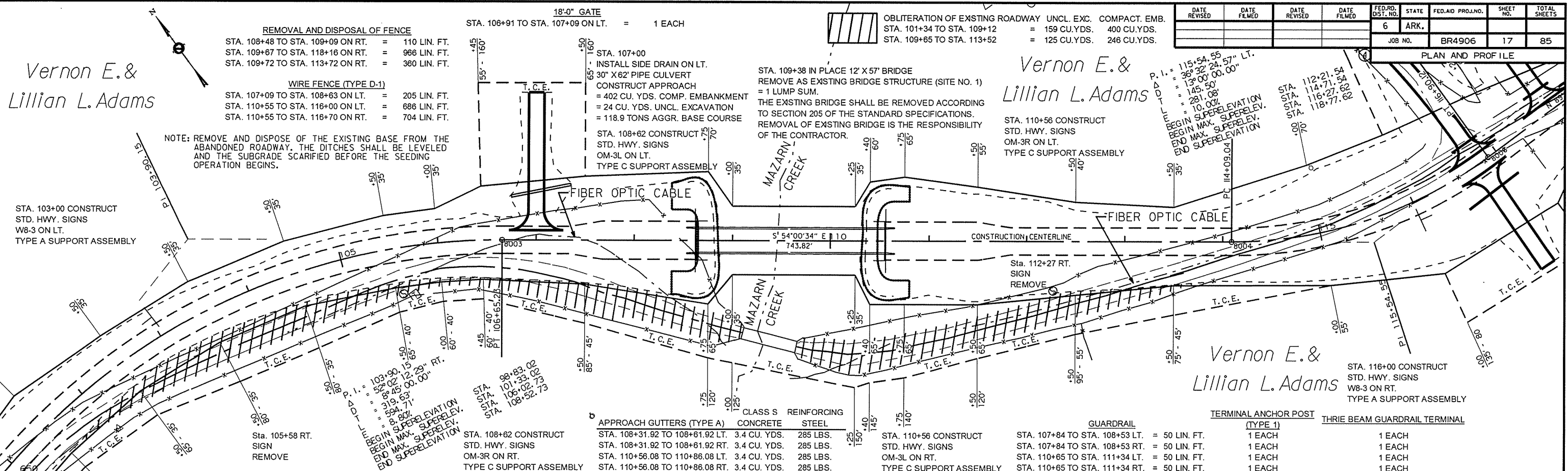


TRAFFIC CONTROL DEVICES

STATION	DESCRIPTION	QUANTITY
640	W20-1 SIGN	16 SQ. FT.
640	W20-1 SIGN	16 SQ. FT.
640	W20-1 SIGN	16 SQ. FT.
640	W20-1 SIGN	16 SQ. FT.
640	W20-1 SIGN	16 SQ. FT.
640	W20-1 SIGN	16 SQ. FT.
630	G20-1 SIGN	10 SQ. FT.
630	G20-1 SIGN	10 SQ. FT.
620	G20-2 SIGN	8 SQ. FT.
620	G20-2 SIGN	8 SQ. FT.
610	BARRICADES (TYPE III)	32 LIN. FT.
610	TRAFFIC DRUMS	10 EACH
610	VERTICAL PANELS	40 EACH



REFER TO SURVEY CONTROL DETAIL SHEETS FOR HORIZONTAL AND VERTICAL CONTROL DATA.



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		17	85

JOB NO. BR4906

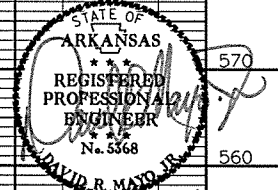
Vernon E. &
Lillian L. Adams

Vernon E. &
Lillian L. Adams

Vernon E. &
Lillian L. Adams

STA. 108+61.92 BRIDGE END
 BRIDGE NO. 04930
 CONT. COMP. W-BEAM UNIT
 24' - 0" CLEAR ROADWAY
 BRIDGE LENGTH = 194' - 2"
 STA. 110+56.08 BRIDGE END

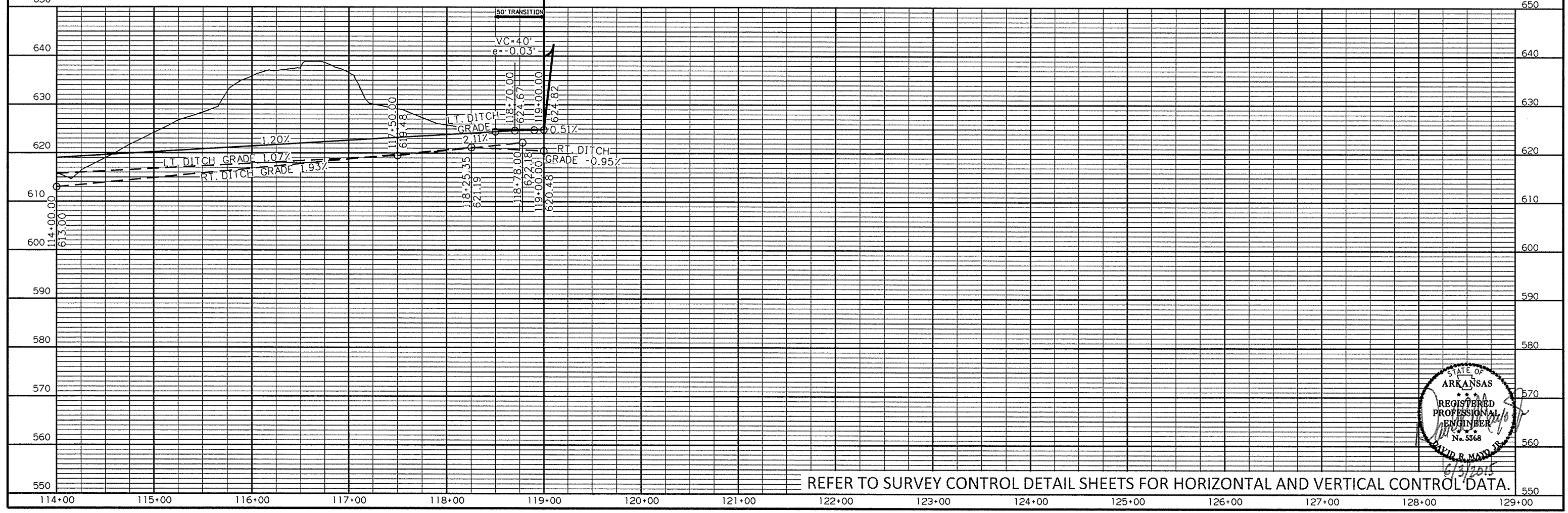
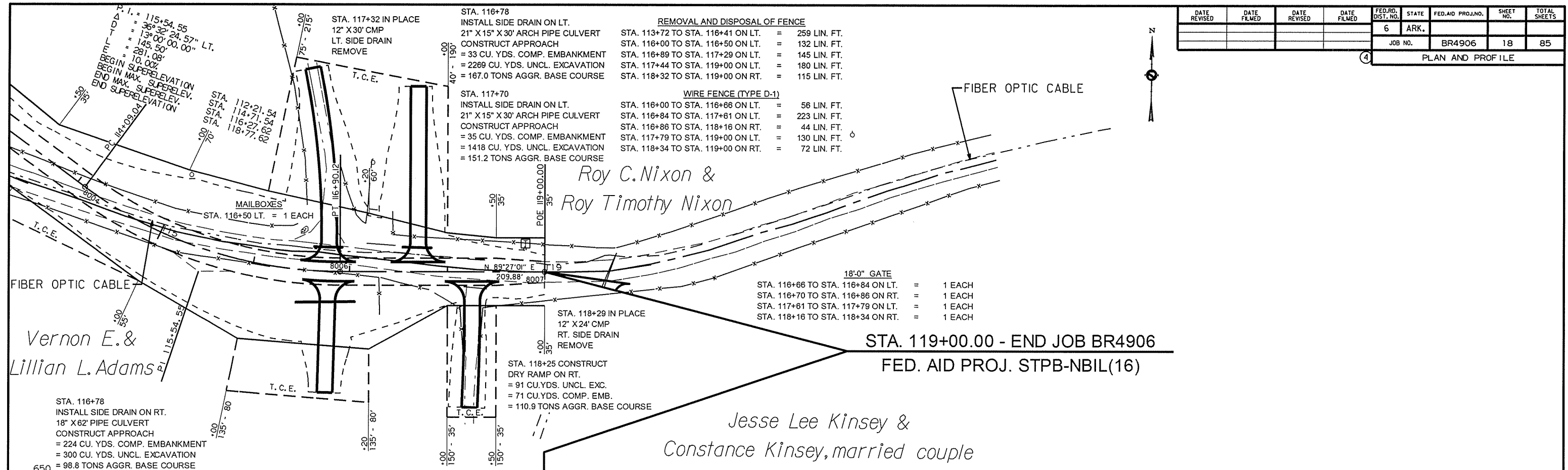
FOR THE CONSTRUCTION OF TEMPORARY WORK RAMP OR HAUL ROADS, THIS STREAM IS CLASSIFIED AS A PERENNIAL STREAM. THE STREAM BANK ELEVATIONS ARE 601 FEET MSL BETWEEN STATIONS 108+25 AND 110+40. REFER TO SECTION 110.06(c) OF THE 2014 STANDARD SPECIFICATIONS.



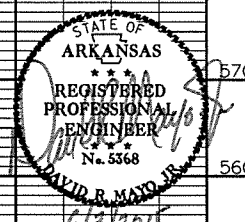
REFER TO SURVEY CONTROL DETAIL SHEETS FOR HORIZONTAL AND VERTICAL CONTROL DATA.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	18	85	

PLAN AND PROFILE

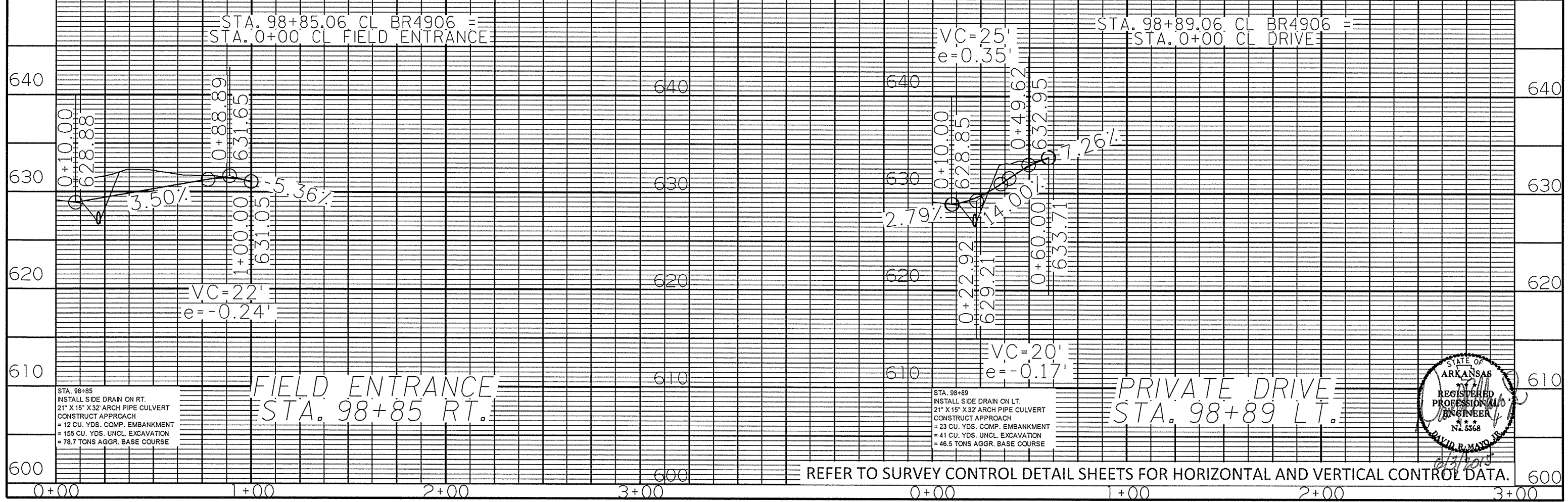
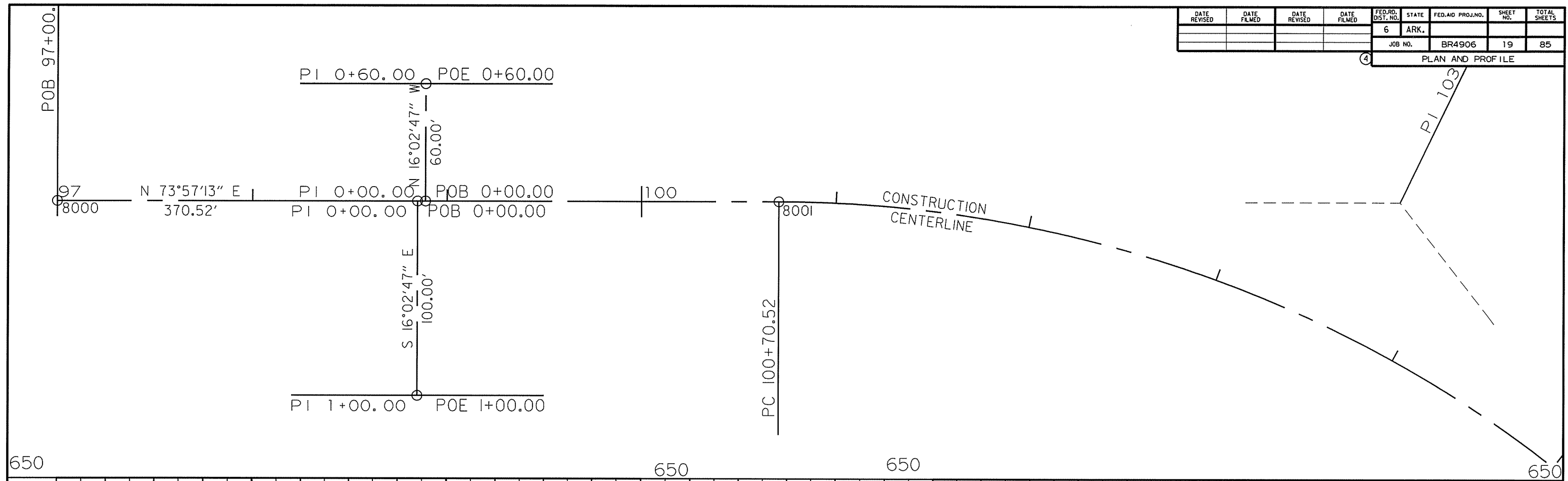


REFER TO SURVEY CONTROL DETAIL SHEETS FOR HORIZONTAL AND VERTICAL CONTROL DATA.



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	19	85	

PLAN AND PROFILE



STA. 98+85
 INSTALL SIDE DRAIN ON RT.
 21" X 15" X 32' ARCH PIPE CULVERT
 CONSTRUCT APPROACH
 = 12 CU. YDS. COMP. EMBANKMENT
 = 155 CU. YDS. UNCL. EXCAVATION
 = 78.7 TONS AGGR. BASE COURSE

FIELD ENTRANCE
 STA. 98+85 RT.

STA. 98+89
 INSTALL SIDE DRAIN ON LT.
 21" X 15" X 32' ARCH PIPE CULVERT
 CONSTRUCT APPROACH
 = 23 CU. YDS. COMP. EMBANKMENT
 = 41 CU. YDS. UNCL. EXCAVATION
 = 46.5 TONS AGGR. BASE COURSE

PRIVATE DRIVE
 STA. 98+89 LT.



REFER TO SURVEY CONTROL DETAIL SHEETS FOR HORIZONTAL AND VERTICAL CONTROL DATA.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	20	85	

④ PLAN AND PROFILE

PI 1+50.00 POE 1+50.00

N 35°59'26" E
150.00'

PI 0+00.00 POB 0+00.00

S 54°00'34" E 110
743.82'

CONSTRUCTION CENTER

105

650

650

STA. 107+00.00 CL BR4906 =
STA. 0+00 CL FIELD ENTRANCE

640

640

630

630

VC=23'
e=-0.22'

620

620

0+10.00
611.39
60+28.36
612.33

VC=107'
e=0.90'

1+50.00
612.89

610

610

5.13%

-2.57%

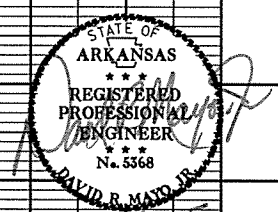
4.13%

FIELD ENTRANCE
STA. 107+00 LT.

600

0+95.10
610.62

- STA. 107+00
- INSTALL SIDE DRAIN ON LT.
- 30" X 62' PIPE CULVERT
- CONSTRUCT APPROACH
- = 402 CU. YDS. COMP. EMBANKMENT
- = 24 CU. YDS. UNCL. EXCAVATION
- = 118.9 TONS AGGR. BASE COURSE



REFER TO SURVEY CONTROL DETAIL SHEETS FOR HORIZONTAL AND VERTICAL CONTROL DATA.

0+00

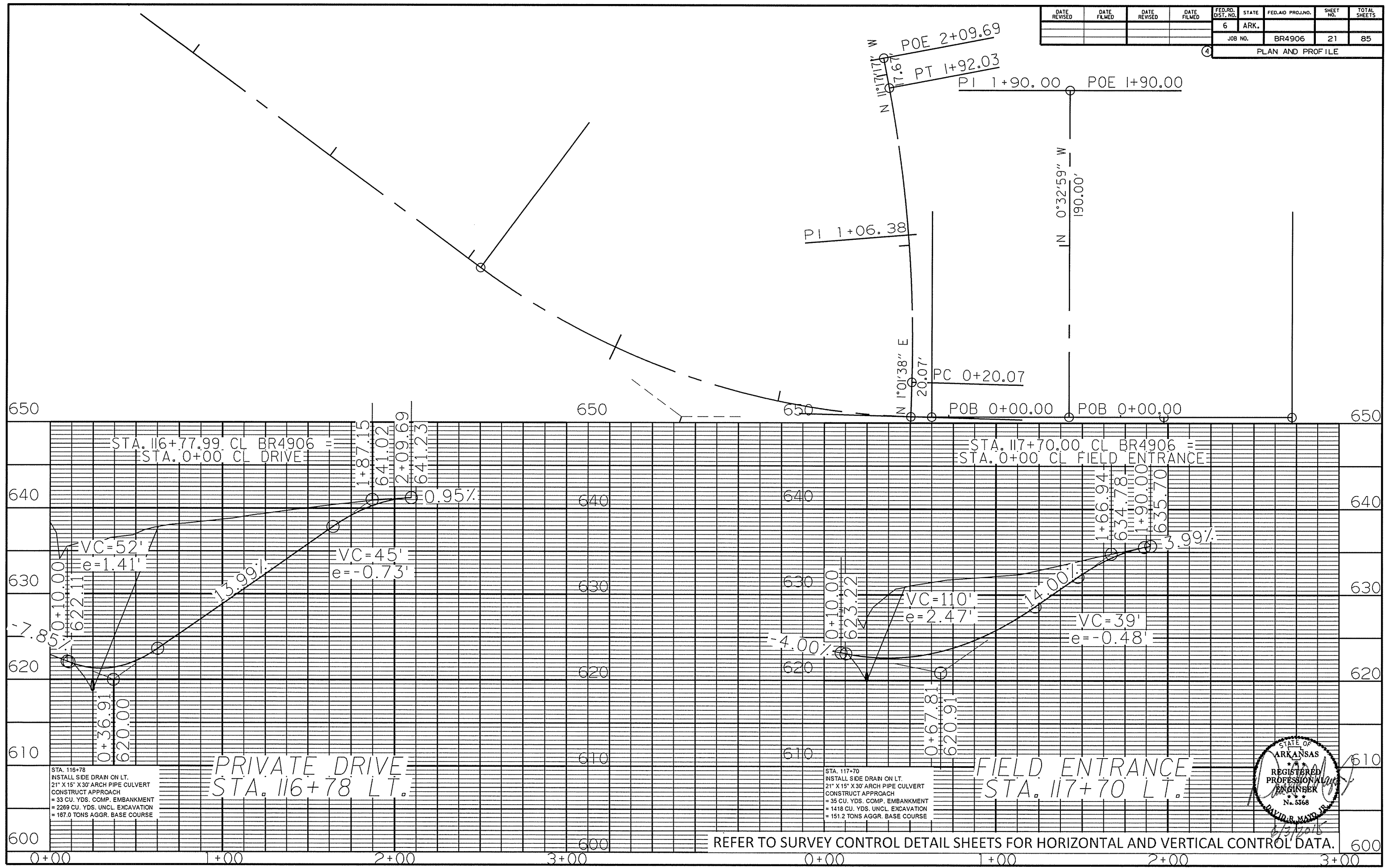
1+00

2+00

3+00

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	21	85	

PLAN AND PROFILE



STA. 116+78
 INSTALL SIDE DRAIN ON LT.
 21" X 15" X 30' ARCH PIPE CULVERT
 CONSTRUCT APPROACH
 = 33 CU. YDS. COMP. EMBANKMENT
 = 2269 CU. YDS. UNCL. EXCAVATION
 = 167.0 TONS AGGR. BASE COURSE

PRIVATE DRIVE
 STA. 116+78 LT.

STA. 117+70
 INSTALL SIDE DRAIN ON LT.
 21" X 15" X 30' ARCH PIPE CULVERT
 CONSTRUCT APPROACH
 = 35 CU. YDS. COMP. EMBANKMENT
 = 1418 CU. YDS. UNCL. EXCAVATION
 = 151.2 TONS AGGR. BASE COURSE

FIELD ENTRANCE
 STA. 117+70 LT.

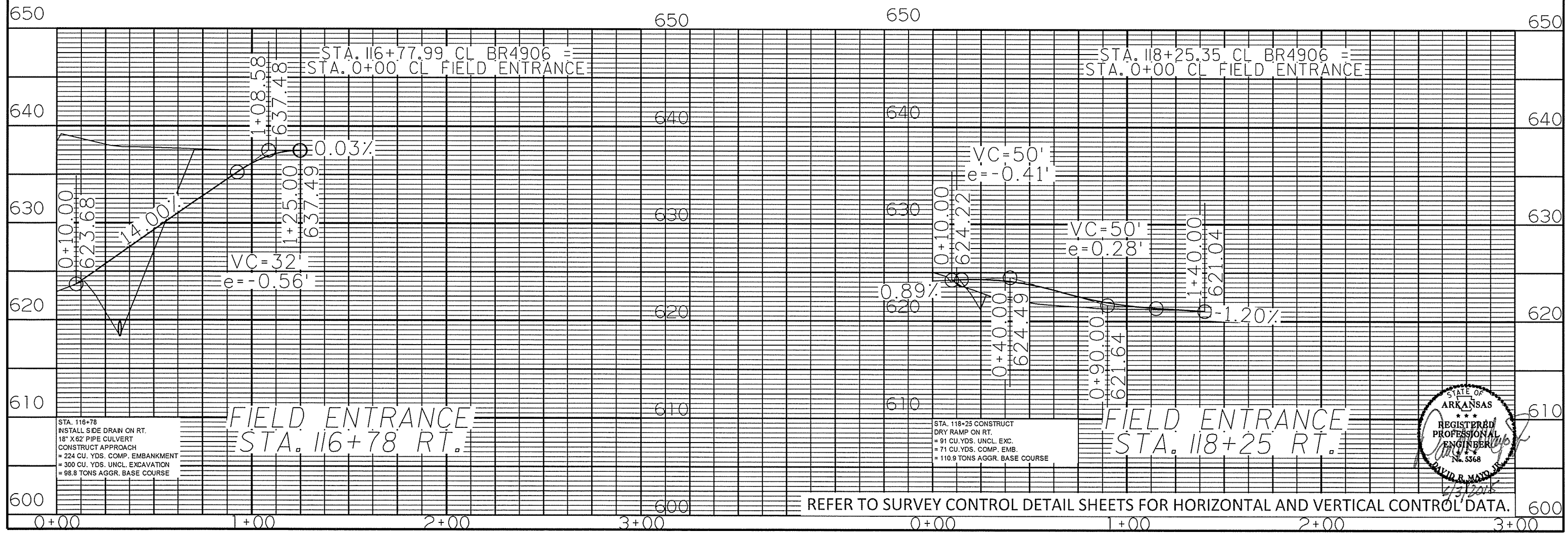
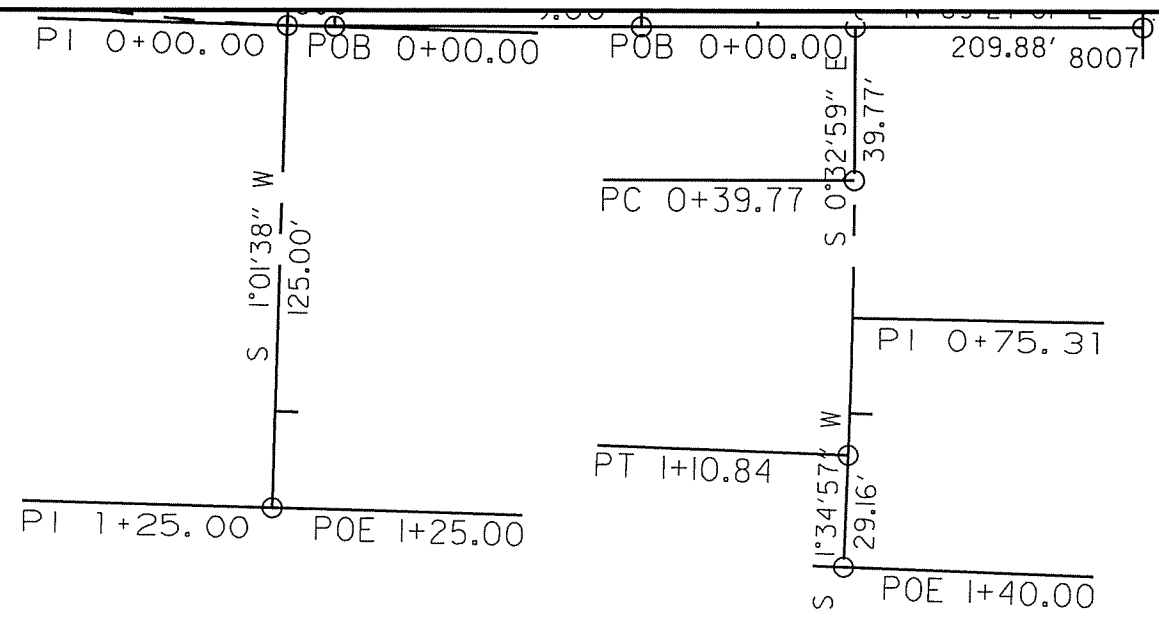


REFER TO SURVEY CONTROL DETAIL SHEETS FOR HORIZONTAL AND VERTICAL CONTROL DATA.

DATE REVISED	DATE FILED	DATE REVISED	DATE FILED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		22	85

PLAN AND PROFILE

PI 115+54.55

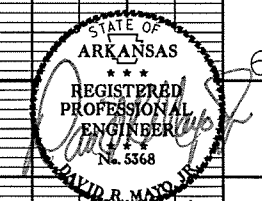


STA. 116+78
 INSTALL SIDE DRAIN ON RT.
 18" X 62' PIPE CULVERT
 CONSTRUCT APPROACH
 = 224 CU. YDS. COMP. EMBANKMENT
 = 300 CU. YDS. UNCL. EXCAVATION
 = 98.8 TONS AGGR. BASE COURSE

FIELD ENTRANCE
 STA. 116+78 RT.

STA. 118+25 CONSTRUCT
 DRY RAMP ON RT.
 = 91 CU. YDS. UNCL. EXC.
 = 71 CU. YDS. COMP. EMB.
 = 110.9 TONS AGGR. BASE COURSE

FIELD ENTRANCE
 STA. 118+25 RT.

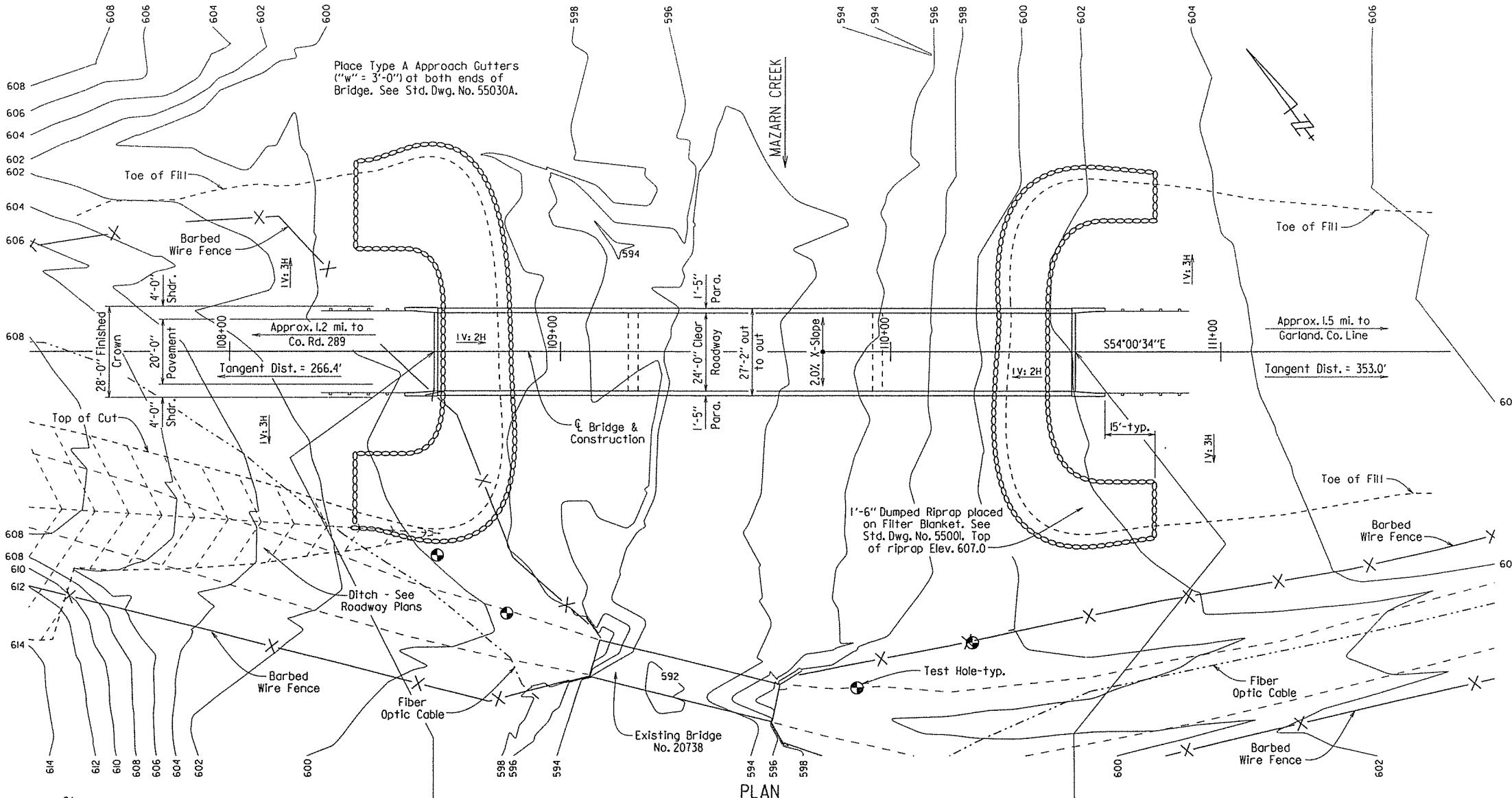


REFER TO SURVEY CONTROL DETAIL SHEETS FOR HORIZONTAL AND VERTICAL CONTROL DATA.

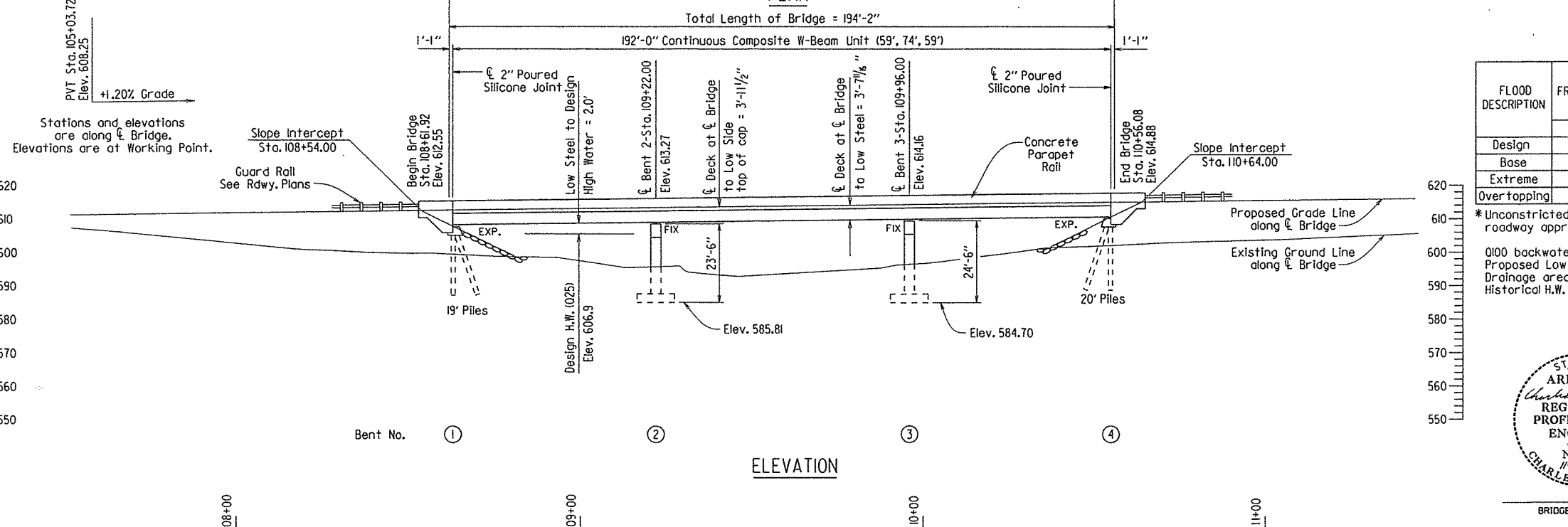
For R/W Data, see Rdwy. Plans

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		23	85
				JOB NO.		BR4906	23	85

GENERAL NOTES
 BENCH MARK: Vertical Control Data are shown in the Survey Control Data Sheets.
 CONSTRUCTION SPECIFICATION: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction (2014 edition) with applicable Supplemental Specifications and Special Provisions. Unless otherwise noted, Section and Subsection numbers in the plans refer to the Standard Construction Specifications.
 DESIGN SPECIFICATIONS: AASHTO LRFD Bridge Design Specifications (6th Edition, 2012) with Interim Specifications.
 LIVE LOADING: HL93 SEISMIC ZONE: I
 MATERIALS AND STRENGTHS:
 Class (SAC) Concrete (superstructure) f'c = 4,000 psi
 Class S Concrete (substructure) f'c = 3,500 psi
 Reinforcing Steel (Grade 60, AASHTO M31 or M322, Type A) fy = 60,000 psi
 Structural Steel (AASHTO M270, Gr. 36) Fy = 36,000 psi
 Structural Steel (AASHTO M270, Gr. 50W) Fy = 50,000 psi
 BORING LOGS: Boring logs may be obtained from the Construction Contract Procurement Section of the Program Management Division.
 STEEL PILING: Piling in Bents 1 & 4 shall be HP 12x53 and shall be driven with an approved air, steam or diesel hammer to a minimum safe bearing capacity of 60 tons per pile, and into material designated as Hard Shale on the boring legend. Lengths of piling shown are for estimating quantities and for use in determining payment for cut-off and build-up in accordance with Section 805. On all piles the Contractor shall use approved steel H-Pile driving points. Actual pile lengths to be determined in the field. Piling shall be driven after embankment to bottom of cap is in place.
 FOOTINGS: Intermediate bent footings shall be set a minimum of 2'-6" into material designated on the boring legend as Hard Shale. The top of the footings shall be set at or below the channel bottom. Foundations for footings shall be prepared in accordance with Subsection 801.04. Rock excavations shall be made to neat lines of the concrete footings. Care shall be exercised to avoid shattering of the rock faces by excessive blasting. Concrete in footings shall be poured directly against excavated surfaces of rock. Excavations shall be backfilled and compacted to the level of surrounding ground in accordance with Subsection 801.08.
 BRIDGE DECK: The concrete bridge deck shall be given a fine finish as specified for final finishing in Subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish.
 DETAIL DRAWINGS:
 End Bents 56454
 Intermediate Bents 56455
 192' Continuous Composite W-Beam Unit 56456-56461
 Elastomeric Bearings 56462
 Steel Piling 55020
 Type A Approach Gutter 55030A
 EXISTING BRIDGE: The existing bridge No. 20738 is 12' wide and 58' long and consists of 3 concrete spans approx. 19' long with embedded railroad rails supported by concrete piers on concrete footings.
 REMOVAL AND SALVAGE: After the new bridge is opened to traffic, the existing bridge No. 20738 shall be removed in accordance with Section 205. All material from the existing bridge shall become the property of the contractor. The existing bridge is located approx. 100' downstream from the new bridge.
 MAINTENANCE OF TRAFFIC: For maintenance of traffic, see Roadway Plans.



PLAN



ELEVATION

HYDRAULIC DATA

FLOOD DESCRIPTION	FREQUENCY	DISCHARGE	*NATURAL WATER SURFACE ELEVATION	WATER SURFACE ELEV. WITH BACKWATER
	YEARS		CFS	FEET
Design	25	11600	606.3	606.9
Base	100	16400	607.9	608.9
Extreme	500	22600	609.8	611.5
Overtopping	90	16220	607.9	608.8

*Unconstricted water surface without structure or roadway approaches.
 0100 backwater elevation for existing structure = 608.1 ft.
 Proposed Low Bridge Chord elevation = 608.92 ft.
 Drainage area = 30.5 square miles.
 Historical H.W. Elev. = 607.0 ft.

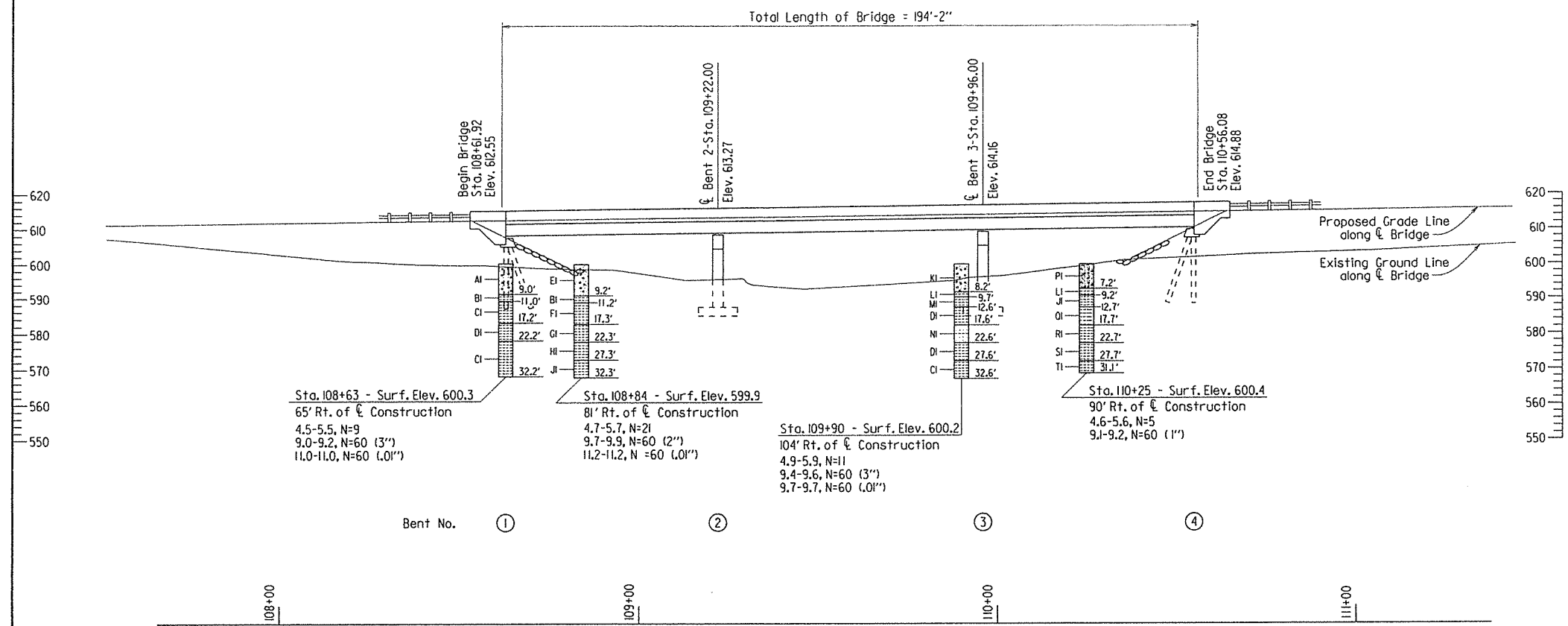


SHEET 1 OF 2
 LAYOUT OF BRIDGE OVER
 MAZARN CREEK
 MAZARN CREEK STR. & APPRS. (S)
 MONTGOMERY COUNTY
 COUNTY ROAD 34
 ARKANSAS STATE HIGHWAY COMMISSION
 LITTLE ROCK, ARK.
 DRAWN BY: KDH DATE: 3-17-14 FILENAME: bbr4906_ll.dgn
 CHECKED BY: ADW DATE: 10-20-14 SCALE: 1" = 20'
 DESIGNED BY: CSL DATE: 4/14
 BRIDGE NO. 04930 DRAWING NO. 56452

PRINT DATE: 11/14/2014

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BR4906	24	85

① 04930 - LAYOUT - 56453



ELEVATION

BORING LEGEND

- AI-Moist, Loose, Brown Sand with Gravel(Sandstone Fragments)
- BI-SHALE - Dark Gray, Weathered, Medium Hard
- CI-SHALE - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Calcite Partings, Fractured Layers and Slickensides
- DI-SHALE - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Calcite Partings, and Slickensides
- EI-Moist, Medium Dense, Brown Sand with Gravel(Sandstone Fragments)
- FI-SHALE WITH OCCASIONAL WEATHERED SHALE AND SANDSTONE LAYERS - Dark Gray, Laminated, Hard, with Steep Dip, Fractured Layers and Slickensides
- GI-SHALE WITH OCCASIONAL SANDSTONE LAYERS - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Calcite Partings, Fractured Layers and Slickensides
- HI-SHALE - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Calcite Seams, and Slickensides
- JI-SHALE - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Calcite Seams, Fractured Layers and Slickensides
- KI-Moist, Medium Dense, Brown Sand with Gravel(Sandstone Fragments)
- LI-SHALE - Gray, Weathered, Medium Hard
- MI-SHALE - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Fractured Layers, and Slickensides
- NI-SANDSTONE WITH FREQUENT SHALE LAYERS AND SEAMS - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Calcite and Quartz Seams, Fractured Layers and Slickensides
- PI-Moist, Loose, Brown Sand with Clay and Gravel(Sandstone Fragments)
- QI-SHALE WITH A SANDSTONE LAYER - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Calcite and Quartz Seams and Layers, Fractured Layers, Trace of Pyrite and Slickensides
- RI-SHALE - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Calcite and Quartz Layers, Fractured Layers, Trace of Pyrite and Slickensides
- SI-SHALE - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Calcite Seams, Fractured Layers, Trace of Pyrite and Slickensides
- TI-SHALE - Dark Gray, Laminated, Slightly Weathered, Hard, with Steep Dip, Calcite Partings, Fractured Layers, Trace of Pyrite and Slickensides



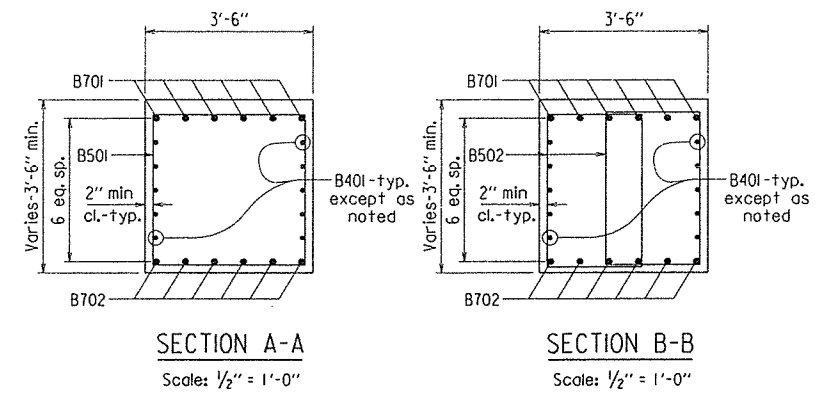
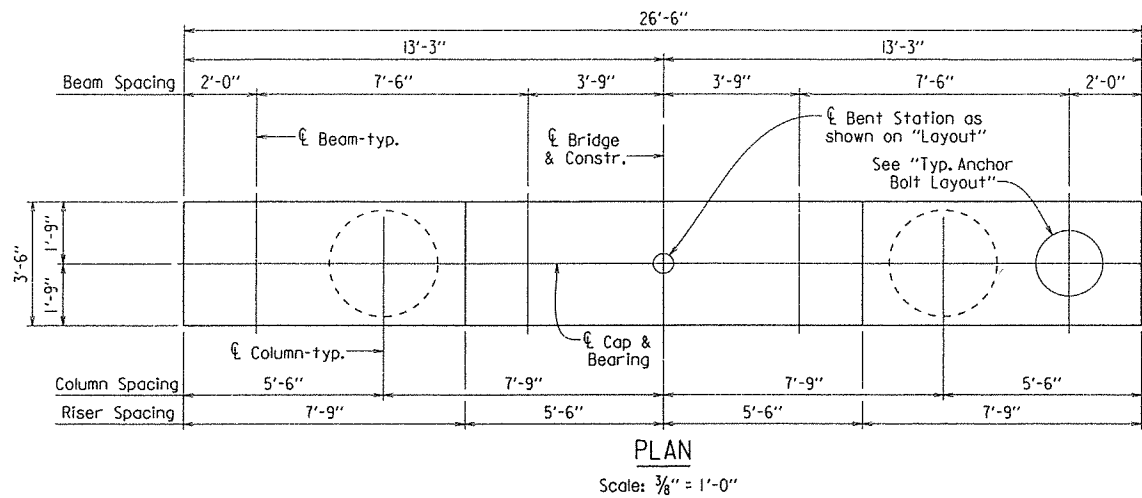
SHEET 2 OF 2
 LAYOUT OF BRIDGE OVER
 MAZARN CREEK
 MAZARN CREEK STR. & APPRS. (S)
 MONTGOMERY COUNTY

COUNTY ROAD 34
 ARKANSAS STATE HIGHWAY COMMISSION
 LITTLE ROCK, ARK.

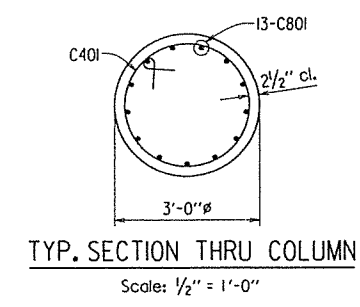
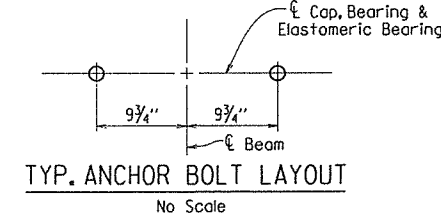
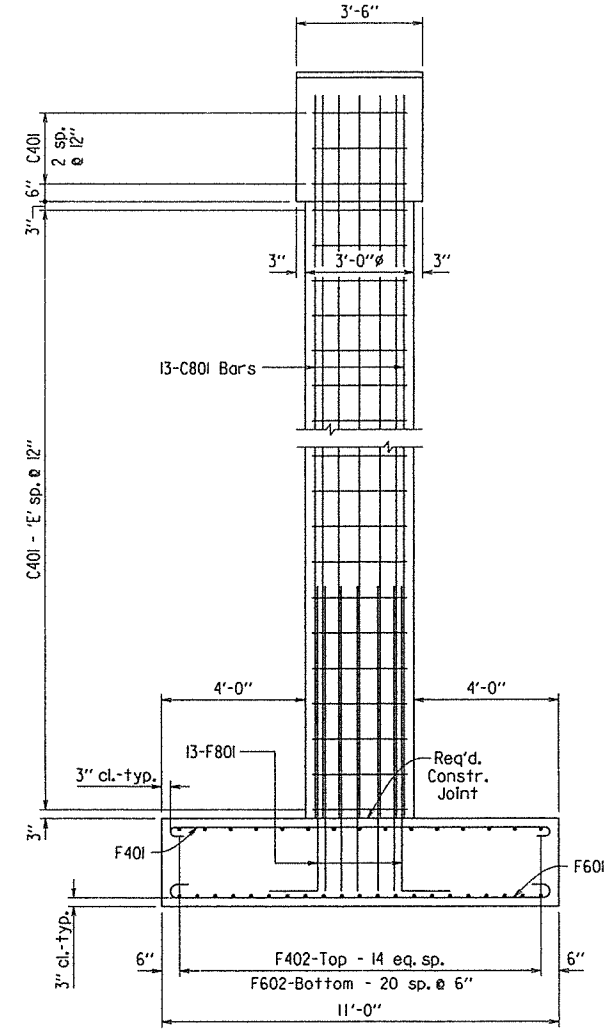
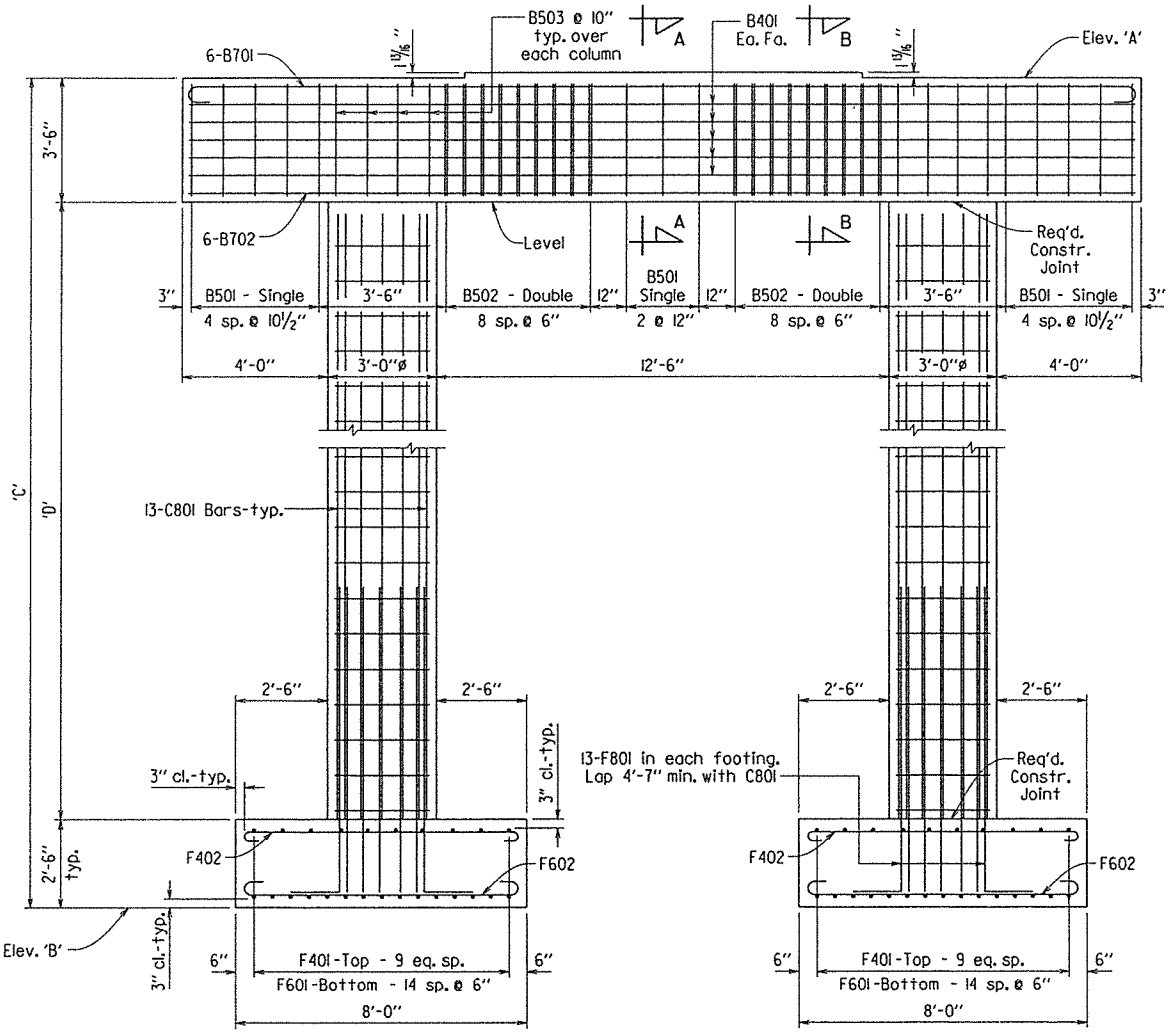
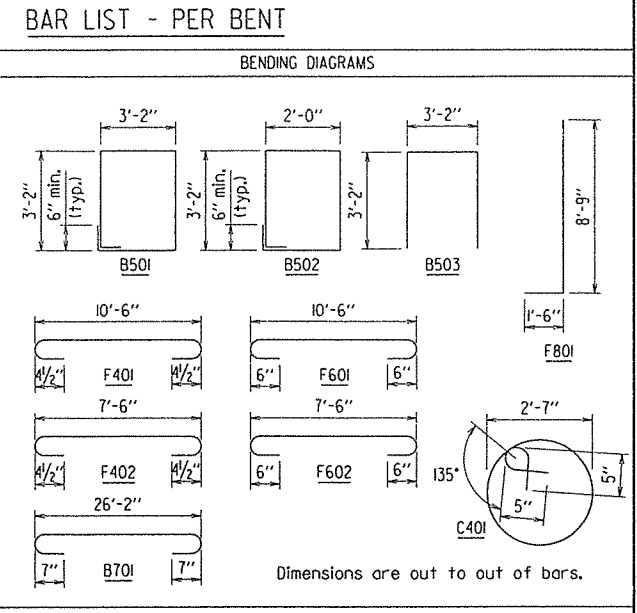
DRAWN BY: KDH DATE: 3-17-14 FILENAME: bbr4906.ll.dgn
 CHECKED BY: ADW DATE: 12-22-14 SCALE: 1" = 20'
 DESIGNED BY: CSR DATE: 4/14
 BRIDGE NO. 04930 DRAWING NO. 56453

PRINT DATE: 11/14/2014

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		26	85
				04930 - INT. BENTS		- 56455		



MARK	NO. REQ'D.	LENGTH	P.D.
B401	10	26'-2"	Str.
C401	4	9'-3"	3"
F401	20	11'-6"	3"
F402	30	8'-6"	3"
B501	13	13'-2"	2 1/2"
B502	36	10'-10"	2 1/2"
B503	8	9'-4"	2 1/2"
F601	30	11'-10"	4 1/2"
F602	42	8'-10"	4 1/2"
B701	6	27'-10"	5 1/4"
B702	6	26'-2"	Str.
C801	26	0'	Str.
F801	26	10'-1"	6"



GENERAL NOTES

All concrete shall be Class S with a minimum 28 day compressive strength of f'c = 3500 psi. Concrete shall be poured in the dry and all exposed corners to be chamfered 3/4" unless otherwise noted.

All reinforcing steel shall be Grade 60 (yield strength = 60,000 psi) conforming to AASHTO M31 or M322, Type A, with mill test reports.

Reinforcing bars in top of cap shall be properly placed to avoid interference with anchor bolts or sheet metal sleeves.

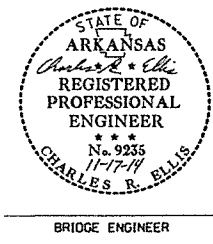
For additional information, see Layout.

ELEVATION
Scale: 3/8" = 1'-0"

END VIEW
Scale: 3/8" = 1'-0"

TABLE OF VARIABLES

BENT	Elev. 'A'	Elev. 'B'	'C'	'D'	'E'	'F'	'G'
2	609.31	585.81	23'-6"	17'-6"	17	42	20'-6"
3	609.20	584.70	24'-6"	18'-6"	18	44	21'-6"



DETAILS OF INTERMEDIATE BENTS
MAZARN CREEK

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

BRIDGE NO. 04930 DRAWING NO. 56455

DATE: 10-9-14
CHECKED BY: CSR
DESIGNED BY: ADW
DATE: 11/17/14
SCALE: AS NOTED

PRINT DATE: 11/17/2014

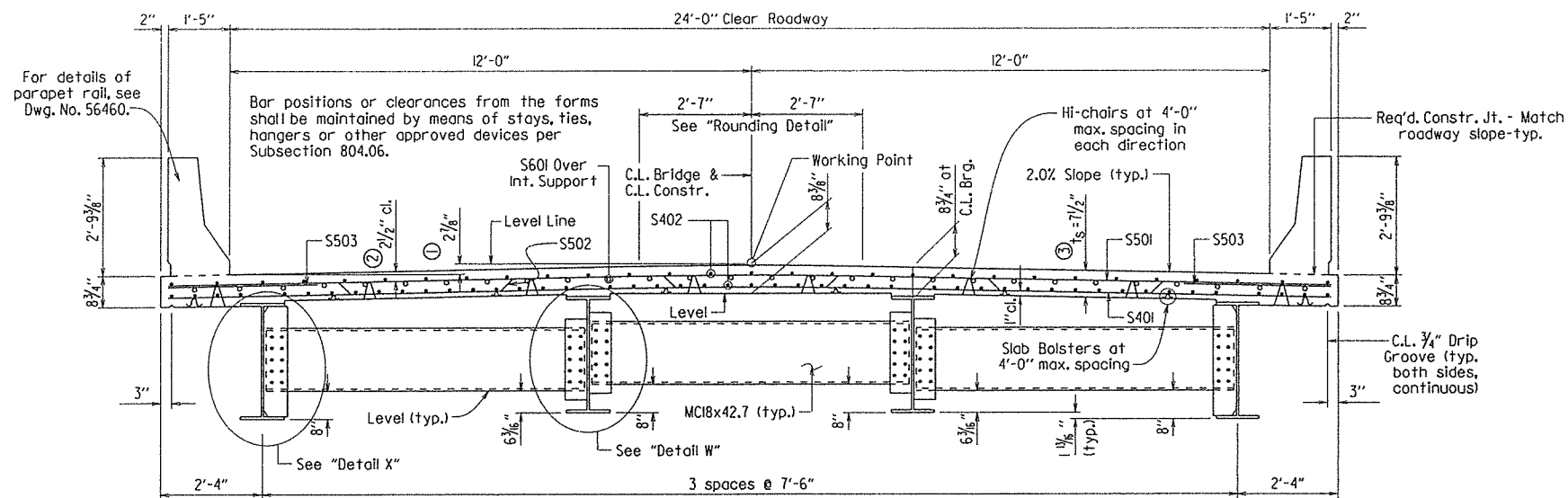
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
JOB NO. BR4906							27	85
① 04930 - W-BEAM UNIT							- 56456	

SLAB REINFORCING
 Transverse: S501-Top; S401-Bottom @ 12" o.c. — Alternate
 S502 @ 12" o.c. bent up over beams
 S503 bundled with #5 bars in top at both gutterlines
 Longitudinal: S402 as shown
 S601 as shown centered over int. supports

Note: Class I Protective Surface Treatment shall be applied to the Roadway Surface and the Face and Top of Concrete Parapet Rail.

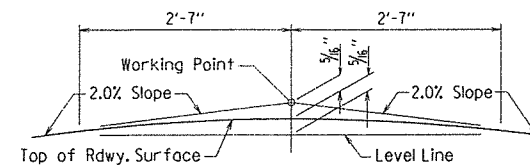
NOTE: At the Contractor's option, two straight #5 bars may be substituted for bar S502. Payment for reinforcing will be based on the weight of bar S502.

- Working point to gutterline.
- Tolerance: Minus = $1/4"$; Plus equal to the amount of slab thickening used to meet slab thickness tolerance. See "Adjustment for Slab Thickness Tolerance".
- See "Adjustment for Slab Thickness Tolerance".



TYPICAL ROADWAY SECTION

Scale: $1/2" = 1'-0"$

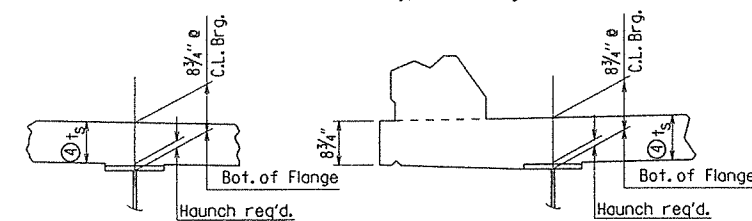


NOTE: Working Point matches Theoretical Roadway Grade.

ROUNDING DETAIL

No Scale

ts = slab thickness as shown in "Typical Roadway Section"



INTERIOR BEAM

EXTERIOR BEAM

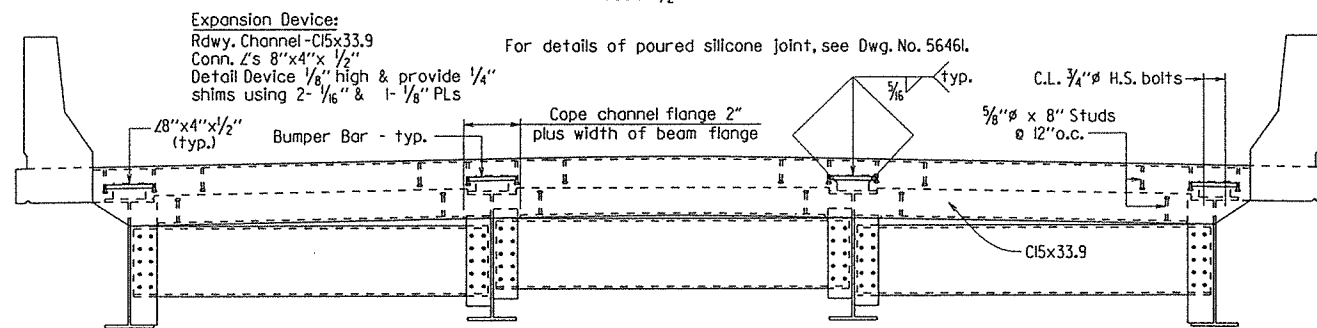
- Tolerance when removable deck forming is used is $+ 1/2"$; $- 1/4"$. Haunch forming is required and shall be adjusted to maintain slab thickness tolerance.

ADJUSTMENT FOR SLAB THICKNESS TOLERANCE

No Scale

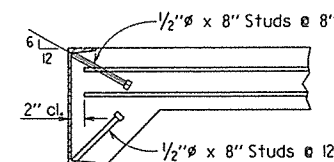
NOTES:
 Haunch dimension may vary within the following limits to maintain the grade and slab thickness tolerance: Minimum occurs when top flange contacts bottom reinforcing steel; Maximum = top flange thickness plus $1/4"$. No increase in concrete and structural steel quantities will be made to maintain tolerances.

Tolerances shown are applicable only when removable deck forming is used. See Std. Dwg. No. 55005 for tolerances when permanent steel deck forms are used. Payment for concrete shall be based on removable deck forming.



TYPICAL SECTION THRU JOINT

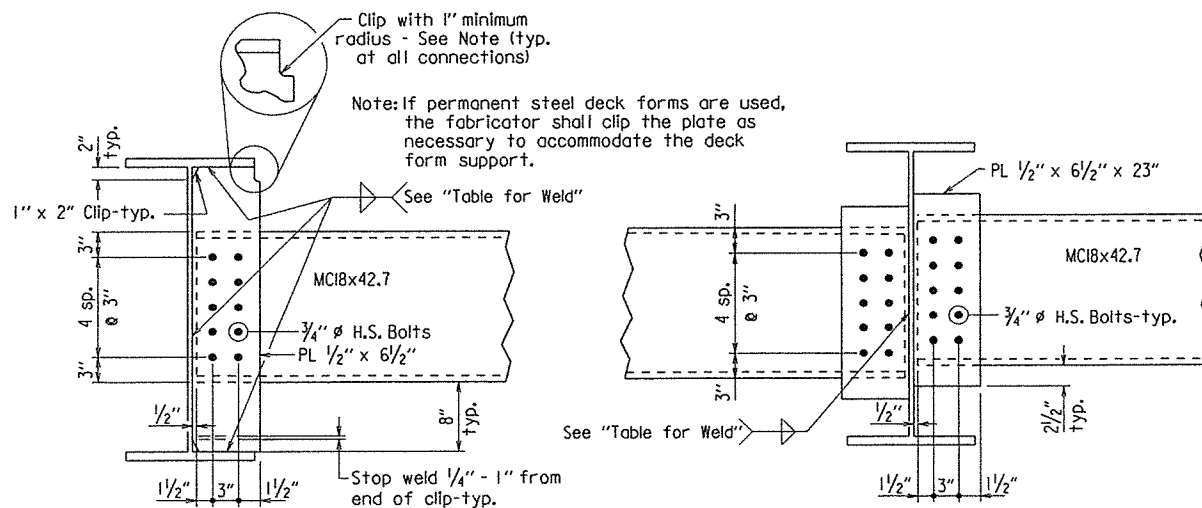
Scale: $1/2" = 1'-0"$



Note: As an alternate to $5/8"$ studs, $1/2"$ x 8" studs spaced as shown may be used. Use weight of $5/8"$ stud as basis of measurement of structural steel in anchors.

DETAILS OF ALTERNATE ANCHORS

No Scale



DETAIL X

No Scale

DETAIL W

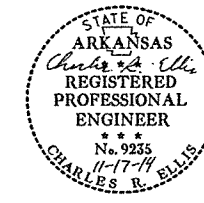
No Scale

Bolts in diaphragm connections shall be properly installed and tightened in accordance with Subsection 807.7L.

TABLE FOR WELD

Material Thickness of Thicker Part Joined (Inches)	Minimum Size of Fillet Weld (Inches)	Single Pass Weld Must Be Used
To $3/4"$ Inclusive	$1/4"$	Used
Over $3/4"$	$5/16"$	

NOTE: When a fillet weld size, as shown on the plans, is larger than the minimum, the first pass shall be that specified for minimum size of fillet weld.



BRIDGE ENGINEER

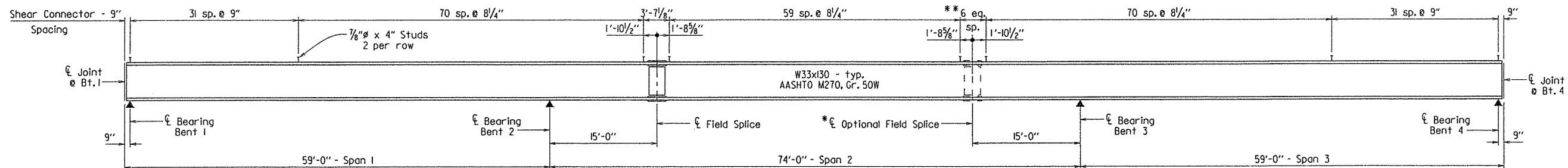
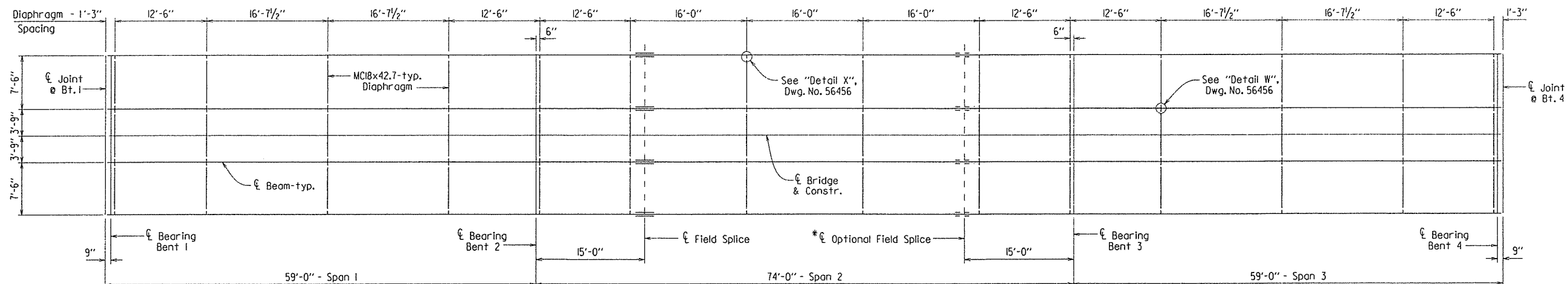
SHEET 1 OF 5
 DETAILS OF 192'-0" CONTINUOUS
 COMPOSITE W-BEAM UNIT
 MAZARN CREEK

ROUTE 309
 ARKANSAS STATE HIGHWAY COMMISSION
 LITTLE ROCK, ARK.

DRAWN BY: KDH DATE: 9-8-14 FILENAME: bbr4906_sl.dgn
 CHECKED BY: CSR DATE: 11/14/14 SCALE: AS NOTED
 DESIGNED BY: ADK DATE: 7-1-14
 BRIDGE NO. 04930 DRAWING NO. 56456

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BR4906	28	85

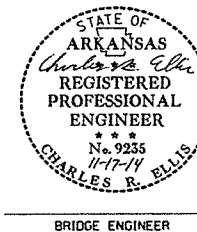
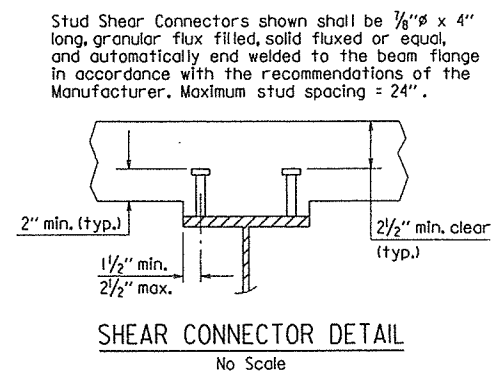
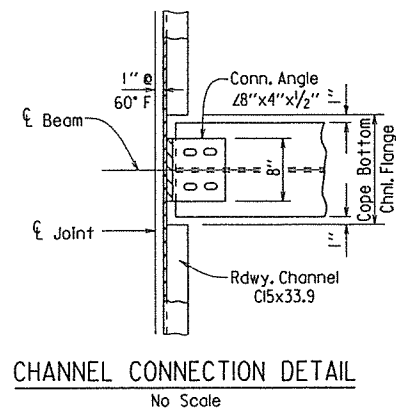
① 04930 - W-BEAM UNIT - 56457



Note: Bolted field splices may be eliminated or shop welded splices may be substituted with the approval of the Engineer. Payment will be made on the basis of plan quantities.

*At the Contractor's option, a field splice may be provided at this location. No additional payment will be made for the optional field splice.

**if the optional field splice is used eliminate the shear connectors at this location.



SHEET 2 OF 5
DETAILS OF 192'-0" CONTINUOUS
COMPOSITE W-BEAM UNIT
MAZARN CREEK

ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: KDH DATE: 9-10-14 FILENAME: bbr4906_sl.dgn
CHECKED BY: CSB DATE: 11/14/14 SCALE: AS NOTED
DESIGNED BY: ADW DATE: 7-14
BRIDGE NO. 04930 DRAWING NO. 56457

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BR4906	29	85
				① 04930 - W-BEAM UNIT		- 56458		

GENERAL NOTES

CONSTRUCTION SPECIFICATIONS: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction (2014 edition) with applicable Supplemental Specifications and Special Provisions.

DESIGN SPECIFICATION: AASHTO LRFD Bridge Design Specifications (6th Edition, 2012) with Interim Specifications.

MATERIAL AND STRENGTHS:
 Class S(AE) Concrete $f'_c = 4,000$ psi
 Reinforcing Steel (Grade 60, AASHTO M31 or M322, Type A) $f_y = 60,000$ psi
 Structural Steel (AASHTO M 270, Gr. 50W) $F_y = 50,000$ psi
 Structural Steel (AASHTO M 270, Gr. 36) $F_y = 36,000$ psi

CONCRETE:
 Concrete shall be poured in the dry and all exposed corners to be chamfered $\frac{3}{4}$ " unless otherwise noted. All concrete shall be Class S(AE) with a minimum 28-day compressive strength $f'_c = 4,000$ psi.

The superstructure details shown are for use when removable deck forming is used and are the basis for measurement of Class S(AE) Concrete. See Standard Drawing No. 55005 for allowable modifications and for tolerances when Permanent Steel Bridge Deck Forms are used. Concrete in bridge superstructure shall be placed, consolidated and screeded off for the entire pour before any concrete has taken its initial set. This may require the use of a retarding agent.

The concrete deck shall be given a fine finish in accordance with Subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish. Movement of the finishing machine across new concrete shall be on planks placed on the surface and shall be prohibited for 72 hours after finishing the pour. Sufficient concrete must be placed ahead of the strike-off to fully load the beam. If a longitudinal strike-off is used, a vertical camber adjustment must be made in the strike-off to account for the future dead load deflection due to the railing. A minimum of 72 hours shall elapse between completion of the slab and the pouring of the parapet railing. Any railing pours made before the entire slab has been placed and cured must be approved by the Engineer.

REINFORCING STEEL:
 All reinforcing steel shall be Grade 60 (yield strength = 60,000 psi) conforming to AASHTO M31 or M322, Type A, with mill test reports. The reinforcing steel is to be accurately located in the forms and firmly held in place by steel wire supports, sufficient in number and size to prevent displacement during the course of construction. The wire supports will not be paid for directly, but will be considered subsidiary to the item "Reinforcing Steel-Bridge (Grade 60)".

STRUCTURAL STEEL:
 All Structural steel shall be AASHTO M 270, Grade 50W unless otherwise noted and shall be paid for as "Structural Steel in Beam Spans (M 270, Gr. 50W)". Grade 50W steel shall not be painted. All exposed surfaces shall be cleaned in accordance with Subsection 807.84(e) unless otherwise noted. Structural steel completely embedded in concrete may be AASHTO M 270, Gr. 36 or Gr. 50 unless otherwise noted.

Drawings show general features of design only. Shop drawings shall be made in accordance with Subsection 807.04, submitted and approval secured before fabrication is begun.

Requests for substitution of structural steel shapes shown with shapes of greater size must be submitted by the Contractor to the Engineer for approval. Steels of equal or greater strengths will be accepted only when shown on the approved shop drawings. Payment will be based on the basis of shapes and materials shown in the plans, and no additional compensation will be made for any adjustments due to substitutions.

Beams and field splice plates are considered main load carrying members and shall meet the Longitudinal Charpy V-Notch Test specified in Subsection 807.05. This work and material will not be paid for directly, but shall be considered subsidiary to the item "Structural Steel in Beam Spans (M270, Gr. 50W)".

All beams shall be blocked in their true position in the shop with the webs horizontal in groups as specified in Subsection 807.54(b)(2). The camber, length of sections, distance between bearings and openings of joints shall be measured with the beams in their true position and this information shall become part of the permanent records for this job. The component parts shall be match marked in this assembly and these marks shall be shown on the erection diagram. All beam dimensions are based on a temperature of 60 degrees F. A tolerance of $\frac{1}{4}$ " +/- is allowed for camber.

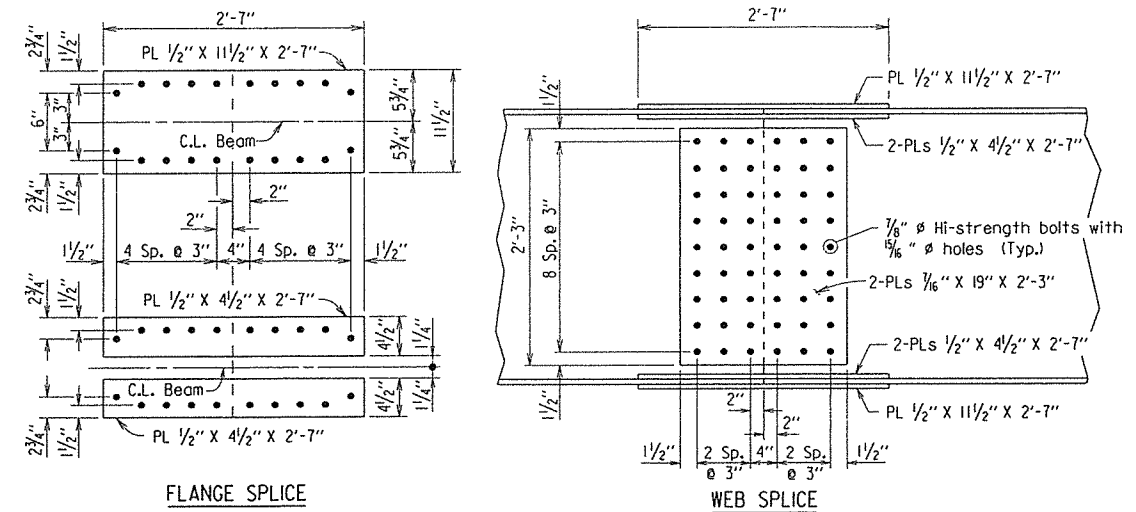
Flange field splice plates shall be cut and fabricated so that the primary direction of ralling is parallel to the direction of the main tensile and/or compressive stresses.

All welding that is to be done during fabrication of structural steel, including temporary welds, shall be detailed on the shop drawings and submitted for approval. If additional welds are required, whether permanent or temporary, a formal request with detailed drawings shall be submitted to the Engineer for approval; however, additional welds used for attaching false work support devices or screed rail supports to the structural steel that do not exceed the limitations of Subsection 802.13 will not require approval prior to construction. All welding shall conform to Subsection 807.26.

Field connections shall be bolted with high-strength bolts and shall be $\frac{3}{4}$ " ϕ bolts unless otherwise noted. Open Holes shall be $\frac{1}{8}$ " ϕ unless otherwise noted. Holes for $\frac{3}{4}$ " ϕ high-strength bolts may be $\frac{1}{8}$ " ϕ if a washer is supplied for use under both the nut and head of the bolt. Bolts shall be placed with heads on the outside face of the exterior beam webs and on the bottom of the beam flanges.

Unless otherwise noted, steel diaphragms shall be installed as beams are erected. All bolts in diaphragms and field splices shall be installed and tightened in accordance with Subsection 807.71 prior to pouring the concrete deck.

All stud shear connectors shall be granular flux filled, solid fluxed, or equal and shall be automatically end welded in accordance with the recommendations of the manufacturer.



All Field Splice plates to be AASHTO M270, Gr. 50W steel.

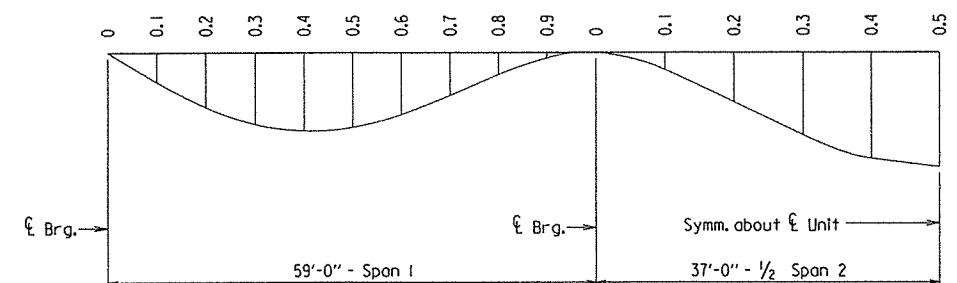
DETAILS OF FIELD SPLICE

No Scale

TABLE OF DEAD LOAD DEFLECTIONS (INCHES)

Point of Deflection	Structural Steel		Structural Steel + Slab		Structural Steel + Slab + Parapet	
	Interior	Exterior	Interior	Exterior	Interior	Exterior
0	0	0	0	0	0	0
0.1	0.031	0.028	0.174	0.147	0.193	0.167
0.2	0.057	0.052	0.320	0.271	0.355	0.308
0.3	0.074	0.068	0.417	0.354	0.463	0.403
0.4	0.081	0.075	0.456	0.388	0.506	0.441
0.5	0.077	0.071	0.435	0.370	0.483	0.420
0.6	0.064	0.059	0.361	0.307	0.401	0.349
0.7	0.045	0.041	0.252	0.213	0.280	0.242
0.8	0.023	0.021	0.130	0.110	0.144	0.125
0.9	0.006	0.005	0.032	0.027	0.035	0.031
0	0	0	0	0	0	0
0.1	0.018	0.016	0.101	0.085	0.112	0.097
0.2	0.051	0.047	0.288	0.245	0.320	0.279
0.3	0.085	0.079	0.481	0.409	0.534	0.465
0.4	0.110	0.101	0.620	0.526	0.688	0.598
0.5	0.119	0.110	0.671	0.569	0.744	0.647

Table is symm. about ϵ Unit

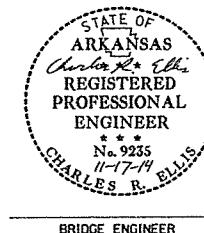


DEAD LOAD DEFLECTIONS DIAGRAM

Note:
 Camber for Dead Load Deflection plus Vertical curve $\pm \frac{1}{4}$ " tolerance. Deflections shown are from a chord from ϵ Bearing to ϵ Bearing. Vertical curve corrections not included. Negative sign (-) indicates point above chord.

SHEET 3 OF 5
 DETAILS OF 192'-0" CONTINUOUS
 COMPOSITE W-BEAM UNIT
 MAZARN CREEK

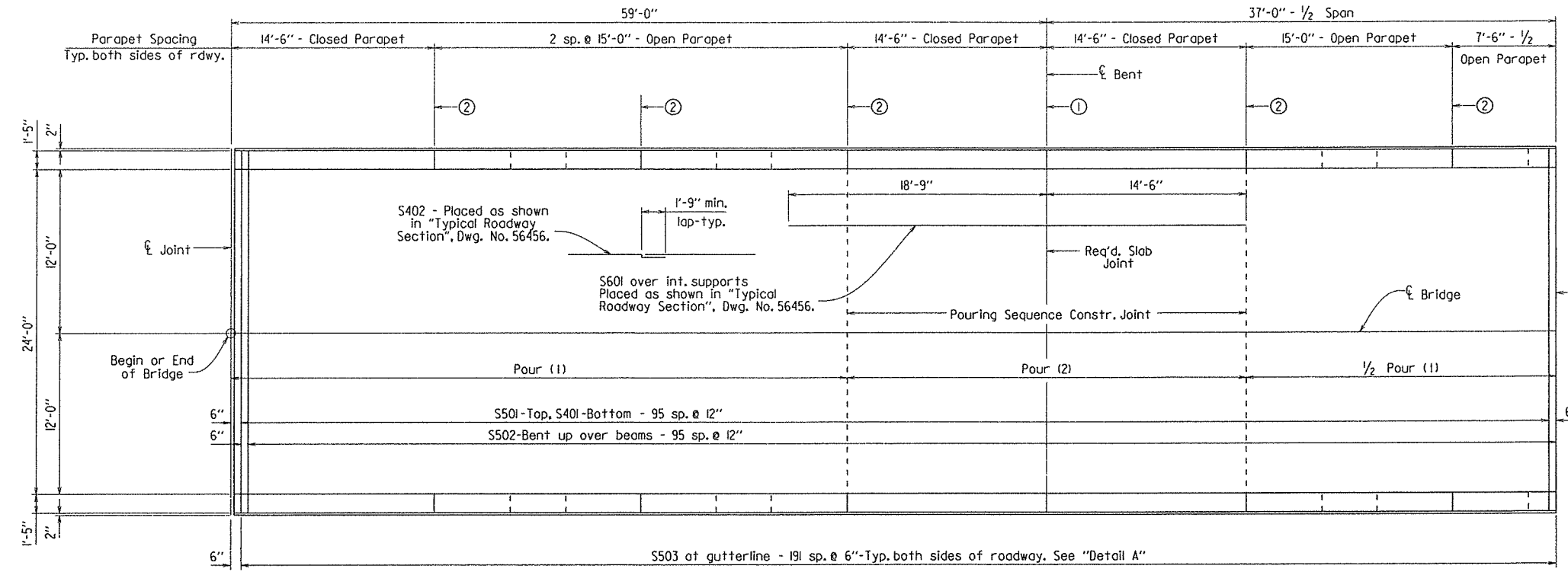
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 ARKANSAS STATE HIGHWAY COMMISSION
 LITTLE ROCK, ARK.
 DRAWN BY: KDH DATE: 9-10-14 FILENAME: bbr4906_sl.dgn
 CHECKED BY: CSE DATE: 11/14/14 SCALE: AS NOTED
 DESIGNED BY: ADW DATE: 7-14
 BRIDGE NO. 04930 DRAWING NO. 56458



PRINT DATE: 11/14/2014

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BR4906	30	85
				① 04930 - W-BEAM UNIT				56459

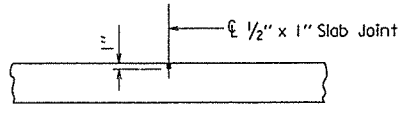
- ① Full-Depth Parapet Joint (1/4" to 1" max.) Stop 4" from top of slab.
- ② Partial-Depth Parapet Joint (1/4" to 1" max.) Stop 1'-2" from top of slab.



Notes:
Required slab joints and pouring sequence construction joints shall align with parapet joints at the gutterline.

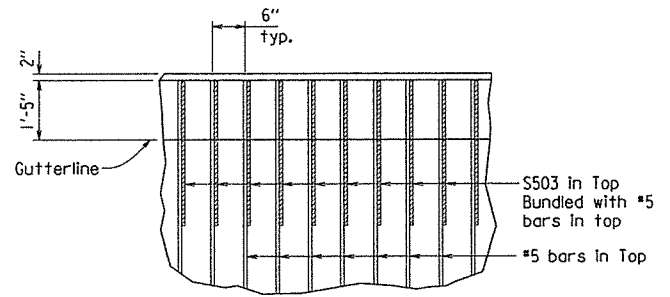
Pours with the same number may be placed simultaneously or separately. All Pours (1) must be placed before Pours (2) can be placed. 48 hours shall elapse between the end of a pour and the start of the next pour. 72 hours shall elapse between the end of a pour and the start of an adjacent pour. Any railing pours made before the entire slab unit has been placed must be approved by the Engineer. The Contractor must obtain approval from the Engineer for any deviation from the pouring sequence shown.

HALF-REINFORCING PLAN
Scale: 3/16" = 1'-0"

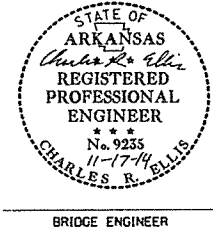


SLAB JOINT DETAIL
No Scale

Use Type 3 or 4 Joint Sealer. See Subsections 501.02(h) and 501.05(j). Backer Rod filler will not be required. Joint Sealer shall be measured and paid for as Class (SAE) Concrete-Bridge. Slab joints shall extend to the outside edge of the deck slab. Slab joints shall be installed before the parapet railing is poured. If slab joints are to be sawed, they shall be sawed as soon as the concrete has sufficiently set to allow sawing of the joint without damage to the slab. Slab joints shall be placed at all pouring sequence construction joints and required slab joint locations. The joint sealer shall extend across the deck slab, gutterline to gutterline for joints that align with parapet joints.



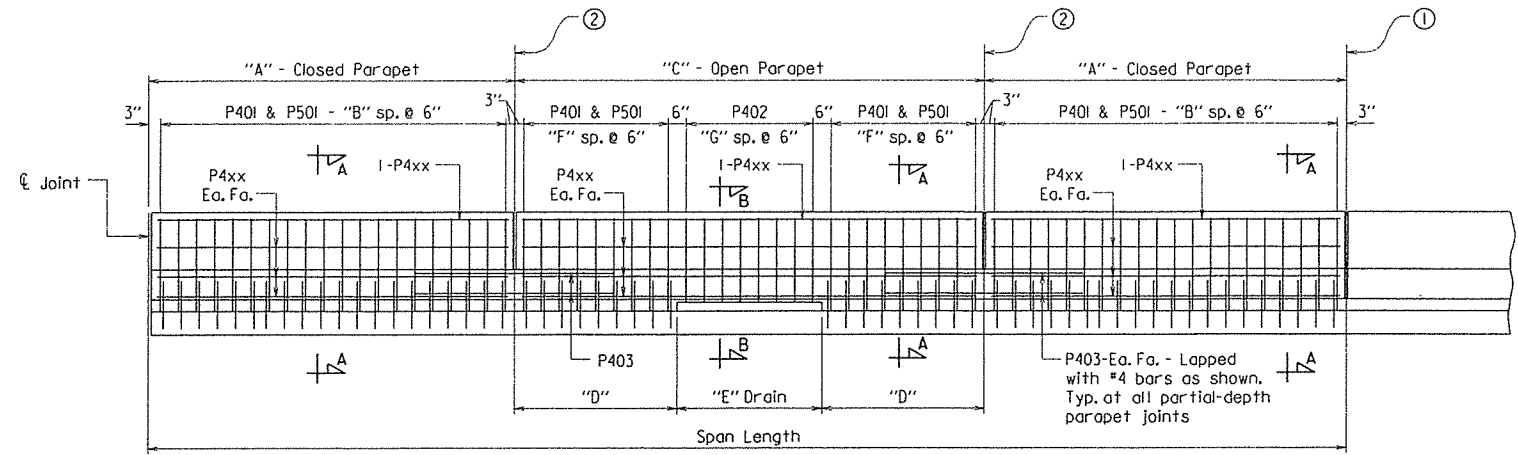
DETAIL A
No Scale



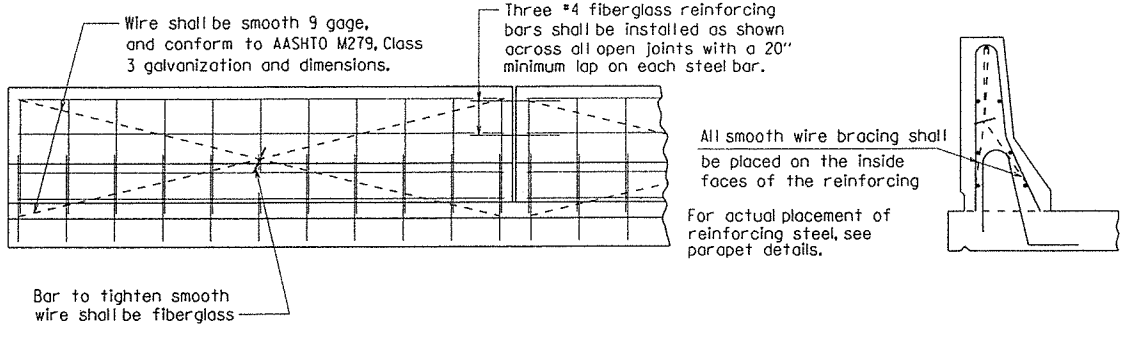
SHEET 4 OF 5
DETAILS OF 192'-0" CONTINUOUS COMPOSITE W-BEAM UNIT
MAZARN CREEK
ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.
DRAWN BY: KDH DATE: 9-10-14 FILENAME: bbr4906.sl.dgn
CHECKED BY: CSE DATE: 11/14/14 SCALE: AS NOTED
DESIGNED BY: A.D.N. DATE: 7/14
BRIDGE NO. 04930 DRAWING NO. 56459

PRINT DATE: 11/14/2014

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
JOB NO. BR4906							31	85
① 04930 - W-BEAM UNIT							- 56460	



① Full-Depth Parapet Joint (1/4" to 1" max.) as shown in "Half-Reinforcing Plan", Dwg. No. 56459. Stop 4" from top of slab.
DETAILS OF PARAPET RAIL
 Scale: 3/8" = 1'-0"
 ② Partial-Depth Parapet Joint (1/4" to 1" max.) as shown in "Half-Reinforcing Plan", Dwg. No. 56459. Stop 1'-2" from top of slab.



All panels shall be braced as required to prevent racking. All open joints shall be sawed as soon as practical to a minimum width of 1/4". To control cracking before sawing all joints must be grooved before the concrete is set. Sawing of the joints must be controlled so it will follow the grooved joint.
 The extruded parapet shall conform to the horizontal and vertical lines shown on the plans or as directed by the Engineer and shall present a smooth, uniform appearance and texture. Unless otherwise noted, exposed surfaces may be given a light brush finish or a Class 3, Textured Coating Finish, in place of the Class 2, Rubbed Finish.

DETAILS OF OPTIONAL SLIPFORMING OF CONCRETE PARAPET RAIL
 No Scale

TABLE OF PARAPET RAIL VARIABLES

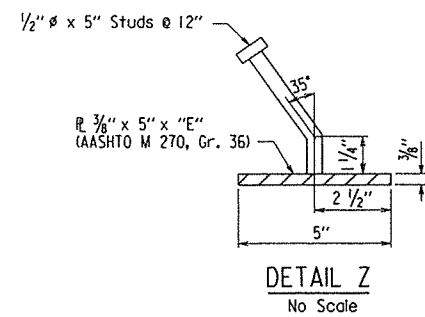
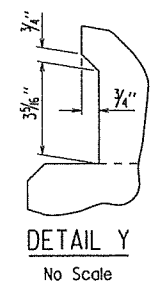
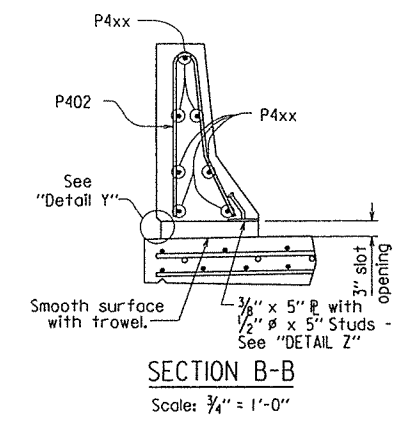
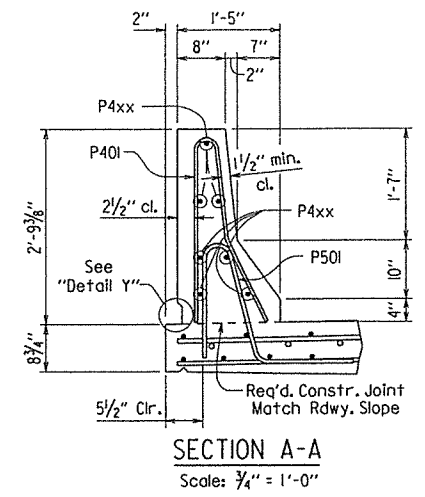
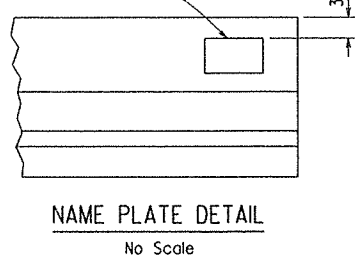
"A" Closed Parapet	"B"	P4xx Bar	"C" Open Parapet	"D"	"E"	"F"	"G"	P4xx Bar
14'-6"	28	P404	15'-0"	5'-6"	4'-0"	10	7	P405

Note: For location of Open and Closed Parapet panels, see "Half-Reinforcing Plan", Dwg. No. 56459.

BAR LIST

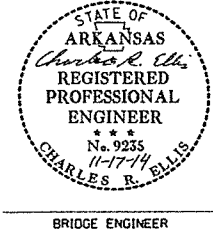
MARK	NO. REQ'D.	LENGTH	P.D.	BENDING DIAGRAMS
S401	192	26'-10"	Str.	<p>Dimensions are out to out of bars. 3" p.d. 2'-7 1/2" 12 3/4" P401 2'-3 1/2" 11" P402 3 3/4" p.d. 1'-8" 1'-9" P501 4'-0 1/2" 3'-9" 3'-9" 1'-10 1/2" 4" 2" 3 1/2" min. Symm. about C S502 ③ 1/2" Over tolerance No Under tolerance</p>
S402	380	39'-9"	Str.	
P401	656	5'-6"	3"	
P402	112	4'-10"	3"	
P403	80	3'-10"	Str.	
P404	84	14'-2"	Str.	
P405	98	14'-8"	Str.	
S501	192	26'-10"	Str.	
S502	191	27'-3"	3"	
S503	766	4'-1"	Str.	
P501	656	4'-10"	3 3/4"	
S601	56	33'-3"	Str.	

Place Type C Bridge Name Plate on right parapet rail approx. 2'-0" from front face of backwall. (Beg. of bridge only)



Note: The surfaces of the 3/8" plates which will not be in contact with concrete shall be painted with aluminum epoxy paint in accordance with Section 638, or as approved by the Engineer. Only one coat is required and shall be applied in the fabricator's shop. Painting will not be paid for directly, but will be considered subsidiary to "Structural Steel in Beam Spans (M270, Gr. 50W)."

Parapet studs shall be 5" long, granular flux filled, solid fluxed or equal, and automatically end welded to the plate. Studs and plates shall meet the requirements of Section 807 and shall be measured and paid for as "Structural Steel in Beam Spans (M270, Gr. 50W)."



SHEET 5 OF 5
 DETAILS OF 192'-0" CONTINUOUS COMPOSITE W-BEAM UNIT
 MAZARN CREEK
 ROUTE SEC.
 ARKANSAS STATE HIGHWAY COMMISSION
 LITTLE ROCK, ARK.
 DRAWN BY: KDH DATE: 9-11-14 FILENAME: bbr4906_sl.dgn
 CHECKED BY: CBR DATE: 11/14/14 SCALE: AS NOTED
 DESIGNED BY: ADN DATE: 7-14
 BRIDGE NO. 04930 DRAWING NO. 56460

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BR4906	32	85
						① 04930 - JOINTS		56461

SILICONE JOINT DATA

Bent Number	"A" Width Perpendicular to Joint at 24 Hour Average Temperature ① of:			"B" Perpendicular to Joint at 60°F	Bumper Bar Size	"D"
	40°F	60°F	80°F			
1 & 4	2 1/8"	2"	1 7/8"	2 1/4" ±	1" x 1"	5"

① The temperature used to set the joint opening shall be the approximate average air temperature during the 24 hour period immediately before the bolts are tightened. The Engineer shall establish the temperature. Interpolation of the table may be necessary.

Notes: The temperature limitations recommended by the sealant manufacturer shall be observed.

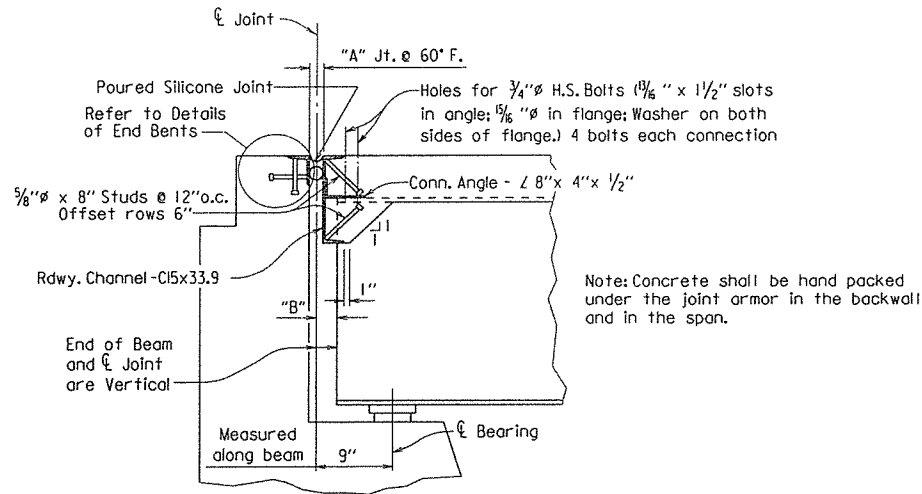
The sealant shall be installed only when the average 24 hour air temperature is between 40° and 80°F.

② BACKER ROD NOTE:

Use an appropriately sized backer rod at the depth shown in the manufacturer's literature based on the joint width at the time of sealing.

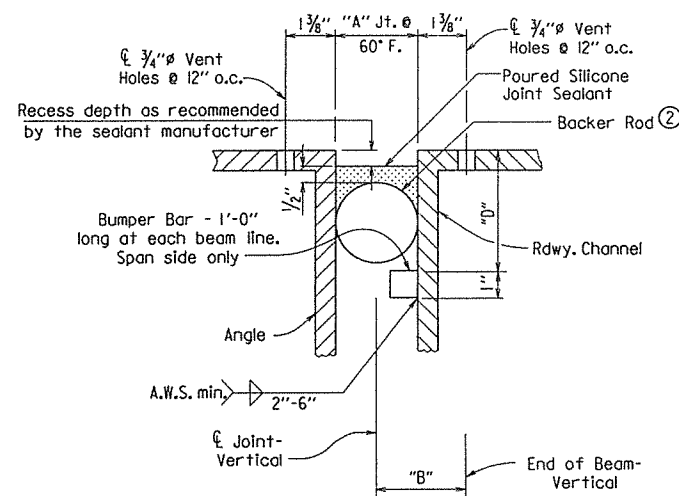
Except as noted, do not install more backer rod that can be sealed in the same day.

The contractor shall verify separation of the backer rod from the joint material after the joint material has set.



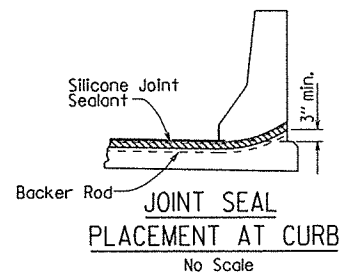
SECTION THRU JOINT AT BENTS 1 & 4

No Scale

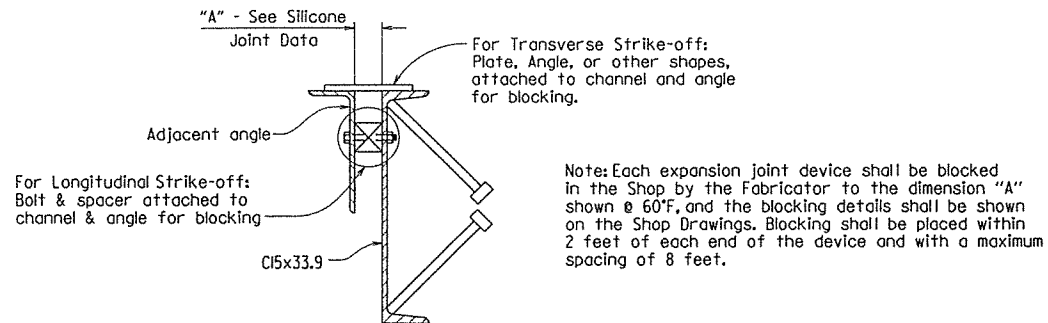


DETAIL OF POURED SILICONE JOINT SEAL

No Scale



No Scale



DETAILS FOR BLOCKING EXPANSION JOINT DEVICE

No Scale

EXPANSION DEVICE INSTALLATION AT END BENTS

The Contractor may elect to install the expansion device for the end bents using one of the following two alternatives:

- 1) The concrete span pour adjacent to the joint shall be placed before the end bent backwall is placed. After the end bent backwall forms are in place and the beams erected, the blocked expansion device shall be installed and adjusted for grade. All connection bolts shall be fully tightened prior to placing the deck concrete adjacent to the bent. Immediately prior to pouring the backwall concrete, the blocking shall be removed, the opening adjusted for temperature, and the backwall constructed.
- 2) The backwall shall be poured to the optional construction joint after beams are erected. The blocked expansion device shall be installed and adjusted for grade. All connection bolts shall be fully tightened prior to placing the deck concrete adjacent to the bent. Immediately prior to pouring the remainder of the backwall concrete, the blocking shall be removed and the opening adjusted for temperature. Backfill shall not be placed behind the backwall until the deck concrete on the adjacent span has been placed.

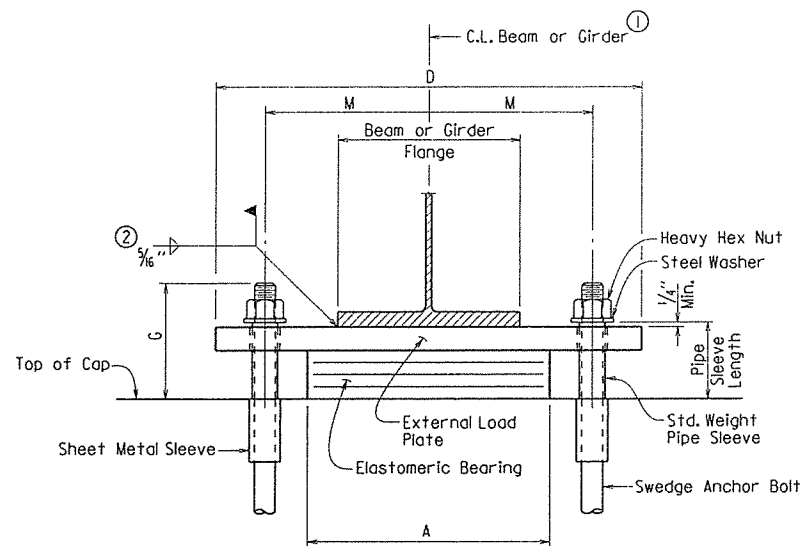


BRIDGE ENGINEER

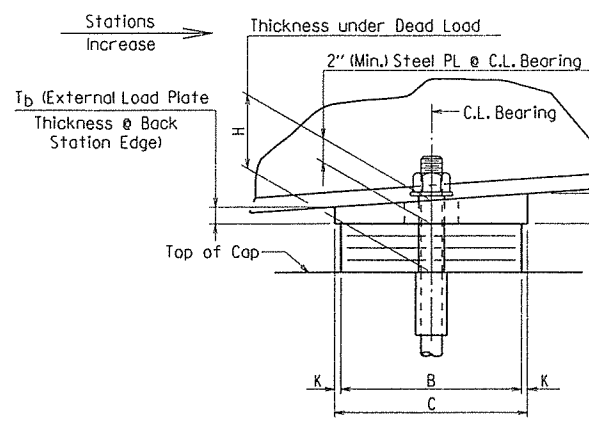
DETAILS OF JOINTS
MAZARN CREEK
ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: KDH DATE: 9-11-14 FILENAME: bbr4906_jtl.dgn
CHECKED BY: CSP DATE: 11/14/14 SCALE: AS NOTED
DESIGNED BY: ADV DATE: 7-19
BRIDGE NO. 04930 DRAWING NO. 56461

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
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							JOB NO.	BR4906
								33
								85
							04930 - ELASTO. BEARINGS - 56462	



FRONT VIEW

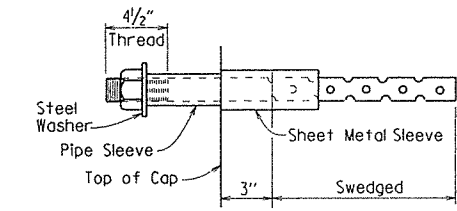


SIDE VIEW

The direction of bevel of the external load plate may not be accurately depicted with respect to T_a and T_b values shown in the "Table of Fabricator Variables".

Unless otherwise approved by the Engineer, welding of the external load plate at expansion bearings to the beam or girder will be allowed only when: 1) the approximate average air temperature during the 24 hour period immediately preceding welding is between 40°F and 80°F; and 2) the slots in the external load plate are positioned to center on the anchor bolts; and 3) no horizontal deformation of the elastomeric pad is evident. If welding at other temperatures is required, the Engineer will provide adjustment data.

Care shall be taken to ensure that the external load plate is in full and complete contact with the beam or girder flange before welding begins.



ANCHOR BOLT DETAIL

Anchor Bolts may be cast in place or drilled and grouted into place. If Anchor Bolts are to be cast in place, the Galvanized Sheet Metal Sleeves will not be required.

If Anchor Bolts are to be drilled and grouted in place, the Galvanized Sheet Metal Sleeves shall be cast in place as shown. Sleeves shall be dry packed with styrofoam, urethane foam or approved equal prior to pouring of concrete. After pouring of the cap and prior to erection of Structural Steel, the dry pack shall be removed and holes for the anchor bolts shall be accurately drilled into the concrete. Bolts placed in drilled holes shall be accurately set and fixed using a 0PL approved epoxy or non-shrink grout that completely fills the holes. Galvanized Sheet Metal Sleeves will not be paid for directly, but will be considered subsidiary to the item "Structural Steel in Beam Spans (M270, Gr. 50W)".

GENERAL NOTES

Elastomeric Bearings shall conform to Section 808 and shall be paid for at the unit price bid for "Elastomeric Bearings".

External load plates shall conform to AASHTO M270, Grade 50W. Pipe sleeves shall be ASTM A53, Grade B, and shall be galvanized to conform to AASHTO M 232, Class C or AASHTO M 298, Class 50.

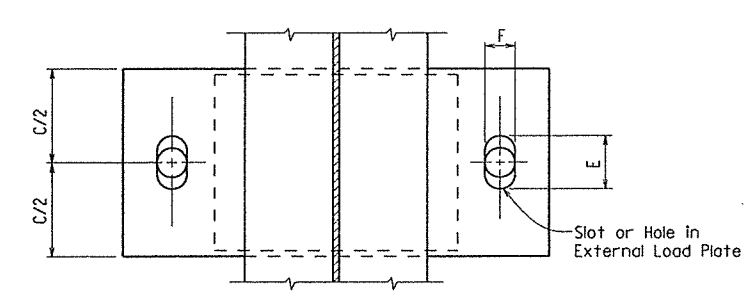
External load plates shall be completely fabricated (including bevel and bolt holes) and shall be cleaned before vulcanizing to the elastomeric bearing. The surface in contact with the elastomeric bearing shall be cleaned in accordance with Subsection 808.03. Other surfaces shall be blast cleaned in accordance with Subsection 807.84(b) for painted steel and 807.84(e) for unpainted Grade 50W steel.

Anchor Bolts, Washers and Nuts shall conform to Subsection 807.07. The anchor bolt grade of steel shall be as specified in the "Table of Fabricator Variables". Indentations shall be circular with rounded bottoms and staggered as shown in the details.

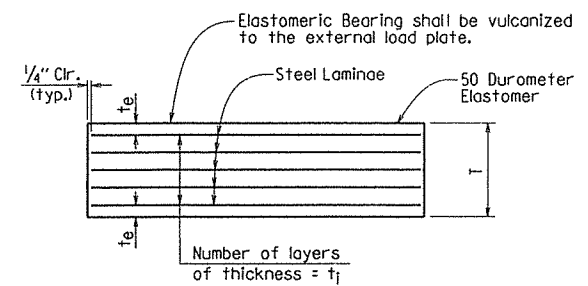
Pipe Sleeves, Anchor Bolts, Washers and Nuts shall be paid for at the unit price bid for "Structural Steel in Beam Spans (M270, Gr. 50W)". External load plates will not be measured and paid for separately, but will be considered incidental to the unit price bid for "Elastomeric Bearings".

Bearings shall be seated in accordance with Subsection 808.08. This work and materials are considered subsidiary to the item "Elastomeric Bearings" and will not be paid for directly.

① C.L. Elastomeric Pad shall be aligned with C.L. Beam or Girder.



PLAN VIEW



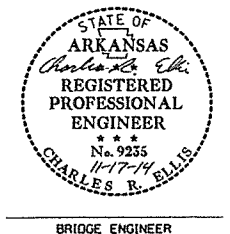
ELASTOMERIC BEARING

t_e = Thickness of elastomer cover on top and bottom of pad
t_i = Thickness of elastomer between steel laminæ
N = Number of elastomer layers of thickness t_i

TABLE OF FABRICATOR VARIABLES

*Maximum Design Load = Service I Limit State

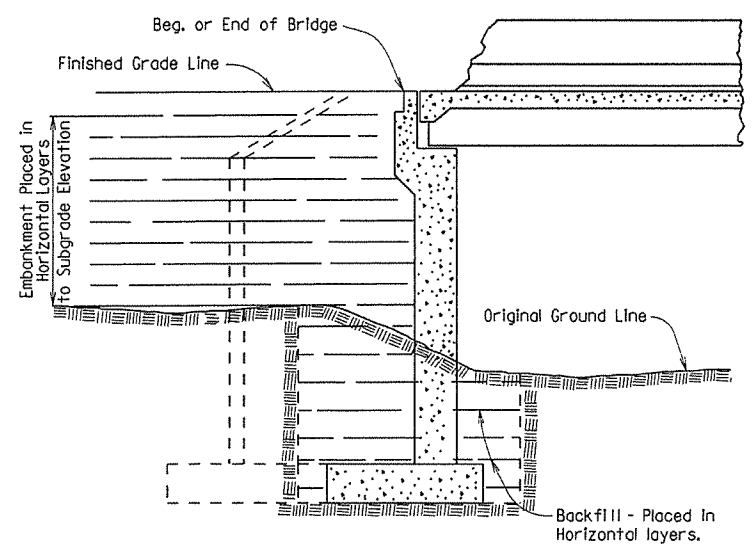
BRIDGE NO.	LOCATION		BEARING TYPE	NO. OF BEARINGS EACH BENT	*MAXIMUM DESIGN LOAD (KIPS)	G	H	ELASTOMERIC PAD				T	EXTERNAL LOAD PLATE						ANCHOR BOLT										
	BENT NO(S)	BEAM OR GIRDER NO.						A	B	N	t _i		t _e	NO. & THICKNESS OF STEEL LAMINAE	C	D	E	F	K	M	T _a	T _b	ANCHOR BOLT (Ø x L)	PIPE SLEEVE SIZE (Ø x L)	SHEET METAL SLEEVE SIZE (Ø x L)	STEEL WASHER SIZE (O.D.)			
04930	1 & 4	All	Exp.	4	89	7"	4 3/8"	12 1/2"	8"	3	1/2"	1/4"	4 @ 12 Ga.	2 7/8"	9"	21 1/2"	3 5/8"	2"	1/2"	8 1/4"	2.05"	1.95"	1 1/4" x 21"	55	1 1/4" x 4 3/4"	3" x 6"	2 1/2"		
	2 & 3	All	Fix	4	193	7 1/4"	3 9/16"	14"	12"	2	1/2"	1/4"	3 @ 12 Ga.	1 9/16"	13"	26"	3 1/8"	3 3/8"	1/2"	9 3/4"	2.08"	1.92"	2" x 29"	55	2 1/2" x 4 1/4"	4" x 6"	3 3/4"		



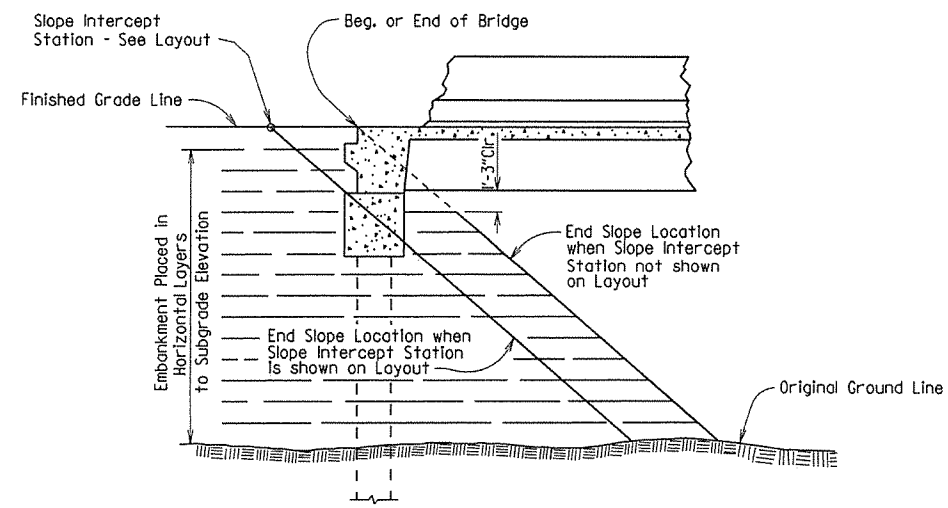
DETAILS OF ELASTOMERIC BEARINGS
MAZARN CREEK
ROUTE SEC.
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.
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BRIDGE NO. 04930 DRAWING NO. 56462

PRINT DATE: 11/14/2014

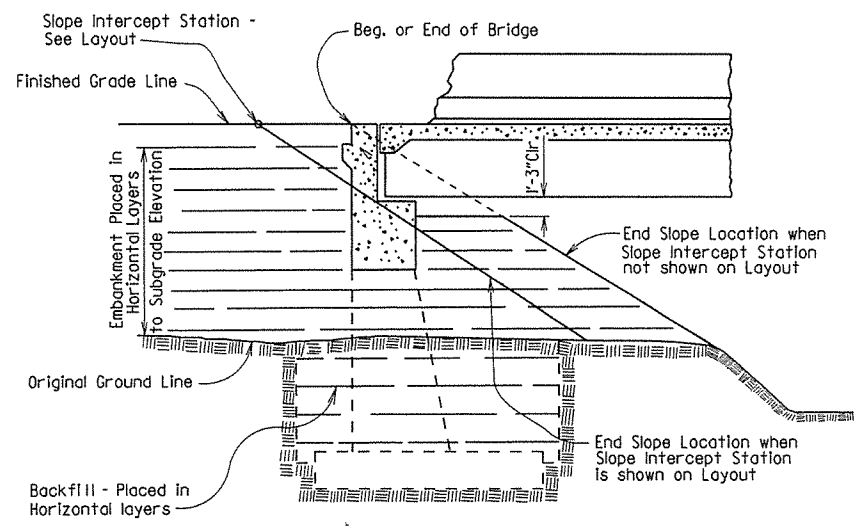
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							JOB NO.	
							EMBANKMENT & BACKFILL	55000



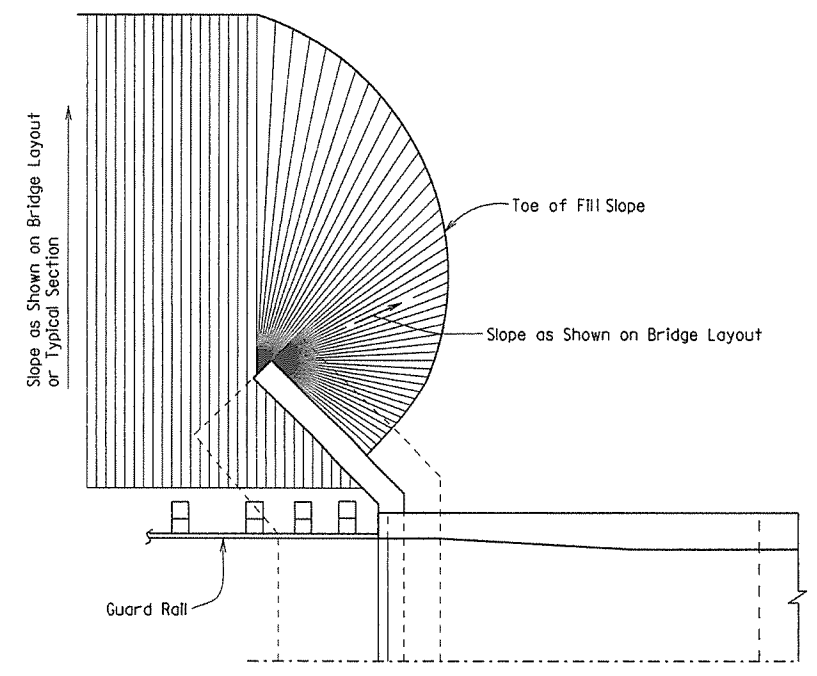
EMBANKMENT CONSTRUCTION AND FOOTING BACKFILL AT VERTICAL WALL ABUTMENTS



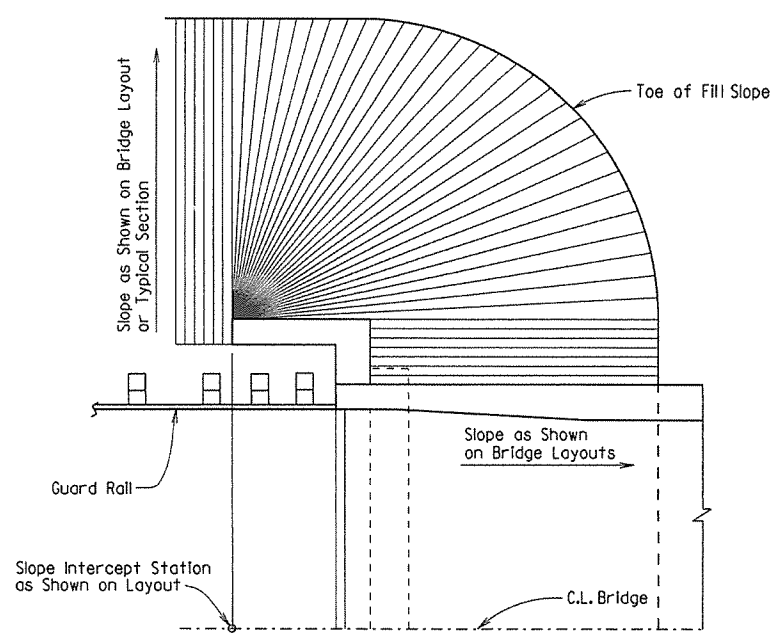
EMBANKMENT CONSTRUCTION AT SPILL-THROUGH PILE END BENTS



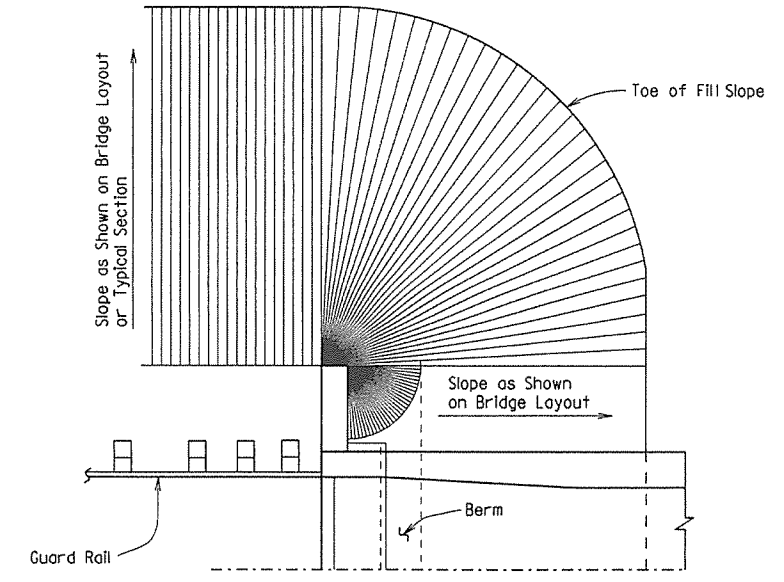
EMBANKMENT CONSTRUCTION AND FOOTING BACKFILL AT SPILL-THROUGH END BENTS



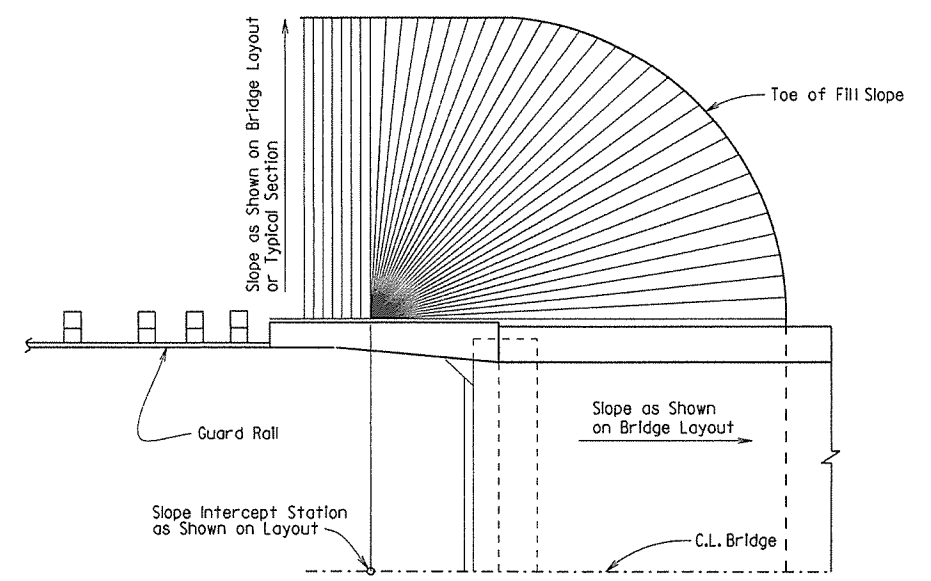
VERTICAL WALL ABUTMENTS



SPILL-THROUGH END BENTS WITH TURNBACK WING



SPILL-THROUGH END BENTS WITH STUB WING



SPILL-THROUGH END BENTS WITH TRANSITION WING

METHOD OF DETERMINING FILL SLOPE LOCATION AT BRIDGE ENDS

GENERAL NOTES

The Bridge End Embankment shall be defined as a section of embankment, not less than 20 feet long adjacent to the bridge end, together with the side slopes and slopes under the bridge end including around the end of wingwalls. Embankment adjacent to structures shall be constructed in 6 inch horizontal layers (loose measure) and compacted by the use of mechanical equipment to the satisfaction of the Engineer. Refer to Subsections 210.09, 210.10 and 801.08 for construction requirements.

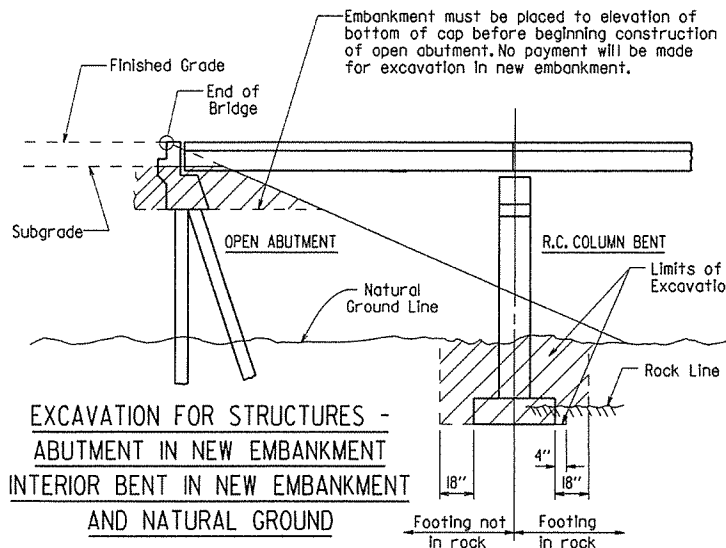
STANDARD DETAILS FOR EMBANKMENT CONSTRUCTION AND BACKFILL AT BRIDGE ENDS

ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

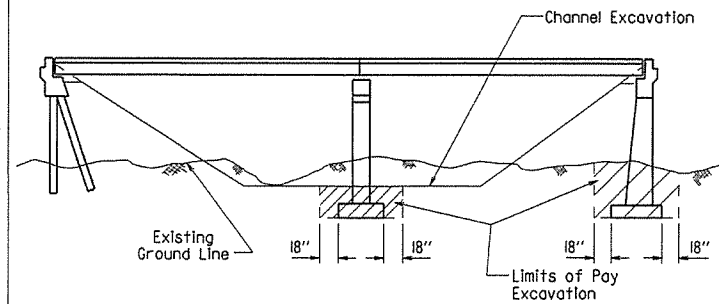
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DESIGNED BY: STD. DATE: -

DRAWING NO. 55000

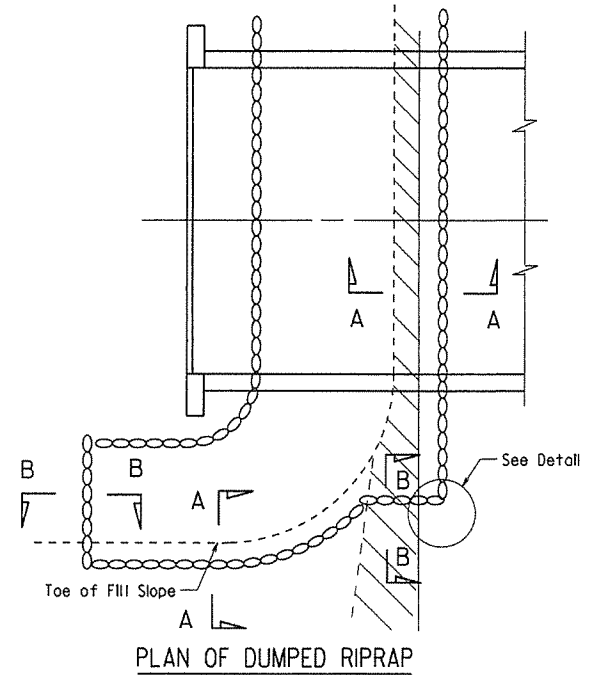
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		35	
				JOB NO.		RIPRAP & EXCAV. 55001		



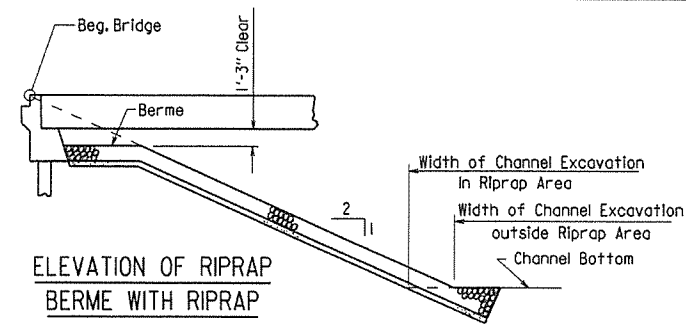
EXCAVATION FOR STRUCTURES - ABUTMENT IN NEW EMBANKMENT AND NATURAL GROUND



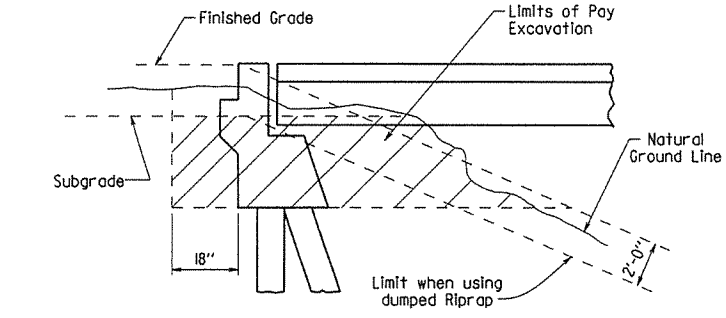
EXCAVATION FOR STRUCTURES - BRIDGE LOCATION WITH DESIGNATED CHANNEL CHANGE



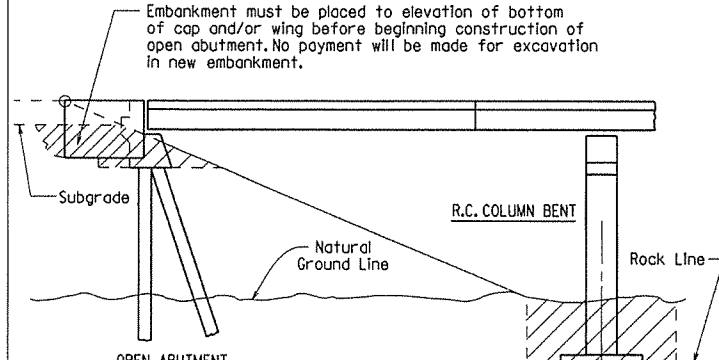
PLAN OF DUMPED RIPRAP



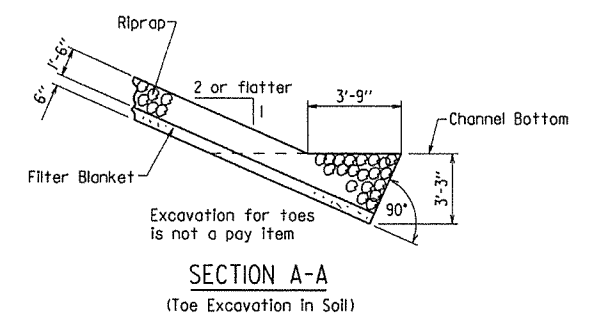
ELEVATION OF RIPRAP BERME WITH RIPRAP



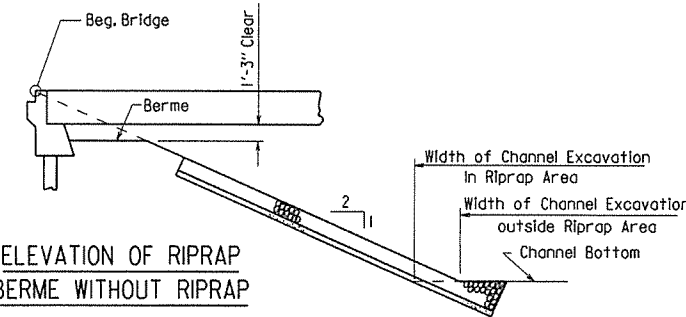
EXCAVATION FOR STRUCTURES - ABUTMENT IN NATURAL GROUND



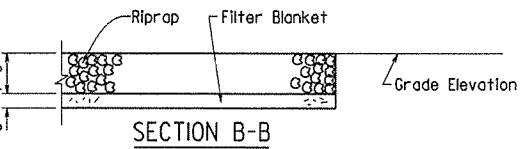
EXCAVATION FOR STRUCTURES - ABUTMENT IN NEW EMBANKMENT WITH TURNBACK WINGS



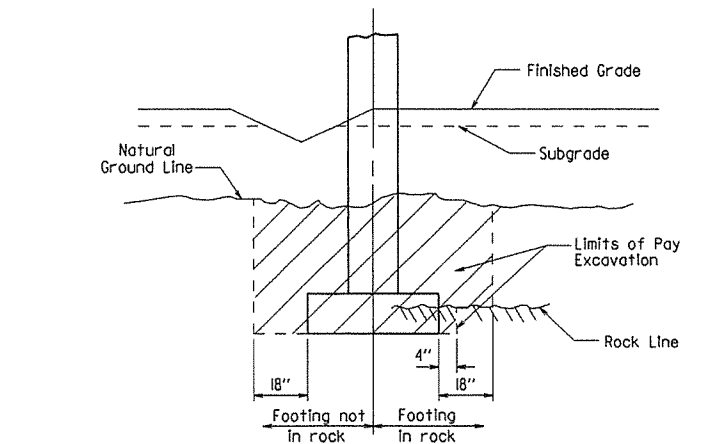
SECTION A-A (Toe Excavation in Soil)



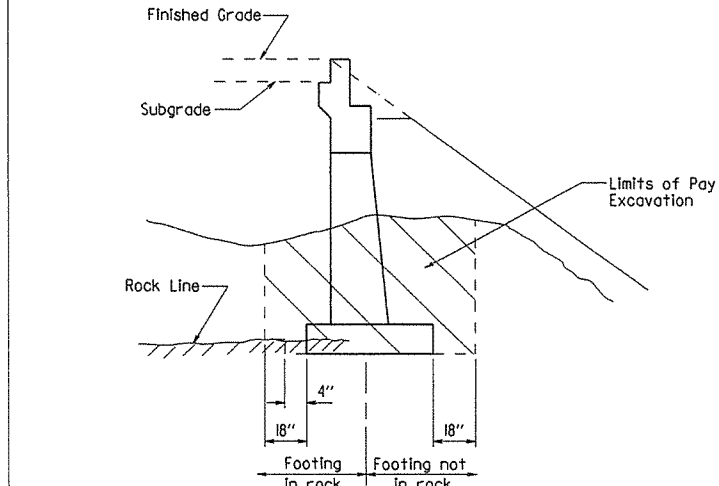
ELEVATION OF RIPRAP BERME WITHOUT RIPRAP



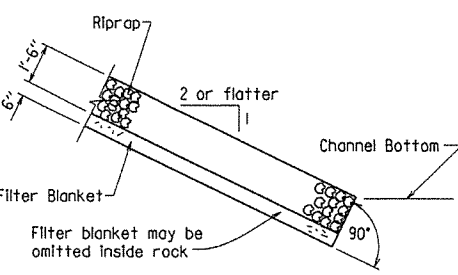
SECTION B-B



EXCAVATION FOR STRUCTURES - BENT IN ROADWAY FILL SECTION AND NATURAL GROUND



EXCAVATION FOR STRUCTURES - ABUTMENT IN NATURAL GROUND AND NEW EMBANKMENT

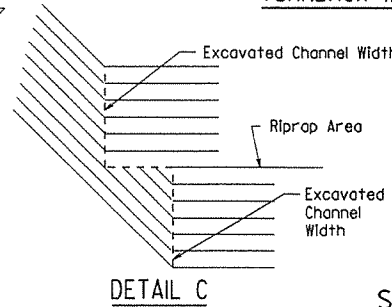


SECTION A-A (Toe Excavation in Rock)

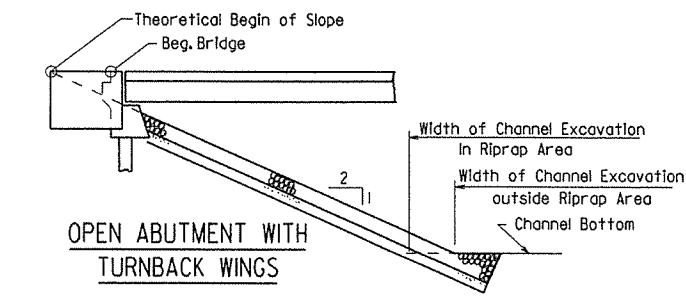
Note: Use this type of toe when rock is encountered which is in a stable condition.

Note: In lieu of an aggregate filter blanket, a synthetic fiber geotextile fabric complying with the requirements of Subsection 816.02(e) may be used.

Note: Details for computing excavation for structures are included for information as to how plan quantities were calculated and for use when adjusting quantities when changing footing elevation.



DETAIL C



OPEN ABUTMENT WITH TURNBACK WINGS

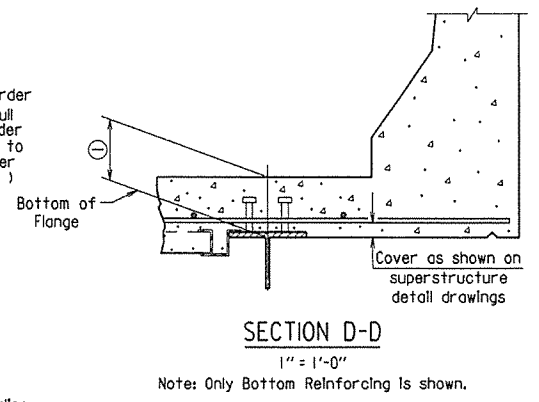
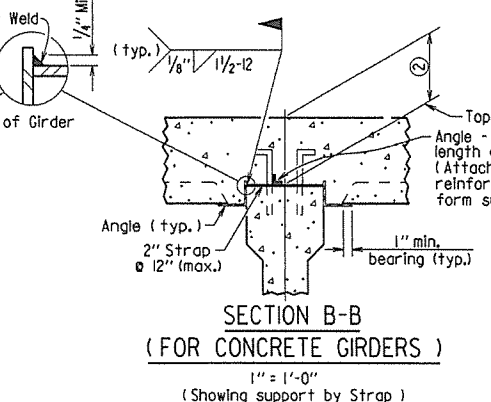
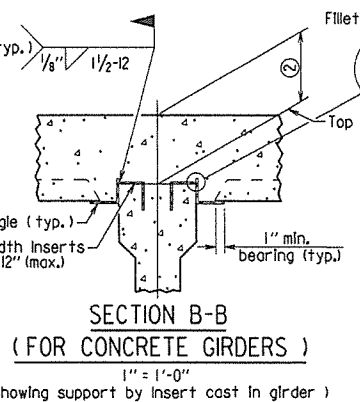
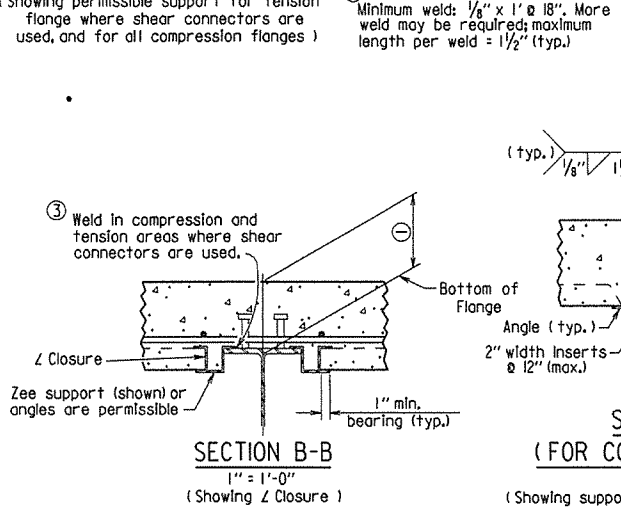
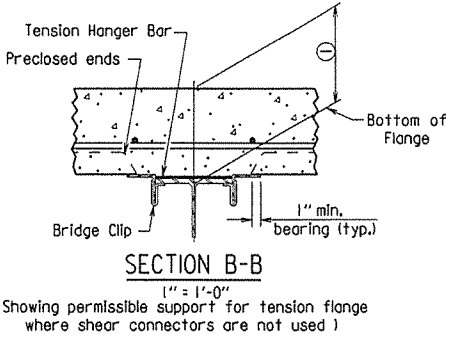
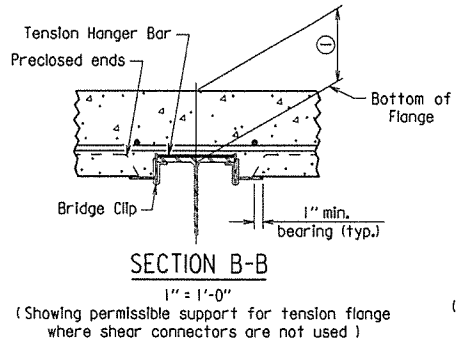
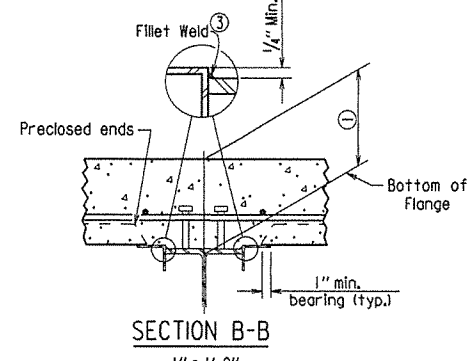
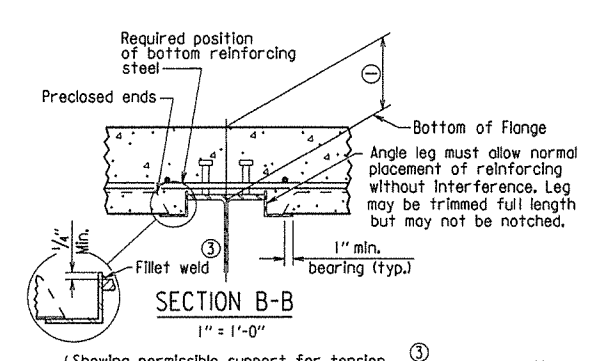
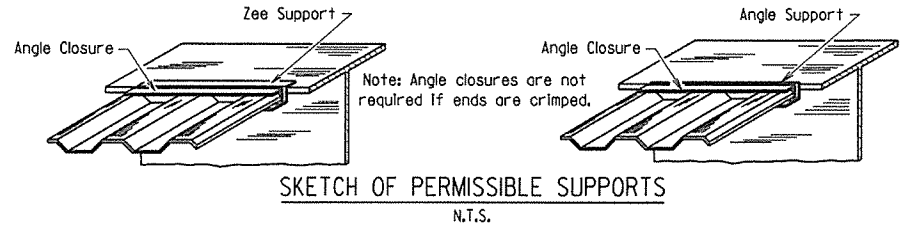
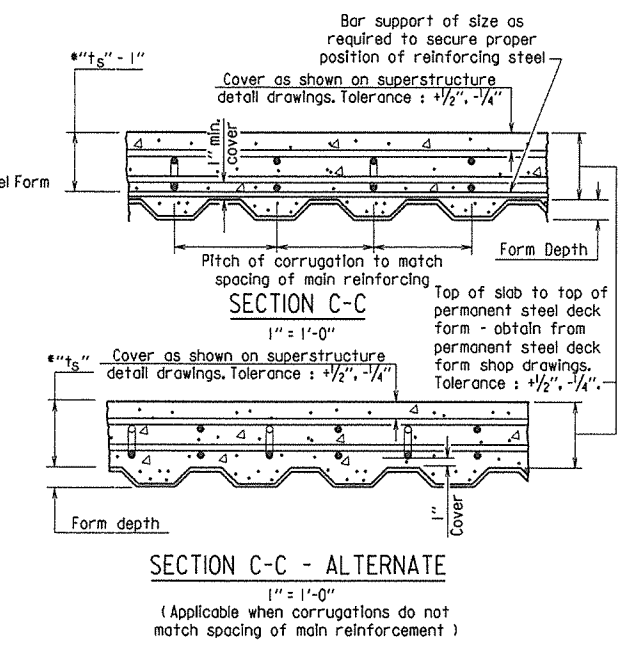
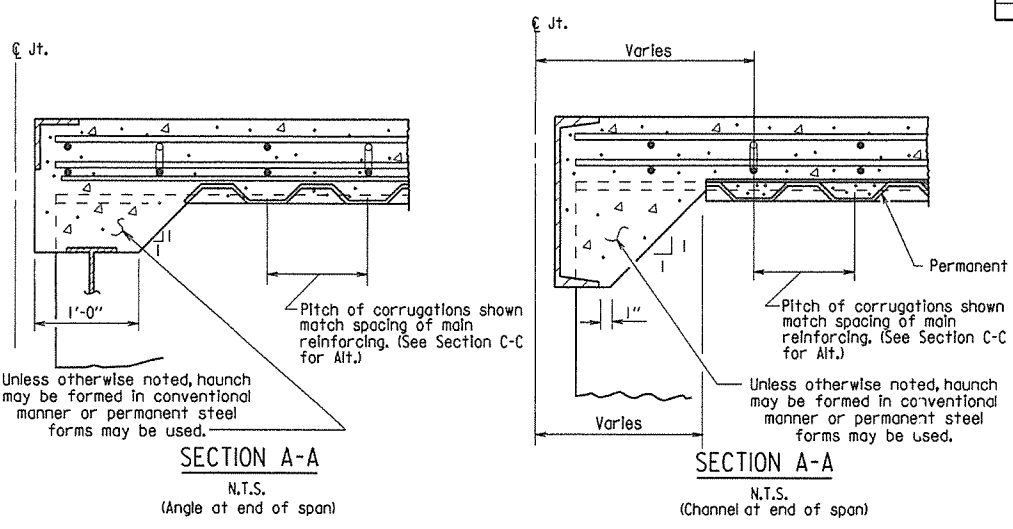
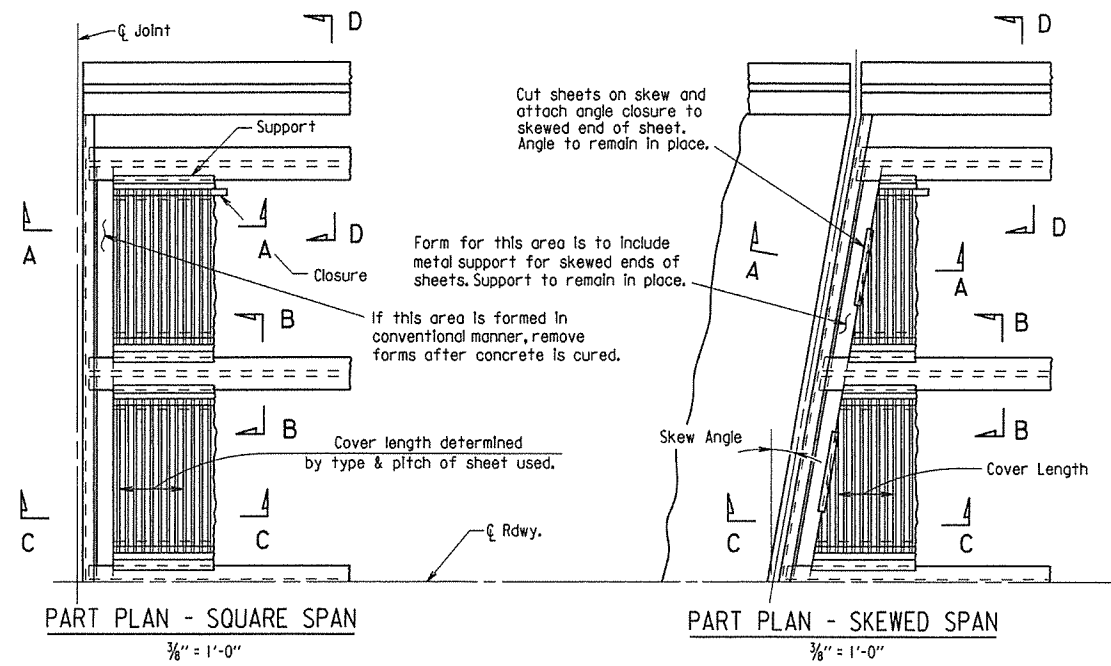
STANDARD DETAILS FOR DUMPED RIPRAP AND FILTER BLANKET AND COMPUTING EXCAVATION FOR STRUCTURES

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

DRAWN BY: KDH DATE: 2-27-2014 FILENAME: b55001.dgn
 CHECKED BY: BEF DATE: 2-27-2014 SCALE: NO SCALE
 DESIGNED BY: STD. DATE:

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		36	
							JOB NO.	
							BRIDGE DECK FORMS	55005



GENERAL NOTES

Permanent steel deck forms may be used at the Contractor's option and shall be at no additional cost to the Department. Such use may result in changes to the dead load deflection of the girder. Any cost for adjustments due to a change in the dead load deflection will be borne by the Contractor. Payment for deck concrete and structural steel will not be increased due to use of permanent steel deck forms.

Permanent steel deck forms shall conform to Subsection 802.14(b). Detailed plans, including detailed calculations and manufacturer's technical brochure, shall be submitted to and approved by the Engineer before work of forming the bridge deck is started.

Welding of form supports to the tension flange of steel girders will be permitted only in areas where shear connectors are used. When welding is not allowed, the method of fastening Z or L supports to the flange must be approved by the Engineer.

Form sheets shall be fastened to supporting members and to each other with galvanized metal screws sufficient in size and number to provide a secure attachment. Alternate methods of attachment must be approved by the Engineer.

When the pitch of form corrugations match the reinforcing spacing, transversely align form sheets across the bridge to maintain the correct orientation of continuous reinforcing bars in the corrugations.

Bar support rods, when used, shall be sized and spaced to adequately support the bottom reinforcing mat at the required position.

High chairs shall be sized to support the top mat of reinforcing at the proper position. High chairs shall be placed at locations shown on the detail drawings.

Specifications: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction (2014 Edition), with applicable Supplemental Specifications and Special Provisions.

STANDARD DETAILS FOR PERMANENT STEEL BRIDGE DECK FORMS FOR STEEL & CONCRETE GIRDER SPANS

ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARK.

DRAWN BY: KDH DATE: 2-27-2014 FILENAME: b55005.dgn
CHECKED BY: BEF DATE: 2-27-2014 SCALE: NONE
DESIGNED BY: STD. DATE: _____

① Distance from top of slab to bottom of top flange as measured at centerline girder and as shown on superstructure detail drawings. This dimension may vary within the following limits to maintain the grade and slab thickness tolerances: Minimum - occurs when either the top flange or the support angle leg contacts the bottom reinforcing steel; Maximum = $t_s + 1 3/4$ " + flange thickness. See Section C-C for slab thickness tolerance between adjacent girder flanges.

② Distance from top of slab to top of girder as measured at centerline girder and as shown on superstructure detail drawings. This dimension may vary within the following limits to maintain the grade and slab thickness tolerances: Minimum - occurs when either the top of girder or the support angle leg contacts the bottom reinforcing steel; Maximum - value shown on the superstructure detail drawings when removable forms are used. See Section C-C for slab thickness tolerance between adjacent girder flanges.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		37	
JOB NO.								

TYPE C NAME PLATE 55011

GENERAL NOTES

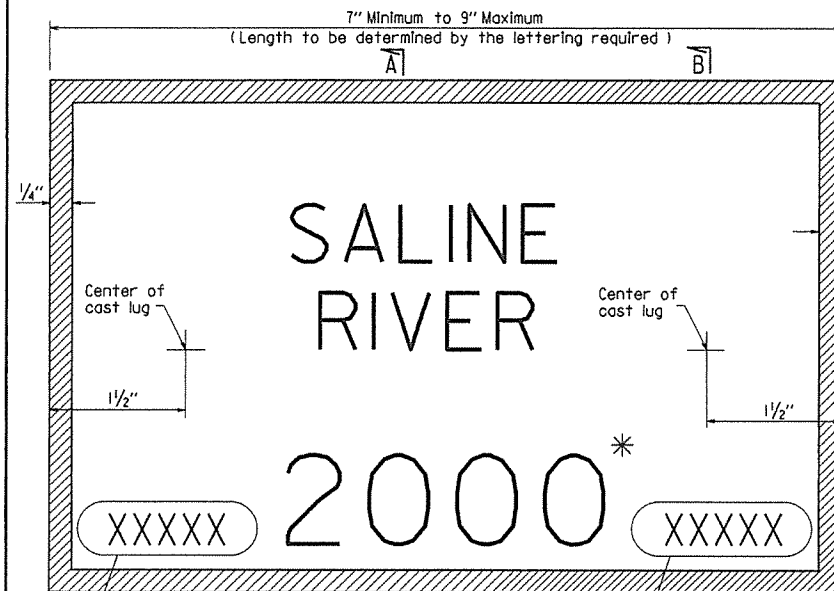
Specifications: Arkansas State Highway and Transportation Department Standard Specifications for Highway Construction, (2014 Edition) with applicable Supplemental Specifications and Special Provisions.

Name plates shall be cast bronze and shall meet the material requirements as specified in Section 812.

Body of plate shall be $\frac{3}{16}$ " thick and shall include two tapering cone lugs $\frac{3}{16}$ " to $\frac{1}{8}$ " x 2" long. The border and all lettering shall be raised $\frac{1}{8}$ " above the face of plate and shall be polished.

All lettering shall be plain gothic, square cut and not tapered.

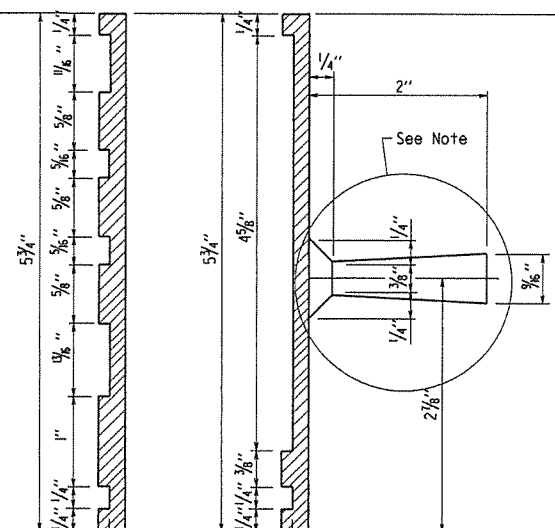
The number of plates required and the location and name on the plate for each bridge shall be as designated on the plans.



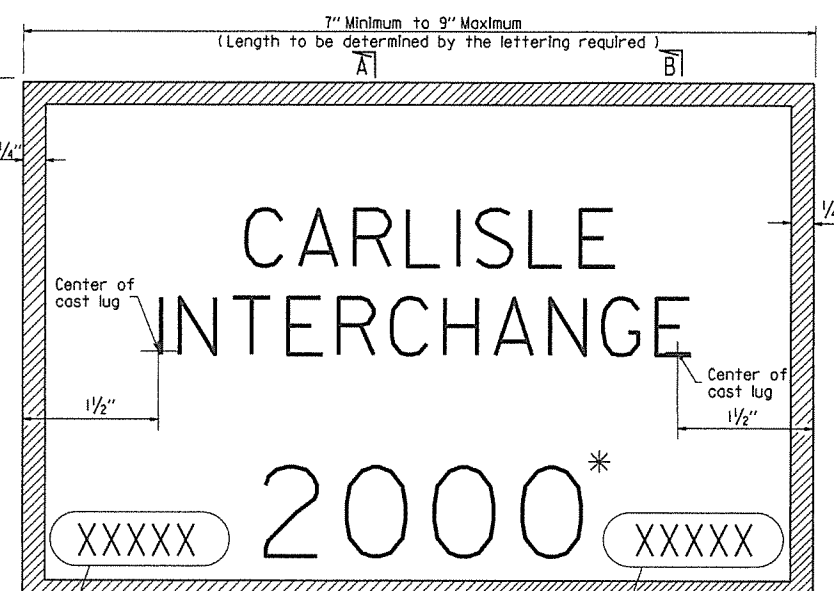
Place the design loading here using $\frac{1}{8}$ " raised letters and numerals $\frac{3}{16}$ " high. Examples: HS 20 HL-93

Place the Bridge number here using $\frac{1}{8}$ " raised letters and numerals $\frac{3}{16}$ " high. Example: 06275

TYPICAL BRIDGE NAME PLATE-STYLE 1 - FULL SIZE
STREAM CROSSINGS



SECTION A-A SECTION B-B

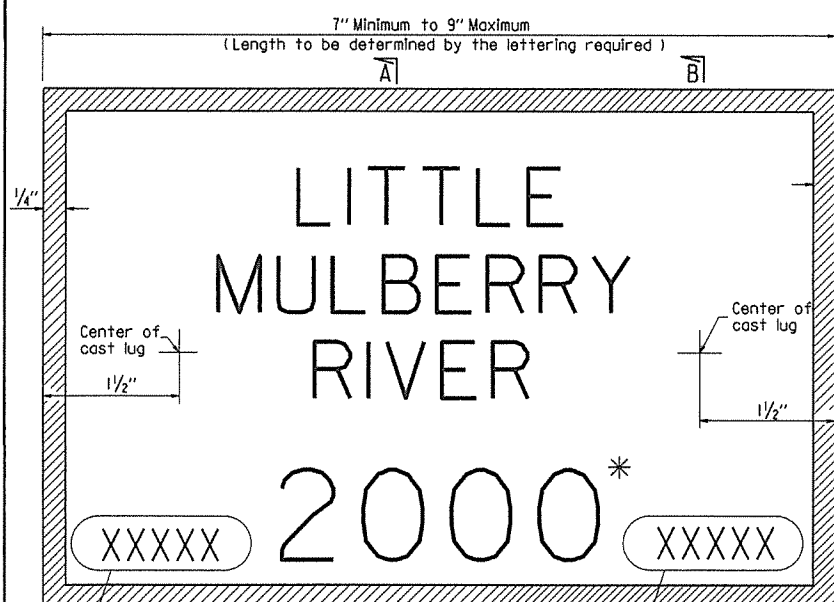


Place the design loading here using $\frac{1}{8}$ " raised letters and numerals $\frac{3}{16}$ " high. Examples: HS 20 HL-93

Place the Bridge number here using $\frac{1}{8}$ " raised letters and numerals $\frac{3}{16}$ " high. Example: 06275

TYPICAL BRIDGE NAME PLATE-STYLE 3 - FULL SIZE
GRADE SEPARATION STRUCTURES

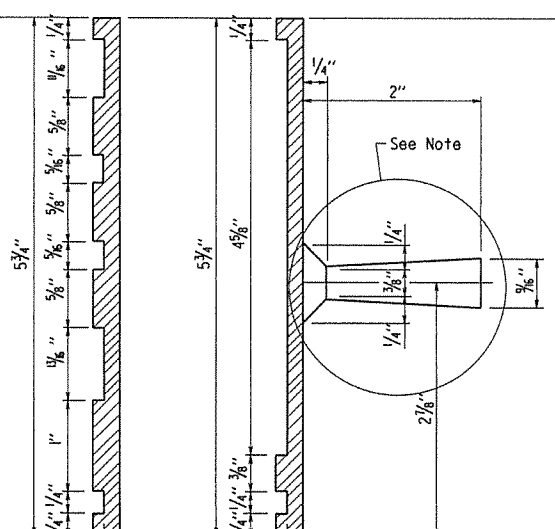
Note: Alternate attachments may be used provided such attachments are submitted and approval secured before fabrication is begun.



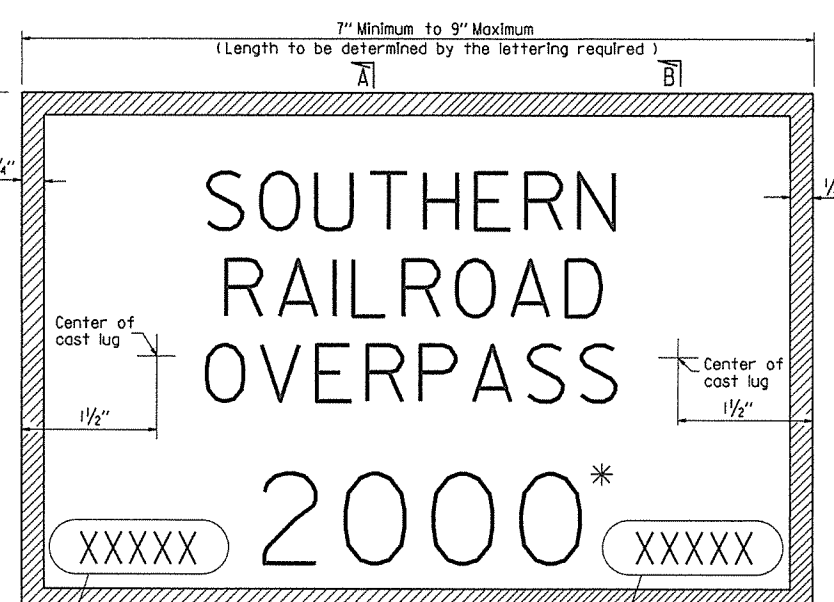
Place the design loading here using $\frac{1}{8}$ " raised letters and numerals $\frac{3}{16}$ " high. Examples: HS 20 HL-93

Place the Bridge number here using $\frac{1}{8}$ " raised letters and numerals $\frac{3}{16}$ " high. Example: 06275

TYPICAL BRIDGE NAME PLATE-STYLE 2 - FULL SIZE
STREAM CROSSINGS



SECTION A-A SECTION B-B



Place the design loading here using $\frac{1}{8}$ " raised letters and numerals $\frac{3}{16}$ " high. Examples: HS 20 HL-93

Place the Bridge number here using $\frac{1}{8}$ " raised letters and numerals $\frac{3}{16}$ " high. Example: 06275

TYPICAL BRIDGE NAME PLATE-STYLE 4 - FULL SIZE
GRADE SEPARATION STRUCTURES

* Year in which contract is awarded.

STANDARD DETAILS FOR
TYPE C BRIDGE NAME PLATES

ARKANSAS STATE HIGHWAY COMMISSION

LITTLE ROCK, ARK.

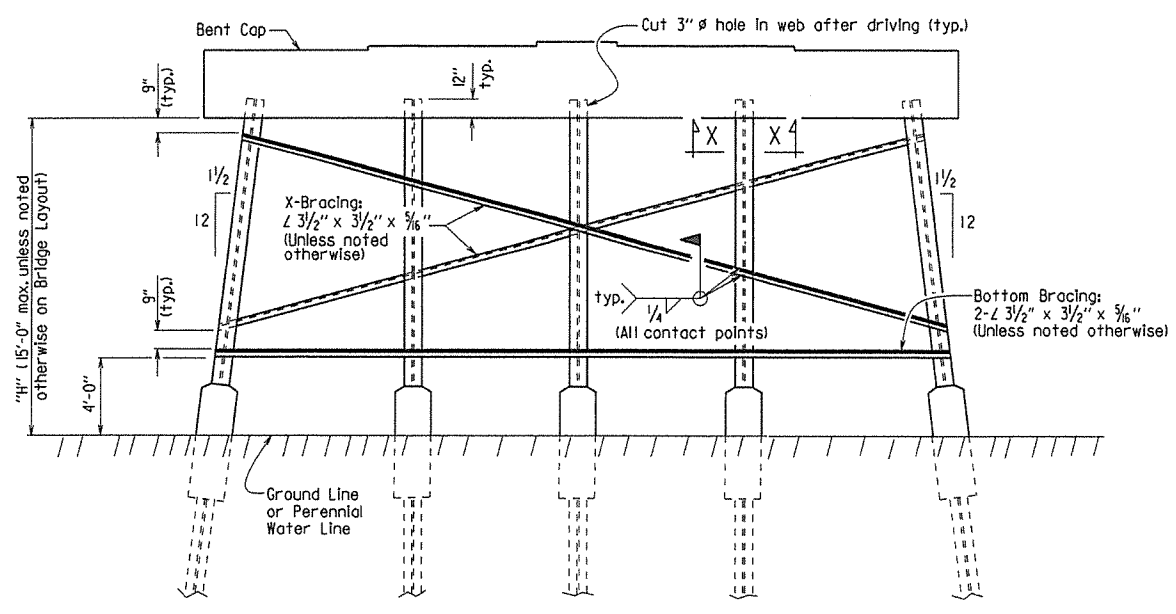
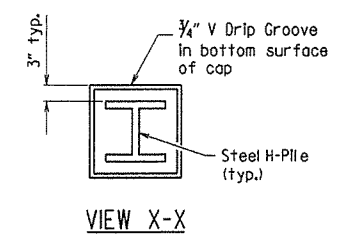
DRAWN BY: KDH DATE: 2-27-2014 FILENAME: b55011.dgn
CHECKED BY: BEF DATE: 2-27-2014 SCALE: NO SCALE
DESIGNED BY: STD. DATE: —

DRAWING NO. 55011

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		38	
JOB NO.							STEEL H-PILES	55020

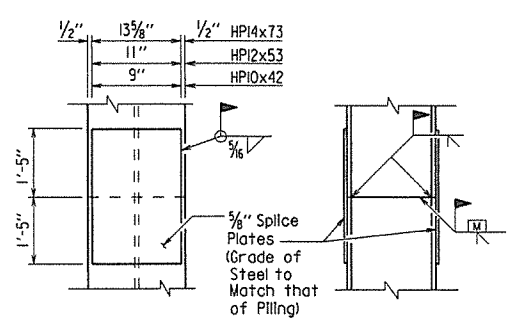
GENERAL NOTES FOR STEEL H-PILES:

Steel H-Piles shall conform to AASHTO M 270, Grade 36 or greater.
 See Bridge Layout and Bent Details for pile size, estimated length, spacing, pile anchorage (if required) and for driving information.
 Steel H-Piles that extend above the ground and are not protected by pile encasement shall be painted in accordance with Subsection 805.02.
 Brackets, lugs, cap plates, pile tips, driving points, pile painting, splicing and welding shall not be paid for directly, but shall be considered subsidiary to the item "Steel Piling".



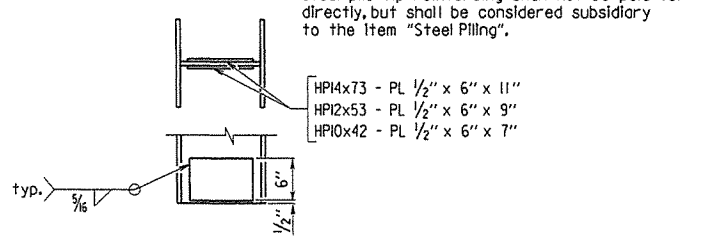
Notes:
 All bracing shall be cut and welded in the field. Each brace shall be furnished in one piece. Payment shall be made under item 807.
 Unless noted otherwise, omit X-Bracing when "H" is less than 8 feet.
 Omit X-Bracing and Bottom Bracing when "H" is 5 feet or less.
 When required on the Bridge Layout sheet, pile encasements shall be constructed. See Notes and Details for H-Pile Encasements.
 Omit all bracing (and V-groove in cap) when pile encasement is extended to bottom of bent cap.

TYPICAL DETAILS OF H-PILE TRESTLE INTERMEDIATE BENT
 (Shown with Partial Height Encasement)



Note:
 The Contractor may for his own convenience and at his own expense provide as many as three splices per pile. Minimum spacing between splices shall be 5 feet.

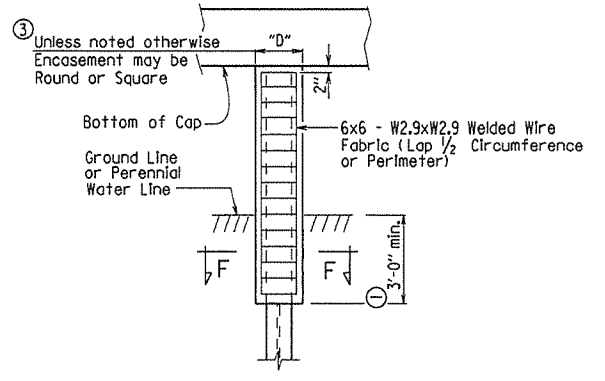
TYPICAL SPLICE DETAILS



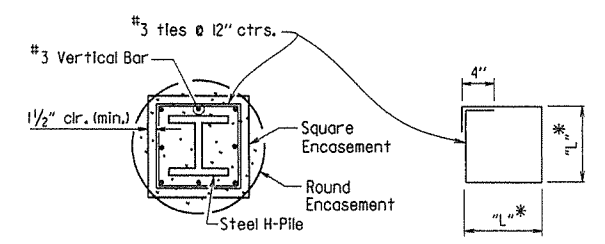
REINFORCING DETAIL FOR STEEL H-PILE TIP

GENERAL NOTES FOR H-PILE ENCASEMENTS:

See Bridge Layout for additional notes and required location of pile encasements.
 All concrete shall be Class S with a minimum 28-day compressive strength, $f'c = 3,500$ psi. If concrete cannot be placed in the dry, Seal Concrete may be used from top to bottom of encasement.
 Reinforcing steel shall be Grade 60 conforming to AASHTO M 31 or M 322, Type A.
 Welded Wire Fabric shall conform to AASHTO M 55 or M 221. Galvanized Corrugated Steel Pipe shall conform to AASHTO M 36 and M 218.
 Concrete, welded wire fabric or reinforcing steel and galvanized pipe shall not be paid for directly, but shall be considered subsidiary to the item "Pile Encasement".



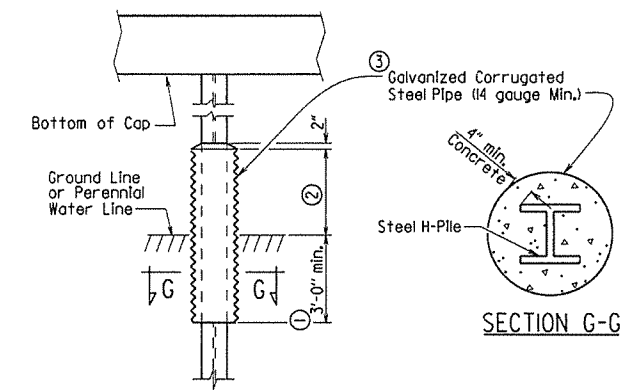
PILE ENCASEMENT DETAIL FOR STEEL H-PILES
 (Shown with Encasement to Bottom of Cap)



SECTION F-F
TABLE OF VARIABLES FOR PILE ENCASEMENT

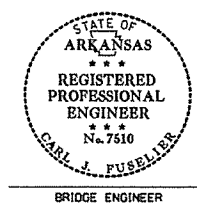
Pile Size	"D"		"L"*
	Square Encsmt.	Round Encsmt.	
HPI0x42	1'-7"	2'-0"	1'-4"
HPI2x53	1'-8"	2'-2"	1'-5"
HPI4x73	1'-11"	2'-6"	1'-8"

* Measured out-to-out of bar.



ALTERNATE PILE ENCASEMENT DETAIL FOR STEEL H-PILES
 (Shown with Partial Height Encasement)

- Unless otherwise noted on Bridge Layout.
- 3'-0" minimum or as shown on Bridge Layout.
- Encasement dimensions shall be sized to maintain a minimum concrete cover of 4" from the H-Pile. Reinforcement shall be sized to provide a minimum concrete cover of 1 1/2" and a minimum clearance of 1 1/4" from the pile.
- Alternate pile encasement, when not extended to bottom of cap, shall have 2" concrete taper for water runoff as shown in the Partial Height Encasement detail.
- Alternate pile encasement may not be allowed. See Bridge Layout.



This document was originally issued and sealed by Carl J. Fuseller, PE No. 7510, on February 27, 2014. This copy is not a signed and sealed document.

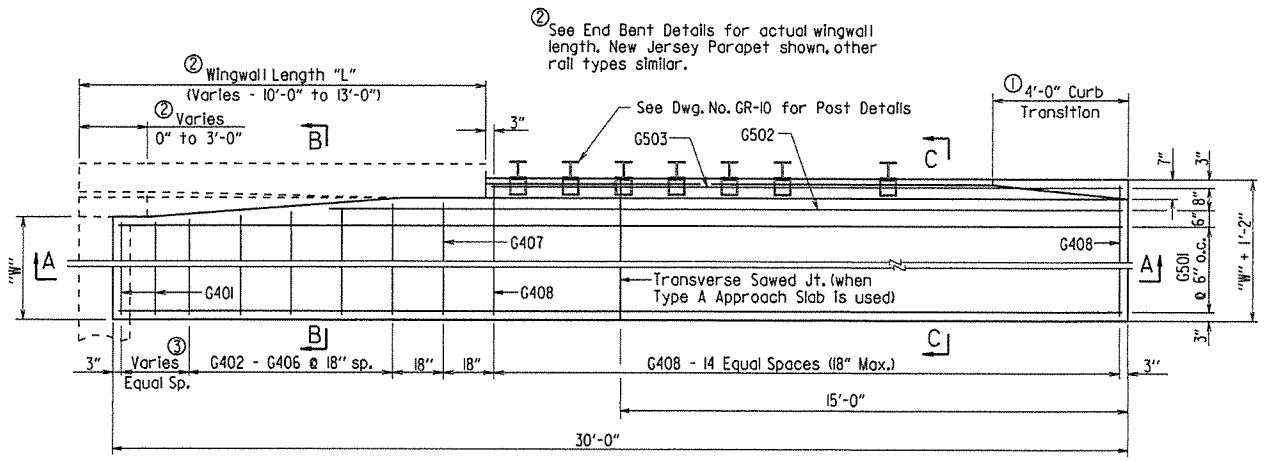
STANDARD DETAILS FOR STEEL H-PILES AND PILE ENCASEMENTS

ARKANSAS STATE HIGHWAY COMMISSION
 LITTLE ROCK, ARK.

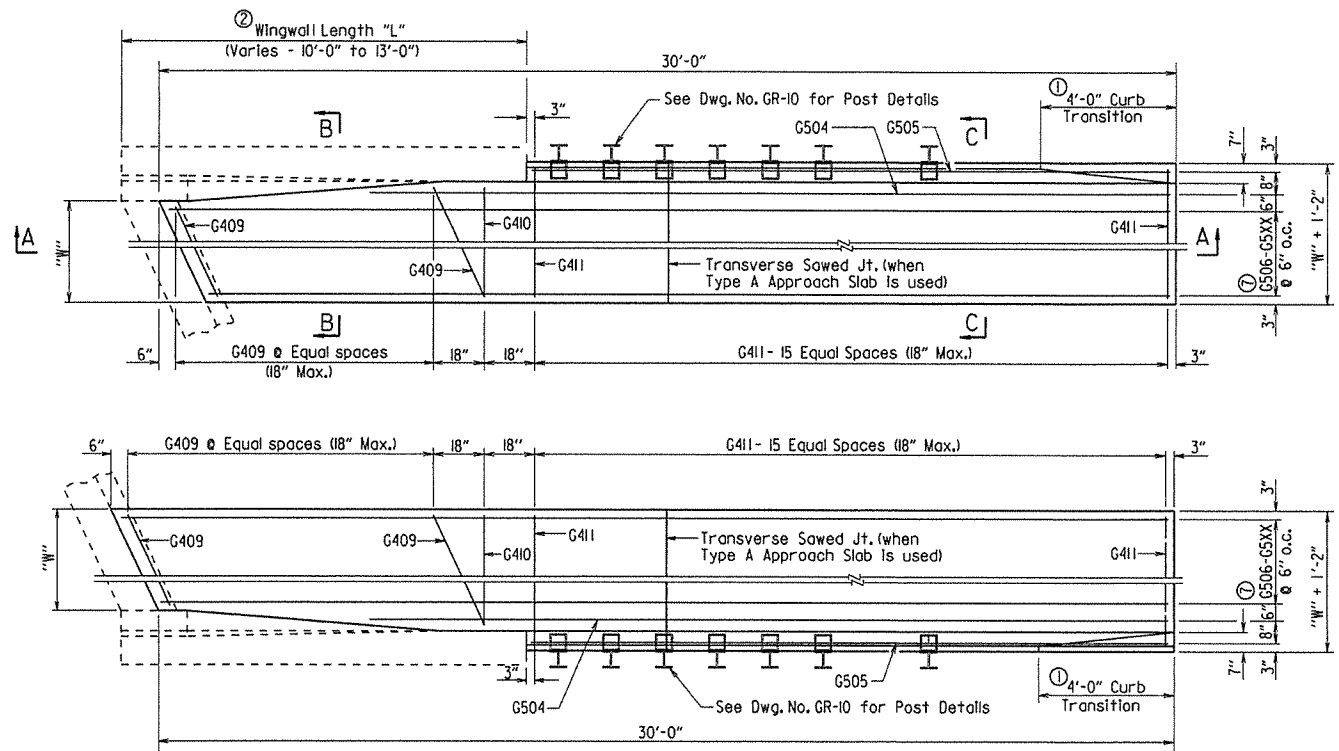
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 CHECKED BY: B.E.F. DATE: 2/27/2014 SCALE: NO SCALE
 DESIGNED BY: STD. DATE: —

DRAWING NO. 55020

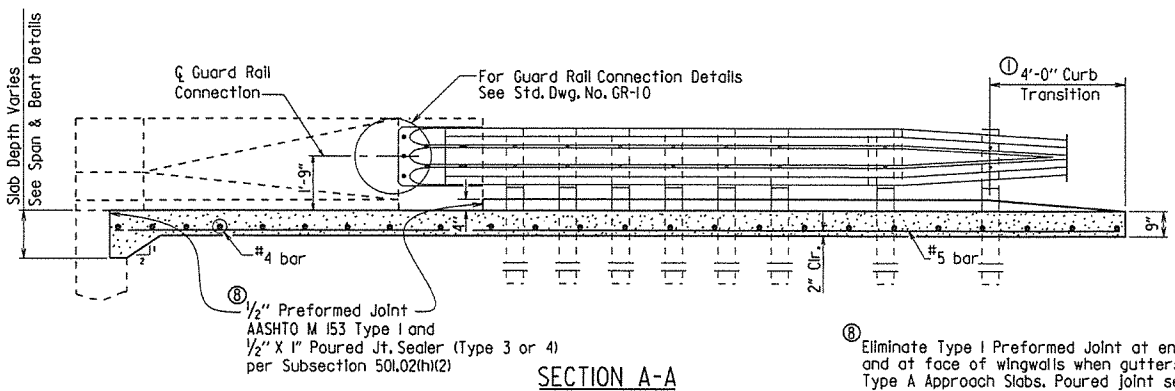
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		39	
JOB NO.							TYPE A GUTTERS	55030A



③ Number of G401 bars vary with wingwall length - See Bar List
HALF PLAN OF APPROACH GUTTERS FOR SQUARE BRIDGE



PLAN OF APPROACH GUTTERS FOR SKEWED BRIDGE

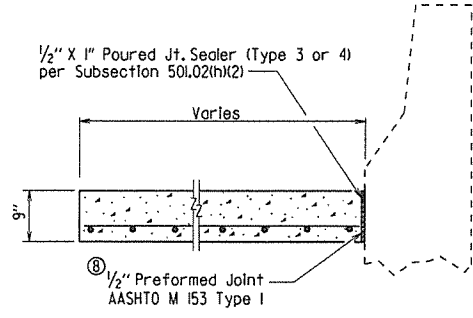


SECTION A-A

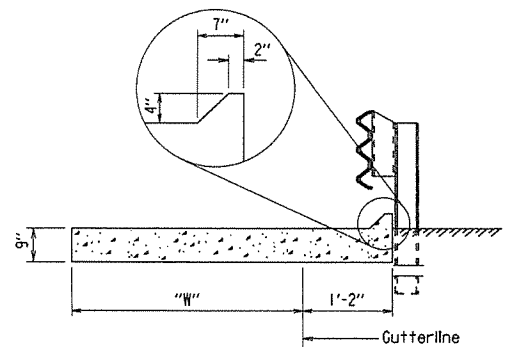
Note:
 All longitudinal lines within the limits of horizontal curves shall be on curves concentric to C.L. Bridge. Adjustment to longitudinal bar lengths may be required. Transverse reinforcing shall be placed on radial lines to C.L. Bridge.

⑧ Eliminate Type I Preformed Joint at end bent backwall and at face of wingwalls when gutters used with Type A Approach Slabs. Poured joint sealer is required, however backer rod shall be eliminated.

① Construct gutter curb with height-transition as shown if drop inlet is not placed at end of gutter.
 Construct gutter curb full height (no height-transition) if drop inlet is placed at end of gutter. Curb height transition placed on drop inlet. See drop inlet details.



SECTION B-B
 N.T.S.



SECTION C-C
 N.T.S.

BAR LIST FOR ONE TYPE A GUTTER

Mark	No. Req'd. for Width "W"				Length
	3'-0"	4'-0"	6'-0"	8'-0"	
G401	④	④	④	④	"W" - 4"
G402-G406	1 each	1 each	1 each	1 each	"W" - 3" to "W" + 2"
G407	1	1	1	1	"W" + 3"
G408	15	15	15	15	"W" + 10"
G501	6	8	12	16	29' - 8"
G502	1	1	1	1	(35' - 5") - "L"
G503	1	1	1	1	30' - 8" - "L"
G409	⑥	⑥	⑥	⑥	⑤
G410	1	1	1	1	"W" + 3"
G411	16	16	16	16	"W" + 10"
G504	1	1	1	1	⑤
G505	1	1	1	1	⑤
G506-G5XX	1 each	1 each	1 each	1 each	⑤

④ 0 for "L" = 10'
 1 for "L" = 11'
 2 for "L" = 12'
 2 for "L" = 13'

⑤ Bar Lengths vary with Skew and Wingwall Length.

⑥ No. Req'd. varies with Skew and Wingwall length.

⑦ G511 for "W" = 3'
 G513 for "W" = 4'
 G517 for "W" = 5'
 G521 for "W" = 8'

QUANTITIES FOR ONE SQUARE APPROACH GUTTER
 (FOR INFORMATION ONLY)

"W" Width (ft.)	Reinforcing Steel (lbs.)	Concrete (Cu. Yds.)
3	285	3.40
4	360	4.25
6	515	5.90
8	665	7.55

Quantities are based on "L" = 10'-0".

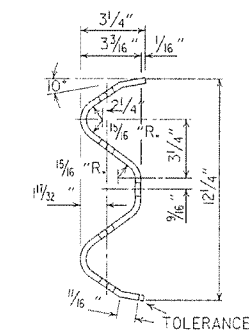
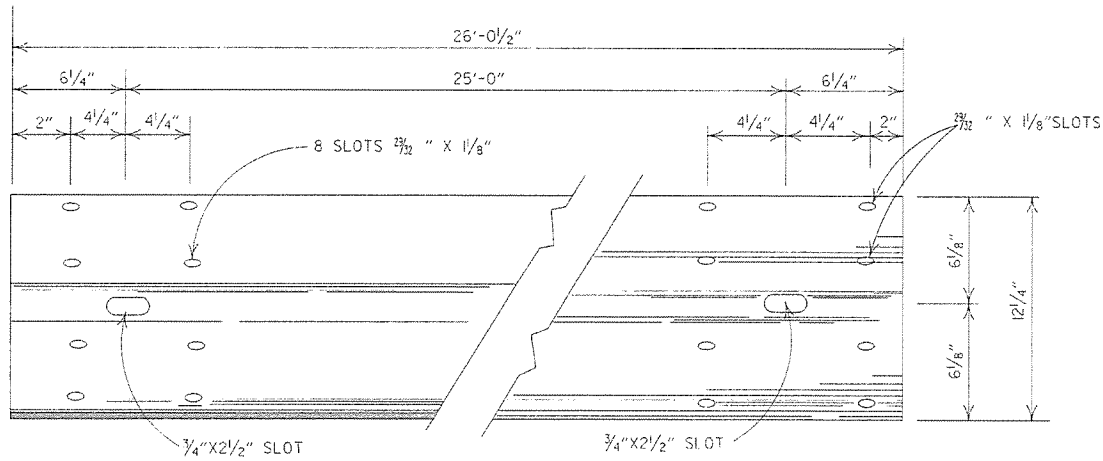
GENERAL NOTES

All concrete shall be Class S or Class S(AE) or mixture used for Portland Cement Concrete Pavement and shall be poured in the dry.
 All reinforcing steel shall be Grade 60 (yield strength = 60,000 psi) conforming to AASHTO M 31 or M 322, Type A, with mill test reports.
 Approach Gutters will be measured and paid for in accordance with Section 504.

STANDARD DETAILS FOR TYPE A APPROACH GUTTERS

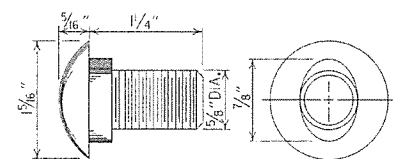
ARKANSAS STATE HIGHWAY COMMISSION
 LITTLE ROCK, ARK.

DRAWN BY: A.M.S. DATE: 2/27/2014 FILENAME: b55030a.dgn
 CHECKED BY: K.W.Y. DATE: 2/27/2014 SCALE: 3/8" = 1'-0"
 DESIGNED BY: STD. DATE: _____ OR AS SHOWN
 DRAWING NO. 55030A

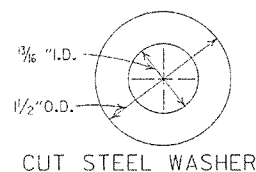


DETAILS OF W-BEAM GUARD RAIL

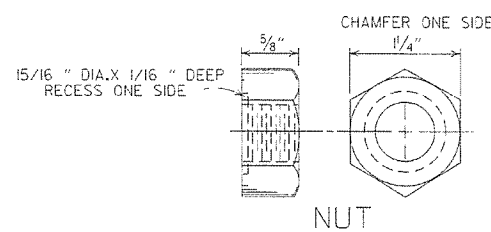
RAIL SECTION OF CLOSELY SIMILAR DIMENSIONS AND COMPARABLE STRENGTH MAY BE SUBSTITUTED IF APPROVED BY THE ENGINEER.



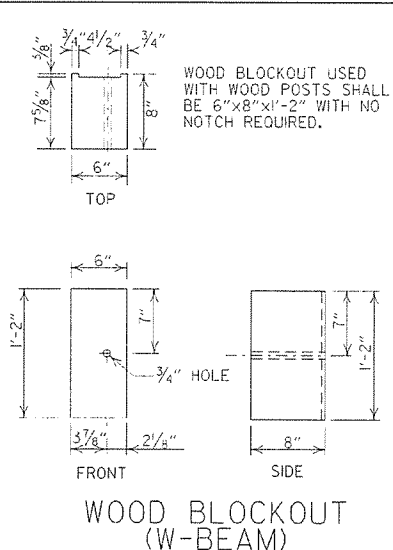
SPLICE BOLT
POST BOLT - SAME EXCEPT LENGTH



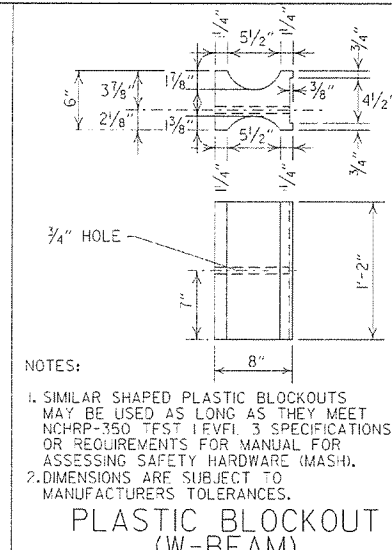
CUT STEEL WASHER



NUT

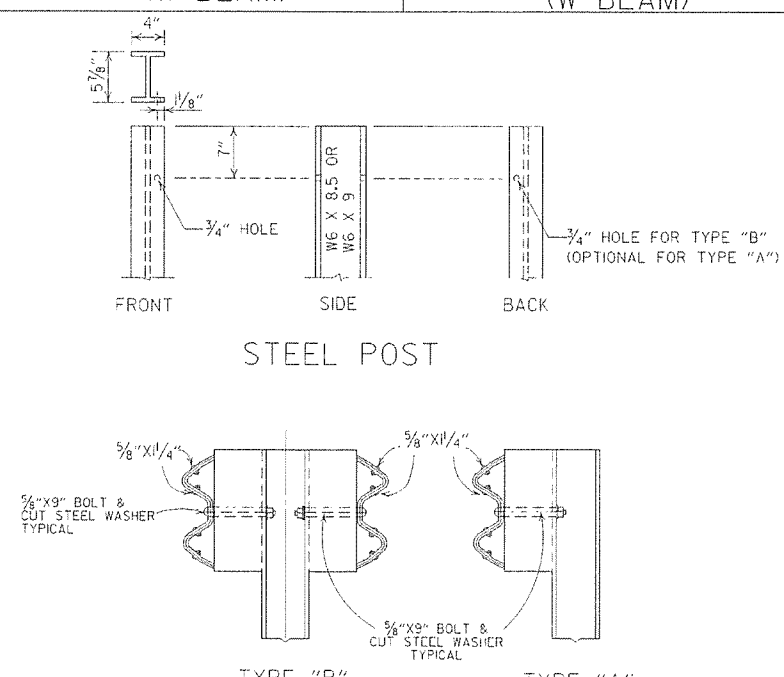


WOOD BLOCKOUT (W-BEAM)



PLASTIC BLOCKOUT (W-BEAM)

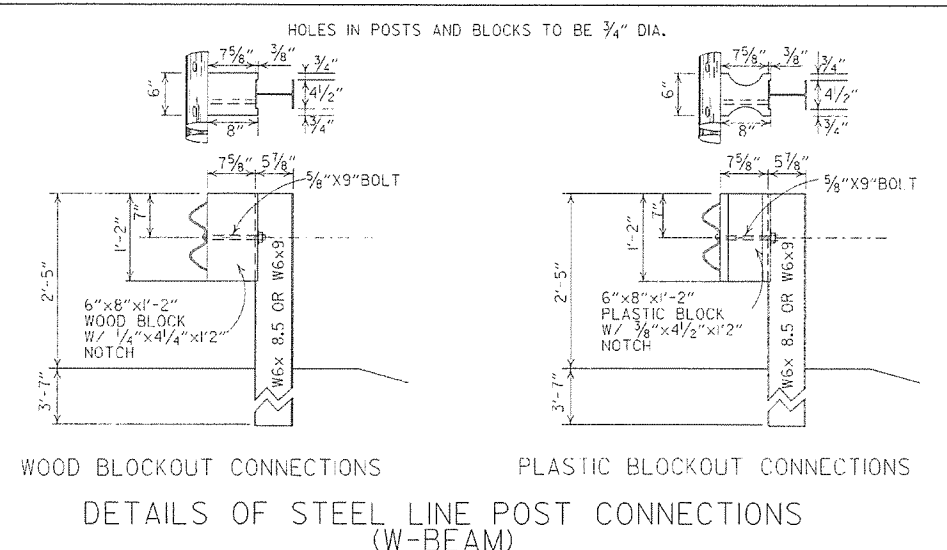
NOTES:
1. SIMILAR SHAPED PLASTIC BLOCKOUTS MAY BE USED AS LONG AS THEY MEET NCHRP-350 TEST LEVEL 3 SPECIFICATIONS OR REQUIREMENTS FOR MANUAL FOR ASSESSING SAFETY HARDWARE (MASH).
2. DIMENSIONS ARE SUBJECT TO MANUFACTURERS TOLERANCES.



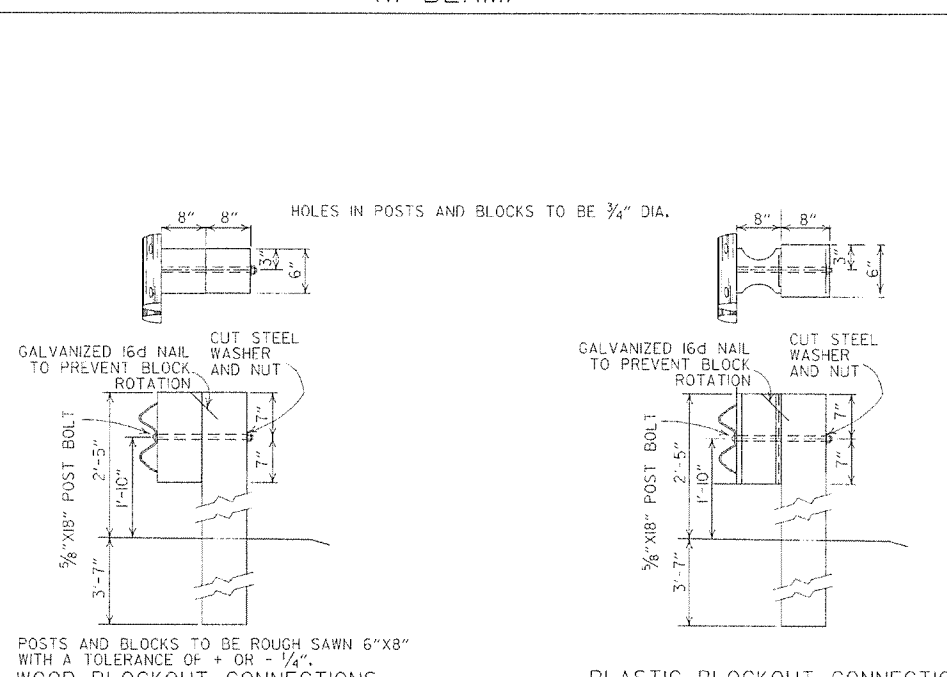
DETAILS OF STEEL LINE POST CONNECTIONS (W-BEAM)

-GENERAL NOTES-

ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN 3/4" BEYOND IT.
WHERE W-BEAM GUARD RAIL CONTINUES, THE INTERMEDIATE SECTIONS SHALL HAVE A POST SPACING OF 6'-3" UNLESS OTHERWISE NOTED.
W-BEAM GUARD RAIL REPRESENTING INTERMEDIATE SECTIONS WILL BE MEASURED ALONG THE ROADWAY FACE FROM CENTERLINE OF POST TO CENTERLINE OF POST.
USE W-BEAM GUARD RAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB. FOR EXTENSIONS OR MODIFICATION OF EXISTING GUARD RAIL, W-BEAM GUARD RAIL COMPONENTS OF THE SAME TYPE AS THOSE EXISTING SHALL BE USED.
ANY BACKFILLING UNDER OR AROUND POST SHALL BE DAMP SAND THOROUGHLY TAMPED IN PLACE.
WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. 1 STRUCTURAL OR BETTER 9.7f (1400 f) OR NO. 1 (350 f) SOUTHERN PINE.
CONTRACTOR SHALL HAVE THE OPTION OF USING WOOD BLOCKOUTS FOR W-BEAM GUARD RAIL OR PLASTIC BLOCKOUTS, AS LONG AS BLOCKOUT USED MEETS NCHRP-350 TEST LEVEL 3 SPECIFICATIONS OR REQUIREMENTS FOR MANUAL FOR ASSESSING SAFETY HARDWARE (MASH) FOR W-BEAM GUARD RAIL.



WOOD BLOCKOUT CONNECTIONS
PLASTIC BLOCKOUT CONNECTIONS
DETAILS OF STEEL LINE POST CONNECTIONS (W-BEAM)



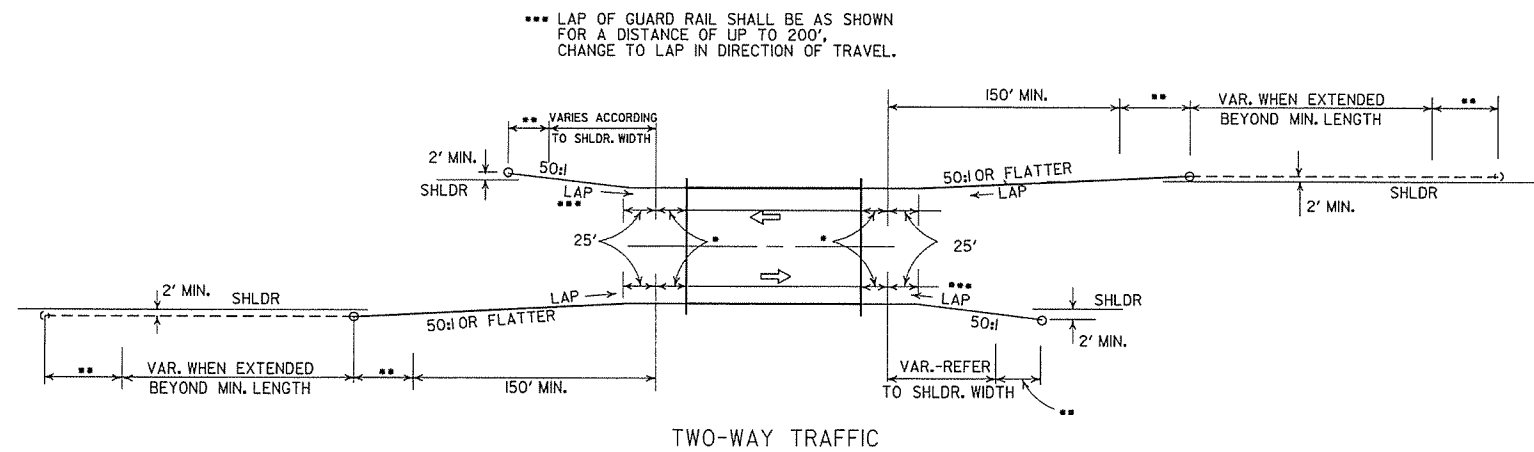
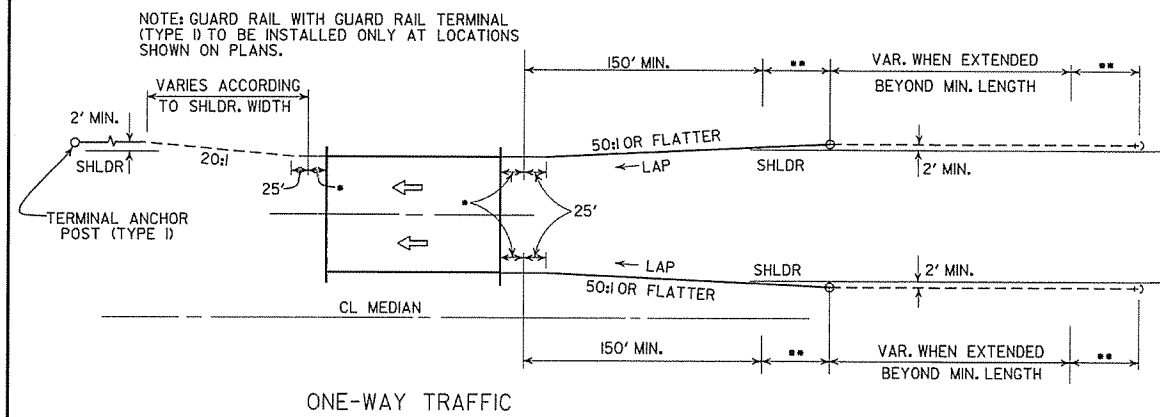
WOOD BLOCKOUT CONNECTIONS
PLASTIC BLOCKOUT CONNECTIONS
DETAILS OF WOOD LINE POST CONNECTIONS (W-BEAM)

7-4-10	RAISED HEIGHT OF GUARD RAIL 1"	
10-15-09	ADDED REFERENCE TO MASH	
4-10-03	REVISED GENERAL NOTES	
8-22-02	REVISED DIMENSION ON WOOD & PLASTIC BLOCKOUT CONNECTIONS & ON STEEL POST	
11-16-01	REVISED WOOD BLOCKOUT & DETAILS OF WOOD LINE POST CONNECTIONS	
3-30-00	REMOVED GUARD RAIL AT BRIDGE ENDS	
11-2-00	ADDED PLASTIC BLOCKOUT	
8-12-98	REV. BLOCKOUTS TO WOOD, DELETED CONC. POST & REV. GENERAL NOTE, DELETED DET. OF GUARD RAIL REPLACE BEHIND CURB & DET. OF POST PLACE IN SOLD ROCK & ADDED DETAILS OF STEEL LINE POST CONN. REMOVED BACK-UP PLATE, REVISED HOLES IN STEEL POLES	
4-3-97	REMOVED "LAP IN DIRECTION OF TRAFFIC" NOTE & PLACED ARROWS ON WASHERS	
10-18-96	REVISED WOOD POST NOTE	
6-2-94	ADDED ALT. STEEL POST SIZE	
8-5-93	REVISED STEEL POST SIZE	8-5-93
10-1-92	REDRAWN & REVISED	10-1-92
8-15-91	REVISED WASHER NOTE	8-15-91
8-2-90	REV. GEN. NOTE & DEPTH OF ANC. POST IN ROCK	8-2-90
7-15-88	REVISED SECTION 3 & GENERAL NOTES	
3-4-88	REV. ANCHOR POST, ELEV. NOTES & POST IN ROCK	780-3-4-88
10-30-87	REVISED WOOD LINE POST DETAIL	546-10-30-87
10-9-87	REDRAWN & REVISED	802-10-9-87
DATE	REVISION	DATE FILED

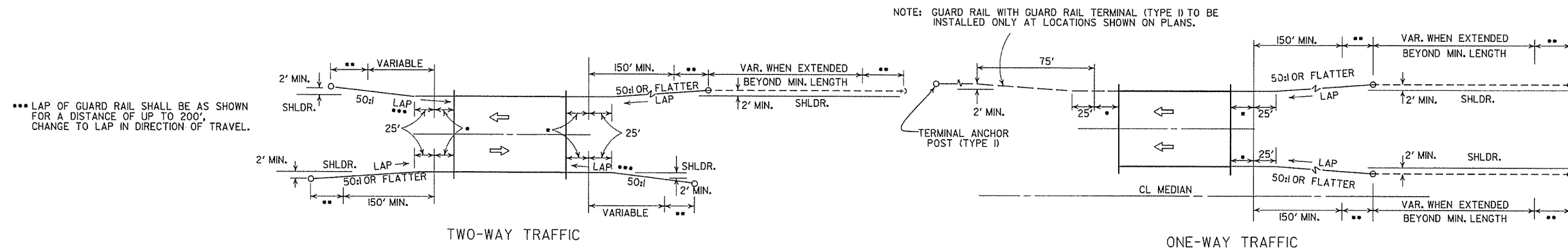
ARKANSAS STATE HIGHWAY COMMISSION

GUARD RAIL DETAILS

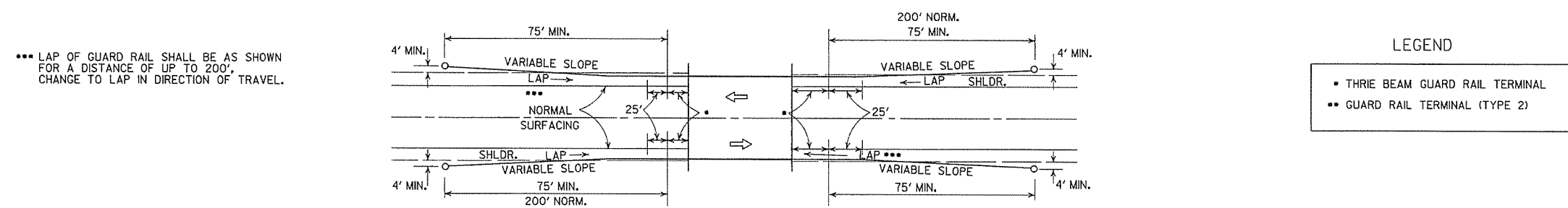
STANDARD DRAWING GR-8



METHODS OF INSTALLATION OF GUARD RAIL AT LESS THAN FULL SHOULDER WIDTH BRIDGES USING GUARD RAIL TERMINAL (TYPE 2)

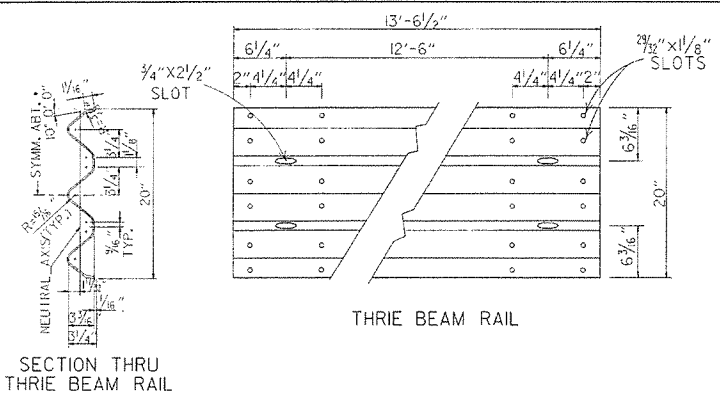


METHOD OF INSTALLATION OF GUARD RAIL AT FULL SHOULDER WIDTH BRIDGES USING GUARD RAIL TERMINAL (TYPE 2)

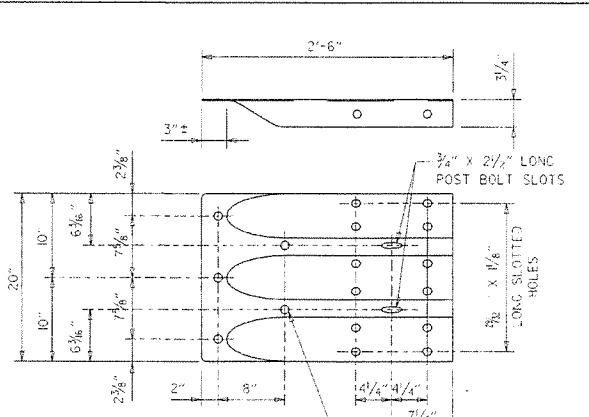


METHOD OF INSTALLATION OF GUARD RAIL USING GUARD RAIL TERMINAL (TYPE I) (FULL SHOULDER WIDTH OR LESS BRIDGES)

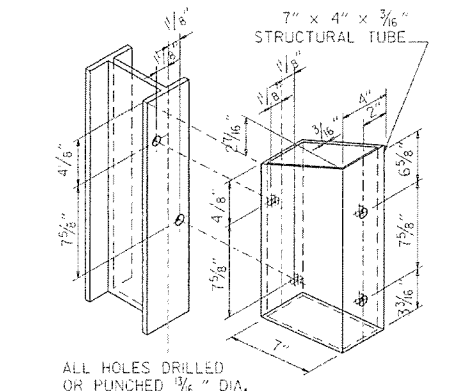
ARKANSAS STATE HIGHWAY COMMISSION		
GUARD RAIL DETAILS		
STANDARD DRAWING GR-9		
4-17-08	REVISED LAYOUTS	
11-10-05	REMOVED GUARD RAIL NOTES AND DETAILS	
11-16-01	DELETED NOTE-METHOD OF INSTALLATION OF GUARD RAIL USING GUARD RAIL TERM. (TY. I)	
1-12-00	ADDED CONSTRUCTION NOTE	1-12-00
6-26-97	REVISED LAYOUT	
10-1-92	REDRAWN & REVISED	10-1-92
10-9-87	ADDED NOTE	
10-9-87	REDRAWN & REVISED	
DATE	REVISION	DATE FILM



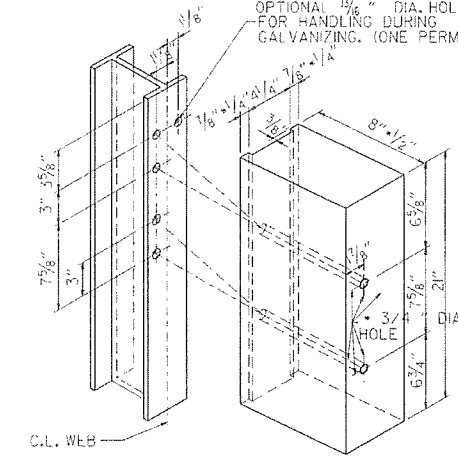
SECTION THRU THRIE BEAM RAIL



SPECIAL END SHOE



STRUCTURAL STEEL TUBING BLOCKOUT DETAIL



HOLE PUNCHING DETAIL FOR STEEL POST & WOOD OR PLASTIC BLOCKOUTS

ATTACH BLOCKOUT TO POST USING 5/8" DIA. HEX HEAD BOLTS WITH 1/2" O.D. CUT STEEL WASHERS AND NUT.

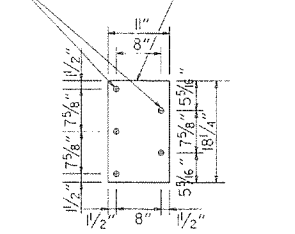
1" DIA. HOLES (TYP.) FOR 7/8" DIA. HIGH-STRENGTH BOLTS

1" DIA. HOLES (TYP.) FOR 7/8" DIA. HIGH STRENGTH BOLTS WITH HEX HEADS, NUTS AND WASHERS

SPECIAL END SHOE

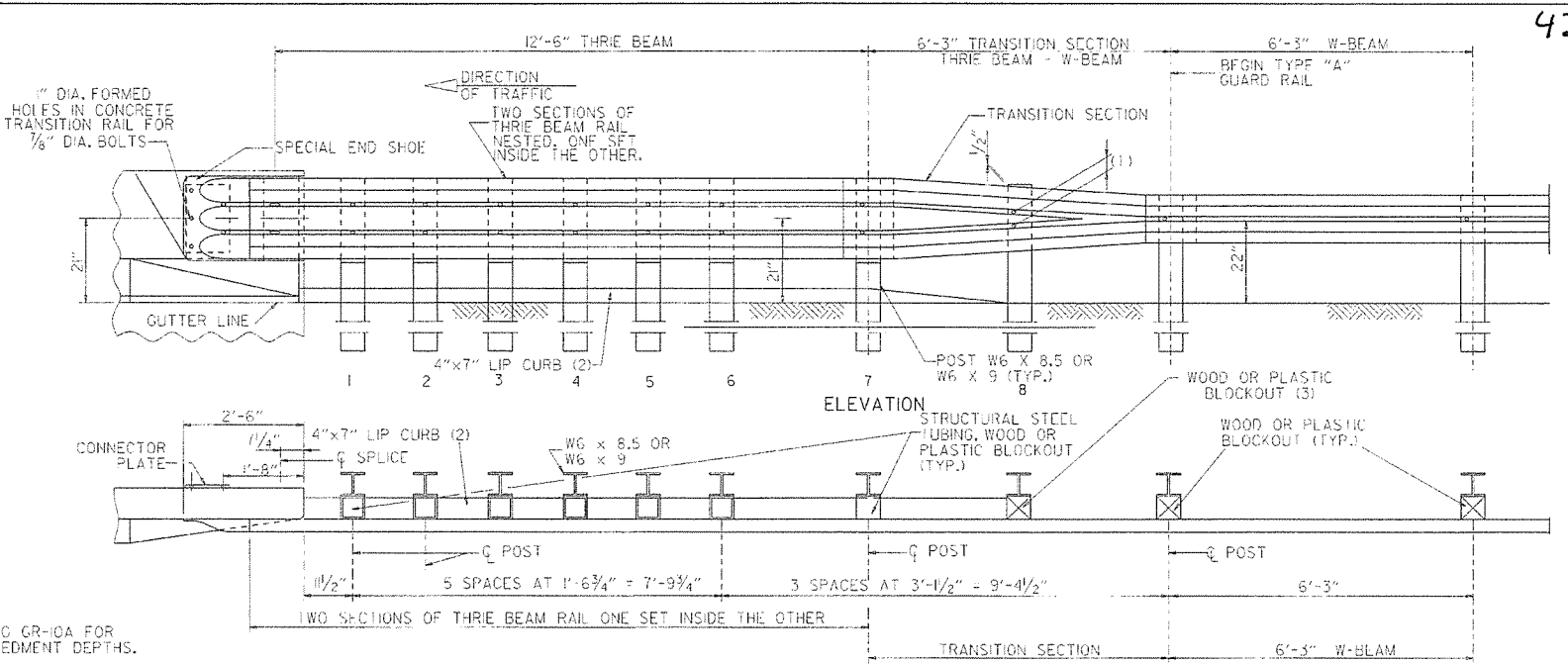
1" DIA. HOLES (TYP.) FOR 7/8" DIA. HIGH-STRENGTH BOLTS

NOTE: SEE STANDARD DRAWING GR-10A FOR GUARD RAIL POST EMBEDMENT DEPTHS.

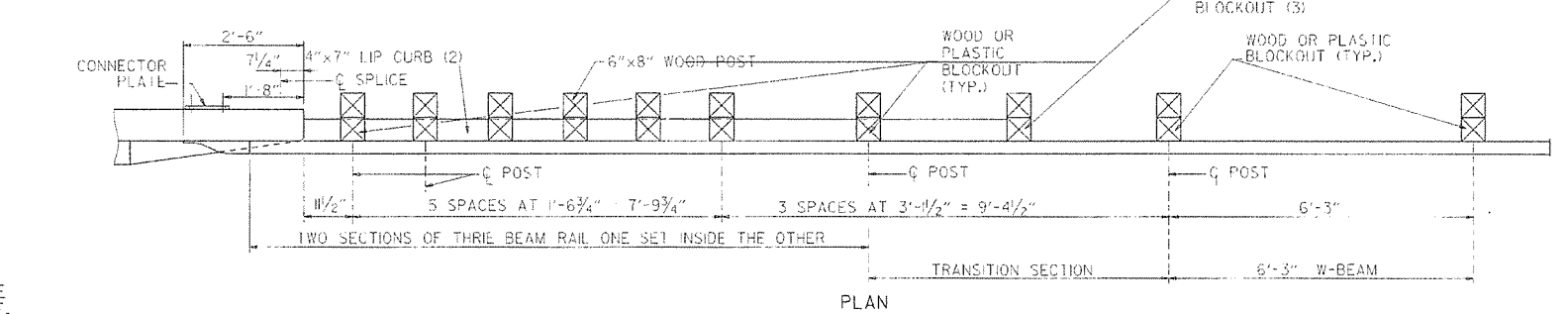


CONNECTOR PLATE

CONNECTOR PLATE SHALL BE AASHTO M270, GR. 36 AND SHALL BE GALVANIZED AFTER FABRICATION. GALVANIZING SHALL CONFORM TO SUBSECTION 807.19 OF THE STANDARD SPECIFICATIONS. CONNECTOR PLATE TO BE BOLTED TO SPECIAL END SHOE USING 5/8" DIA. HIGH STRENGTH BOLTS, WITH THE HEADS PLACED ON THE TRAFFIC FACE. WASHERS SHALL BE USED UNDER THE HEAD AND NUT. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED AND SHALL CONFORM TO SUBSECTION 807.06.



ELEVATION



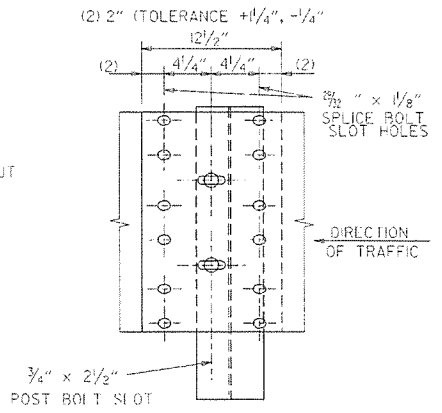
PLAN

- (1) VERIFY BOLT SPACING FROM RAIL TRANSITION PRODUCER.
- (2) REFER TO APPROACH GUTTER DETAILS.
- (3) LENGTH OF BLOCKOUT ON POST 8 TO BE MODIFIED TO FIT RAIL WIDTH.

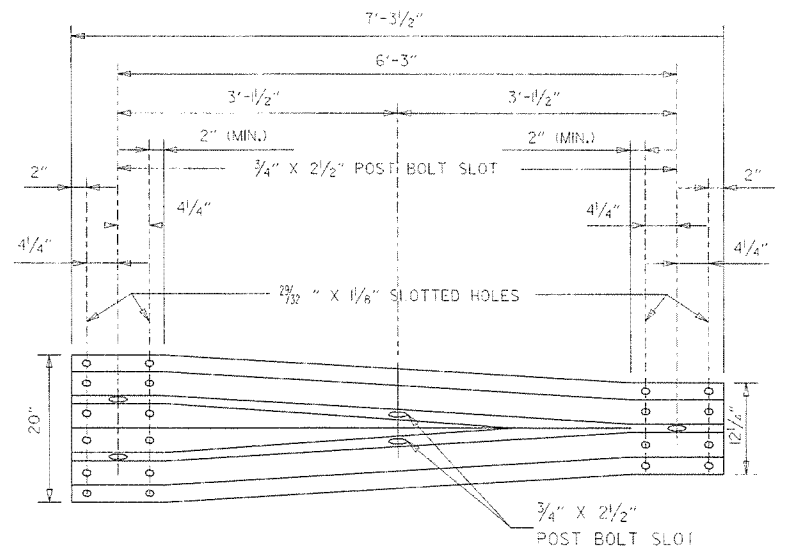
THRIE BEAM GUARD RAIL CONNECTION AT BRIDGE ENDS

GENERAL NOTES:

THE THRIE BEAM RAIL, SPECIAL END SHOE, AND THE TRANSITION SECTION SHALL BE MADE OF STEEL AND SHALL BE 12 GAGE. ZINC COATING SHALL BE TYPE 1. RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION. ALL BOLTS SHALL BE SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND NO MORE THAN 3/4" BEYOND IT. ALL LAP SPLICES, INCLUDING SPECIAL END SHOES, SHALL BE MADE IN THE DIRECTION SHOWN ON STANDARD DRAWINGS GR-9 & GR-11. WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. 1 STRUCTURAL OR BETTER 9.7F (1400 #) OR NO. 1 1350 F SOUTHERN PINE. REFER TO STD. DRWG. GR-10A FOR POST DETAILS. USE THRIE BEAM GUARD RAIL COMPONENTS OF SAME MATERIAL FOR ENTIRE JOB. THRIE BEAM POSTS SHALL BE SAME MATERIAL AS W BEAM POSTS FOR ENTIRE JOB.



THRIE BEAM RAIL SPLICE AT POST



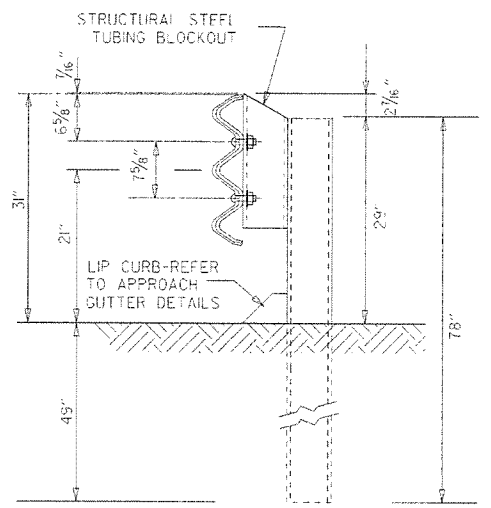
TRANSITION SECTION

DATE	REVISION	DATE	FILM
7-14-10	RAISED HEIGHT OF W-BEAM 1"		
11-29-07	ADDED PLASTIC BLOCKOUTS		
11-10-05	ADDED NOTE FOR ATTACHING STEEL BLOCKOUT		
11-18-04	REVISED GENERAL NOTES		
10-9-03	REVISED GENERAL NOTES		
4-10-03	REVISED GENERAL NOTES		
8-22-02	REVISED NOTE (2)		
6-29-00	MOVED DIMENSION LINES		
5-18-00	ADDED NOTE		
3-30-00	DRAWN & ISSUED		

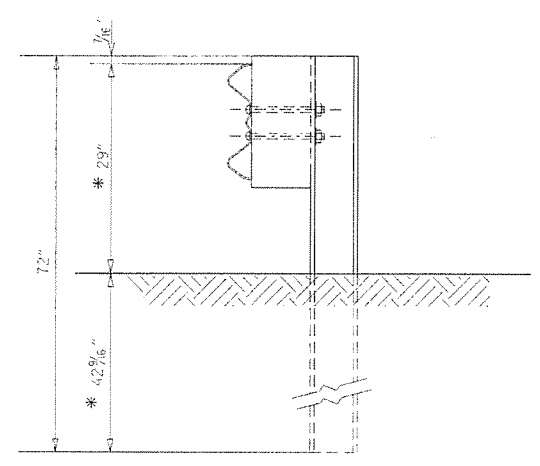
ARKANSAS STATE HIGHWAY COMMISSION

GUARD RAIL DETAILS

STANDARD DRAWING GR-10

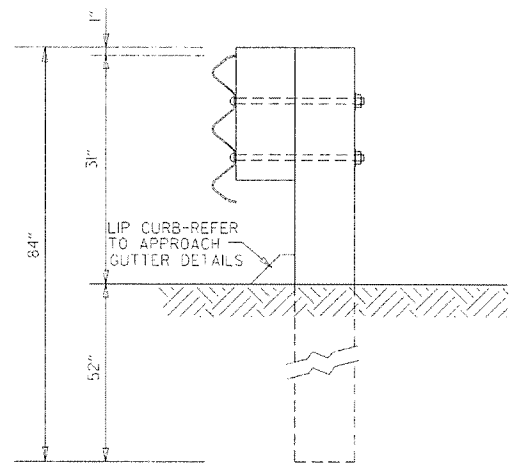


THRIE BEAM RAIL WITH STEEL TUBING BLOCKOUT AND STEEL POST
POSTS 1-7

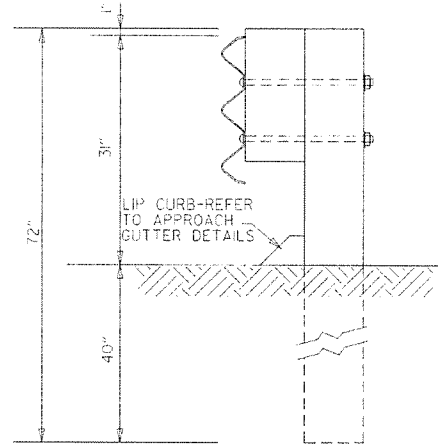


W-BEAM TO THRIE BEAM TRANSITION RAIL WITH WOOD OR PLASTIC BLOCKOUT AND STEEL POST
POST 8

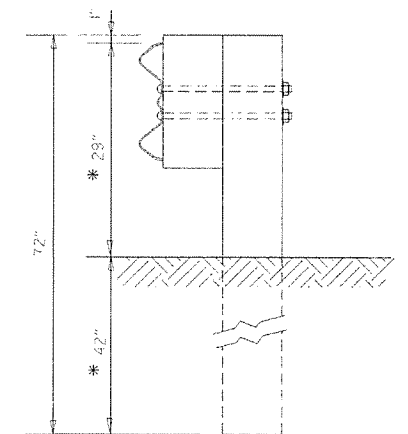
* NOTE:
THESE DIMENSIONS WILL NEED TO BE ADJUSTED IN THE FIELD TO MAKE THE TRANSITION FROM 21" MID POINT OF THRIE BEAM TO 22" MID POINT OF W-BEAM.



THRIE BEAM RAIL WITH WOOD OR PLASTIC BLOCKOUTS & WOOD POSTS
POSTS 1-6



THRIE BEAM RAIL WITH WOOD OR PLASTIC BLOCKOUT & WOOD POST
POST 7

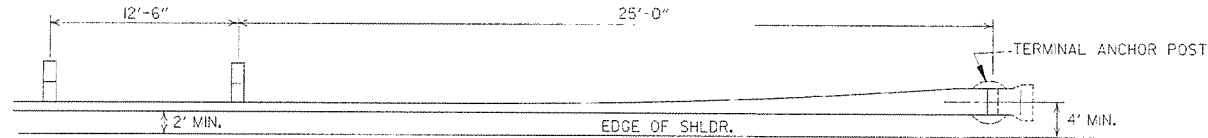


W-BEAM TO THRIE BEAM TRANSITION RAIL WITH WOOD OR PLASTIC BLOCKOUT & WOOD POST
POST 8

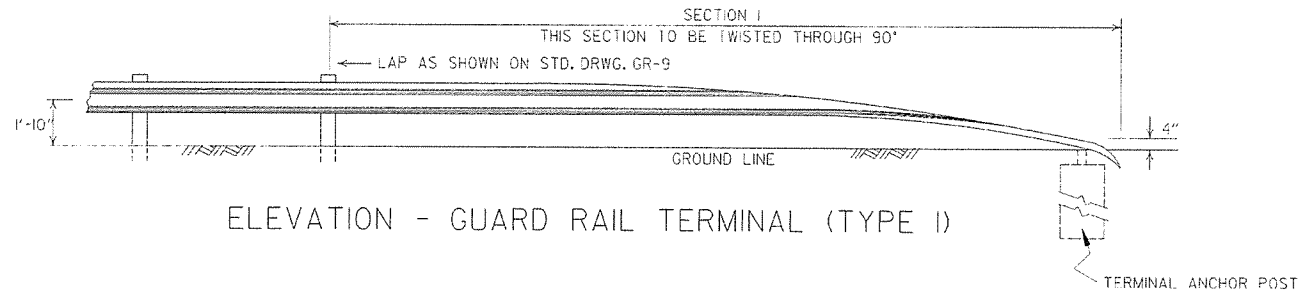
GENERAL NOTES:
RAIL POSTS SHALL BE SET PERPENDICULAR TO THE ROADWAY PROFILE GRADE AND VERTICALLY IN CROSS SECTION.
WOOD POSTS & WOOD BLOCKS SHALL BE EITHER DENSE NO. 1 STRUCTURAL OR BETTER 9.7F (400 F) OR NO. 1 (350 F) SOUTHERN PINE.

7-14-10	REVISED POST & DIMENSIONS	
11-29-07	ADDED PLASTIC BLOCKOUTS	
8-22-02	REVISED LIP CURB NOTE	
3-30-00	DRAWN & ISSUED	
DATE	REVISION	DATE FILM

ARKANSAS STATE HIGHWAY COMMISSION
GUARD RAIL DETAILS
STANDARD DRAWING GR-10A

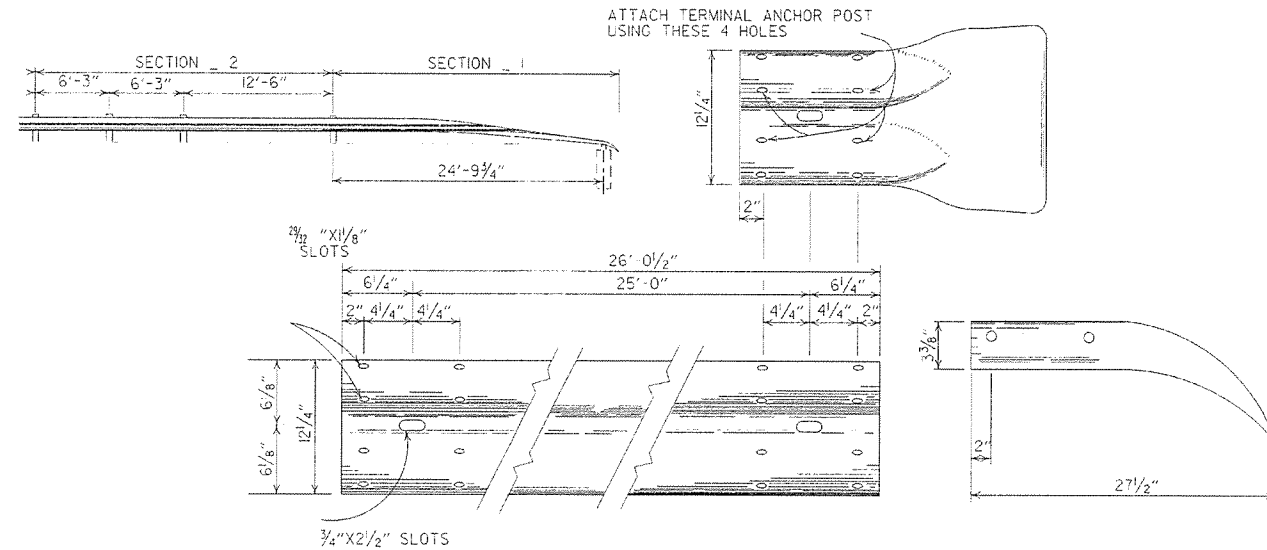


PLAN - GUARD RAIL TERMINAL (TYPE I)



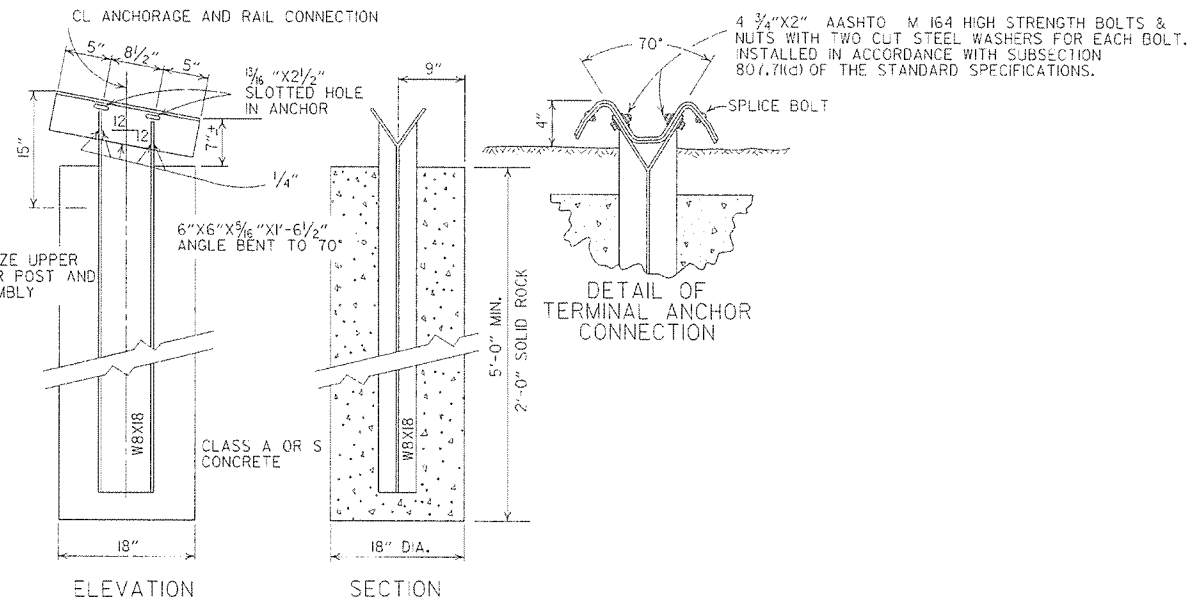
ELEVATION - GUARD RAIL TERMINAL (TYPE I)

NOTE:
SECTIONS 1 AND 2 OF GUARD RAIL TERMINAL SHALL BE PAID FOR AT THE PRICE BID PER LINEAR FOOT OF THE TYPE OF GUARD RAIL SPECIFIED.



SECTION 1

TERMINAL SECTION



ELEVATION

SECTION

DETAIL OF TERMINAL ANCHOR CONNECTION

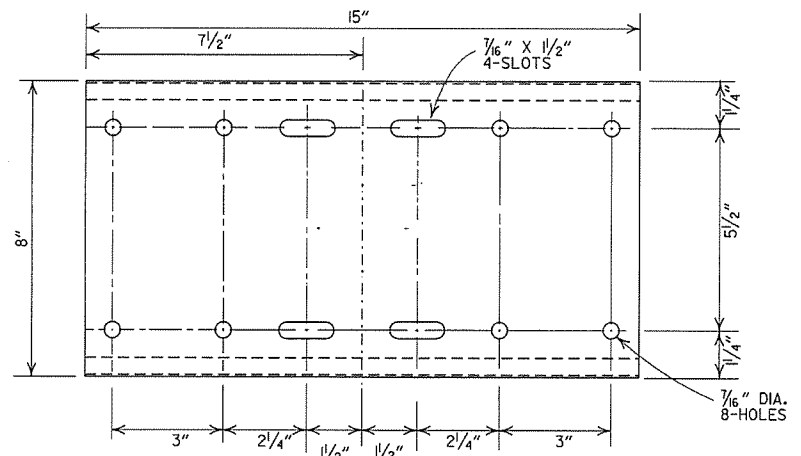
NOTE: GALVANIZE UPPER 15" OF ANCHOR POST AND ANCHOR ASSEMBLY

4 3/4" X 2" AASHTO M 164 HIGH STRENGTH BOLTS & NUTS WITH TWO CUT STEEL WASHERS FOR EACH BOLT. INSTALLED IN ACCORDANCE WITH SUBSECTION 807.7(K) OF THE STANDARD SPECIFICATIONS.

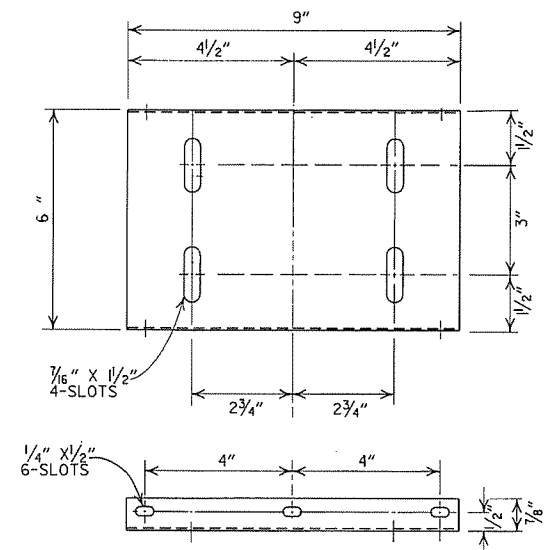
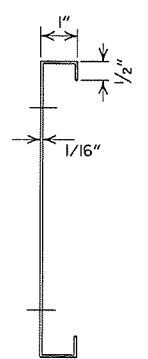
NOTE: RAIL MEMBERS MAY BE BOLTED TO ANGLE AT TERMINAL ANCHOR AND THE TWO ASSEMBLIES POSITIONED TO PROPER ALIGNMENT PRIOR TO PLACING CONCRETE AROUND 8 W/ 17 POST IF CONTRACTOR SO DESIRES.

DETAIL OF TERMINAL ANCHOR POST (TYPE I)

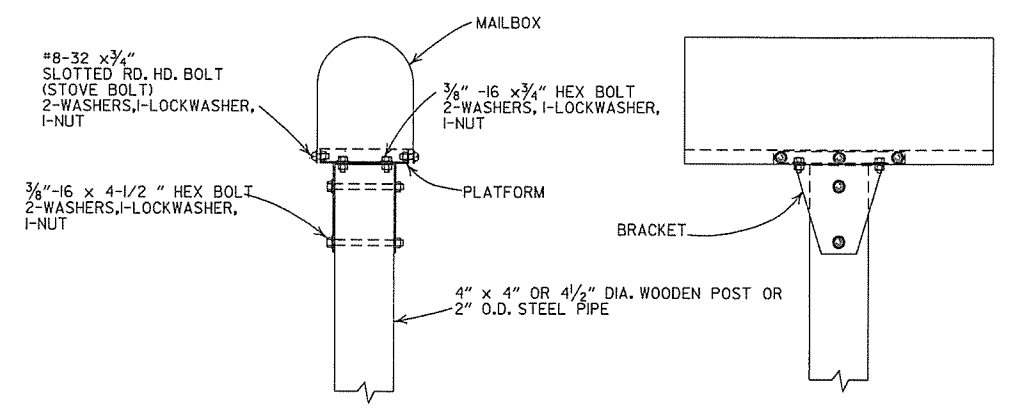
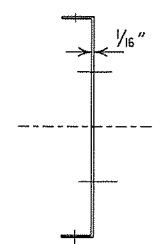
		ARKANSAS STATE HIGHWAY COMMISSION
		GUARD RAIL DETAILS
		STANDARD DRAWING GRT-1
7-14-10	RAISED HEIGHT OF GUARD RAIL 1"	
6-26-97	REVISED LAP NOTE	
10-18-96	REVISED ASTM REF. TO AASHTO	
11-3-94	DIMENSION TERMINAL DETAIL	
11-11-92	ADDED NOTE FOR PAYMENT	11-11-92
10-1-92	DRAWN & ISSUED	10-1-92
DATE	REVISION	DATE FILM



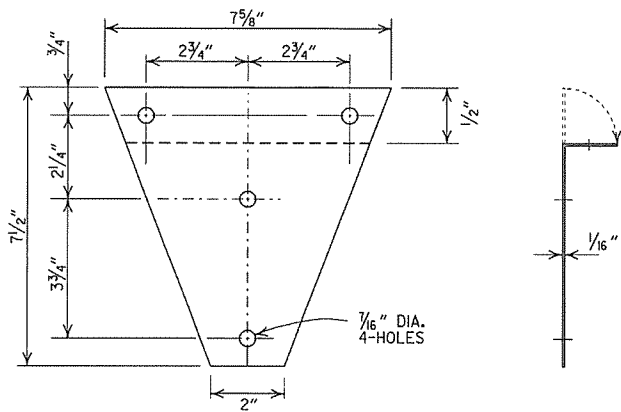
SHELF



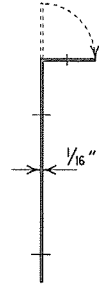
PLATFORM



SINGLE INSTALLATION

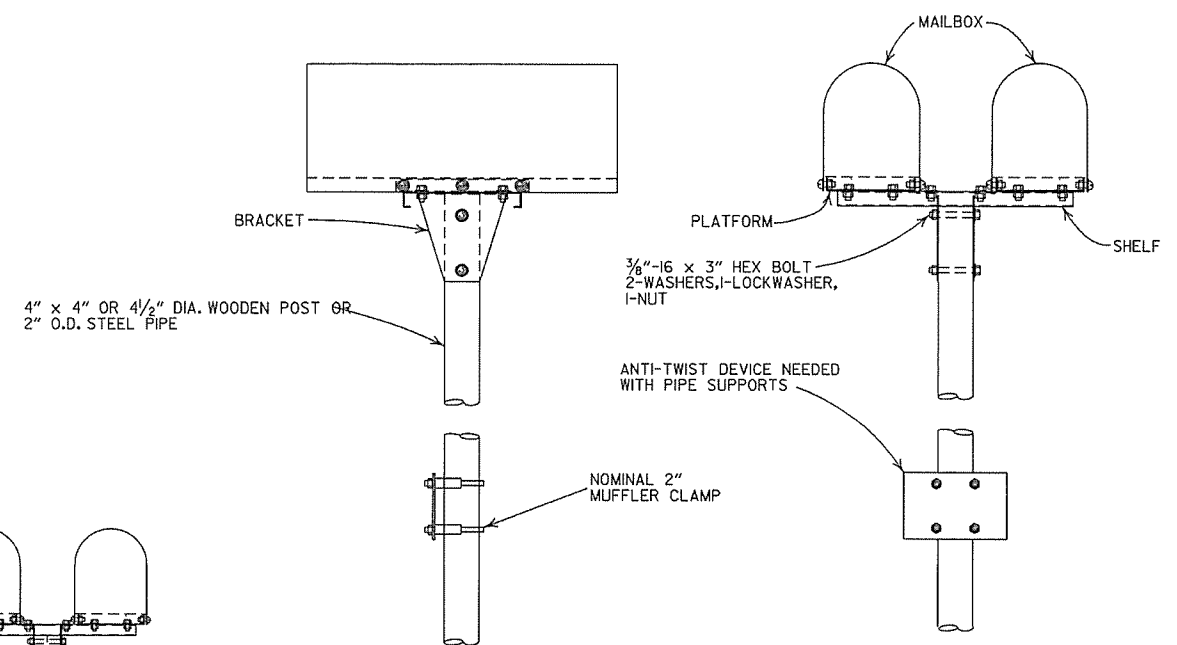


BRACKET

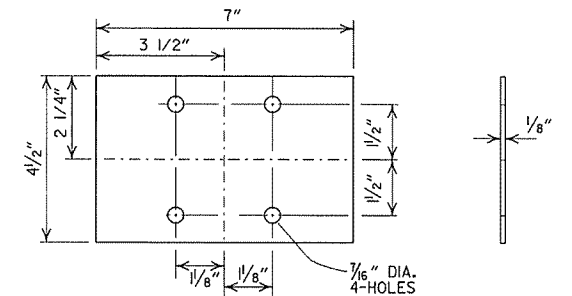


GENERAL NOTES

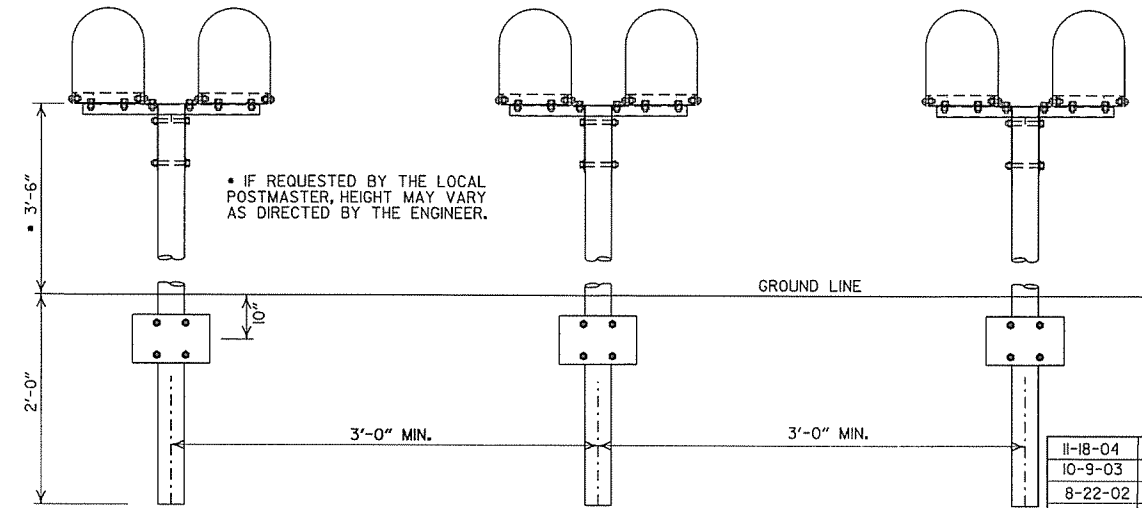
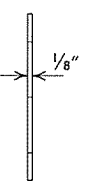
1. MAILBOX POSTS MAY BE WOOD OR METAL. WOOD POSTS SHALL BE PRESSURE TREATED FOR GROUND CONTACT IN ACCORDANCE WITH SECTION 637.02 OF THE STANDARD SPECIFICATIONS.
2. ANTI-TWIST PLATES SHALL BE USED ONLY ON METAL POSTS.
3. MAILBOX SHELF, BRACKET & PLATFORM SHALL BE GALVANIZED OR PAINTED STEEL, HOWEVER TREATED WOOD MAY BE USED WITH WOODEN POSTS. THE WOODEN SHELF, BRACKET & PLATFORM SHALL BE A MINIMUM OF 3/4" THICK AND SHALL BE ASSEMBLED WITH BOLTS OF THE APPROPRIATE LENGTH WITH SIX 8 X 3/4" FLATHEAD WOOD SCREWS USED TO ATTACH THE MAILBOX TO THE PLATFORM.
4. THE MAILBOX SHELF AND PLATFORM THAT IS SHOWN IS FOR STANDARD SIZE MAILBOXES. THE SHELF AND PLATFORM SIZE SHALL BE MODIFIED TO FIT MAILBOXES OF A DIFFERENT SIZE.
5. METAL PIPE FOR MAILBOX SUPPORT SHALL BE 2" OUTSIDE DIAMETER STEEL WITH A WALL THICKNESS OF 0.145" AND A WEIGHT OF 2.72 LBS PER FT. OUTSIDE DIAMETER AND WEIGHT SHALL HAVE A TOLERANCE OF +/- 5% ACCORDING TO AASHTO M 181.
6. MAILBOX SUPPORT SYSTEM DIFFERING FROM THOSE SHOWN MAY BE USED, PROVIDED THEY ARE ON THE AHTD QUALIFIED PRODUCTS LIST FOR MAILBOX SUPPORTS.



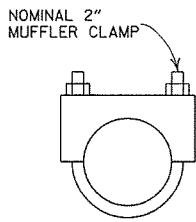
DOUBLE INSTALLATION



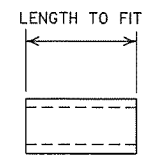
ANTI-TWIST PLATE



SPACING FOR MULTIPLE POST INSTALLATION



CLAMP



SPACER

DATE	FILED	REVISION
11-18-04		REVISED NOTES
10-9-03		REVISED NOTE 6
8-22-02		REVISED NOTE 6
10-18-96		CORRECTED AASHTO
10-1-92		CORRECTED SPELLING
9-26-91		NEW PHONE NUMBER
8-15-91		ADDED NOTE
11-30-89		ADJUSTED HEIGHT & ADDED NOTE
2-16-89		DELETED SLOTS FROM SHELF & PLTF
11-17-88	10-1-92	ADJUSTED DIMENSIONS OF STEEL POSTS
7-15-88	120-7-15-88	ISSUED
		FILMED
		REVISION

ARKANSAS STATE HIGHWAY COMMISSION

MAILBOX DETAILS

STANDARD DRAWING MB-1

REINFORCED CONCRETE ARCH PIPE DIMENSIONS

EQUIV. DIA.	SPAN		RISE	
	AASHTO M 206	AHTD NOMINAL	AASHTO M 206	AHTD NOMINAL
INCHES	INCHES			
15	18	18	11	11
18	22	22	13½	14
21	26	26	15½	16
24	28½	29	18	18
30	36¼	36	22½	23
36	43¾	44	26¾	27
42	51⅞	51	31⅞	31
48	58½	59	36	36
54	65	65	40	40
60	73	73	45	45
72	88	88	54	54
84	102	102	62	62
90	115	115	72	72
96	122	122	77½	77
108	138	138	87⅞	87
120	154	154	96⅞	97
132	168¾	169	106½	107

THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN ± 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M206.

REINFORCED CONCRETE HORIZONTAL ELLIPTICAL PIPE DIMENSIONS

EQUIV. DIA.	AASHTO M 207	
	SPAN	RISE
INCHES	INCHES	
18	23	14
24	30	19
27	34	22
30	38	24
33	42	27
36	45	29
39	49	32
42	53	34
48	60	38
54	68	43
60	76	48
66	83	53
72	91	58
78	98	63
84	106	68

THE MEASURED SPAN AND RISE SHALL NOT VARY MORE THAN ± 2 PERCENT FROM THE VALUES SPECIFIED BY AASHTO M207.

CONSTRUCTION SEQUENCE

1. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
2. INSTALL PIPE TO GRADE.
3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
4. PLACE AND COMPACT THE HAUNCH AREA UP TO THE MIDDLE OF THE PIPE.
5. COMPLETE BACKFILL ACCORDING TO SUBSECTION 606.03.(f)(i).

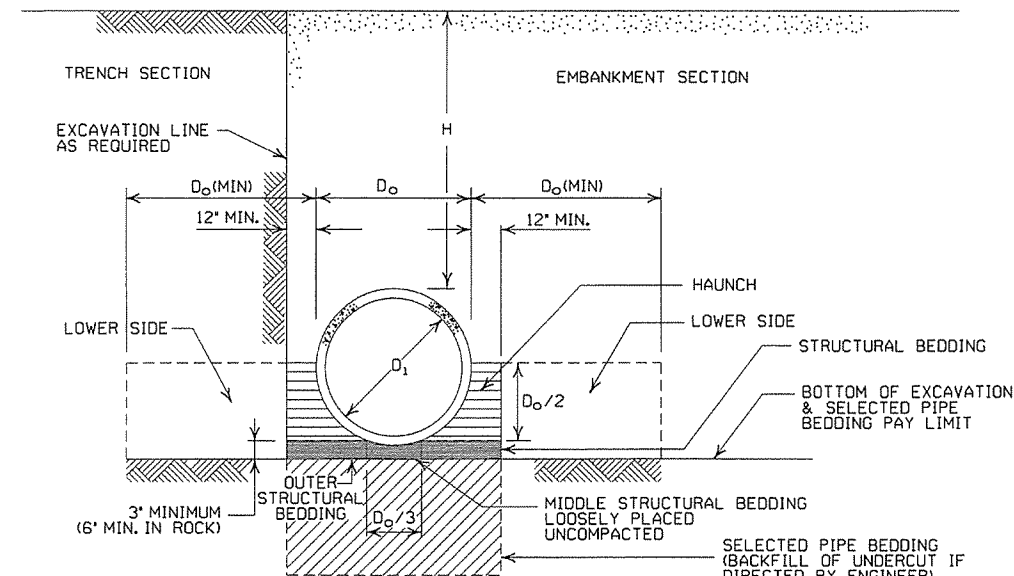
NOTE: HAUNCH AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF CONCRETE PIPE.

- LEGEND -

- D_i = NORMAL INSIDE DIAMETER OF PIPE
- D_o = OUTSIDE DIAMETER OF PIPE
- H = FILL COVER HEIGHT OVER PIPE (FEET)
- MIN. = MINIMUM
- UNDISTURBED SOIL

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR HAUNCH AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 5 OR CLASS 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL*
TYPE 3**	AASHTO CLASSIFICATION A-1 THRU A-6 SOIL OR TYPE 1 OR 2 INSTALLATION MATERIAL

* SM-3 WILL NOT BE ALLOWED.
** MATERIALS SHALL NOT INCLUDE ORGANIC MATERIALS OR STONES LARGER THAN 3 INCHES.



EMBANKMENT AND TRENCH INSTALLATIONS

1. MATERIAL IN THE HAUNCH AND OUTER STRUCTURAL BEDDING SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
2. FOR TRENCHES WITH WALLS OF NATURAL SOIL, THE DENSITY OF THE SOIL IN THE LOWER SIDE ZONE SHALL BE AS FIRM AS THE 95% DENSITY REQUIRED FOR THE HAUNCH. IF THE EXISTING SOIL DOES NOT MEET THIS CRITERIA, IT SHALL BE REMOVED AND RECOMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OF MATERIAL USED.
3. FOR EMBANKMENTS, THE MATERIAL IN THE LOWER SIDE ZONE SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

GENERAL NOTES

1. CONCRETE PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS. UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
2. CONCRETE PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
3. ALL PIPE SHALL CONFORM TO SECTION 606. CIRCULAR R.C. PIPE CULVERTS SHALL CONFORM TO AASHTO M170. R.C. ARCH PIPE CULVERTS SHALL CONFORM TO AASHTO M206 AND HORIZONTAL ELLIPTICAL PIPE CULVERTS SHALL CONFORM TO AASHTO M207.
4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR WORKING CONDITIONS.
6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
8. NOT MORE THAN ONE LIFTING HOLE MAY BE PROVIDED IN CONCRETE PIPE TO FACILITATE HANDLING. HOLE MAY BE CAST IN PLACE, CUT INTO THE FRESH CONCRETE AFTER FORMS ARE REMOVED, OR DRILLED. THE HOLE SHALL NOT BE MORE THAN TWO INCHES IN DIAMETER OR TWO INCHES SQUARE. CUTTING OR DISPLACEMENT OF REINFORCEMENT WILL NOT BE PERMITTED. SPALLED AREAS AROUND THE HOLE SHALL BE REPAIRED IN A WORKMANLIKE MANNER. LIFTING HOLE SHALL BE FILLED WITH MORTAR, CONCRETE, OR OTHER METHOD AS APPROVED BY THE ENGINEER.
9. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
10. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS THE HAUNCH), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."

MINIMUM HEIGHT OF FILL "H" OVER CIRCULAR R.C. PIPE CULVERTS

INSTALLATION TYPE	CLASS OF PIPE			
	CLASS III		CLASS IV	CLASS V
PIPE ID (IN.)	FEET			
12-15	2	2.5	2	1
18-24	2.5	3	2	1
27-33	3	4	2	1
36-42	3.5	5	2	1
48	4.5	5.5	2	1
54-60	5	7	2	1
66-78	6	8	2	1
84-108	7.5	8	2	1

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MAXIMUM HEIGHT OF FILL "H" OVER CIRCULAR R.C. PIPE CULVERTS

INSTALLATION TYPE	CLASS OF PIPE		
	CLASS III	CLASS IV	CLASS V
TYPE 1	21	32	50
TYPE 2	16	25	39
TYPE 3	12	20	30

NOTE: IF FILL HEIGHT EXCEEDS 50 FEET, A SPECIAL DESIGN CONCRETE PIPE WILL BE REQUIRED USING TYPE 1 INSTALLATION.

MINIMUM HEIGHT OF FILL "H" OVER R.C. ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS

INSTALLATION TYPE	CLASS OF PIPE	
	CLASS III	CLASS IV
TYPE 2 OR TYPE 3	FEET	
	2.5	1.5

NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.

NOTE: FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM OF 12" OF PAVEMENT AND/OR BASE.

MAXIMUM HEIGHT OF FILL "H" OVER R.C. ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS

INSTALLATION TYPE	CLASS OF PIPE	
	CLASS III	CLASS IV
TYPE 2	13	21
TYPE 3	10	16


NOTE: TYPE 1 INSTALLATION WILL NOT BE ALLOWED FOR ARCH & HORIZONTAL ELLIPTICAL PIPE CULVERTS.

DATE	REVISION	DATE FILMED
2-27-14	REVISED GENERAL NOTE 1	
12-15-11	REVISED FOR LRFD DESIGN SPECIFICATIONS	
5-18-00	REVISED TYPE 3 BEDDING & ADDED NOTE	
3-30-00	REVISED INSTALLATIONS	
11-06-97	ISSUED	

ARKANSAS STATE HIGHWAY COMMISSION

CONCRETE PIPE CULVERT
FILL HEIGHTS & BEDDING

STANDARD DRAWING PCC-1



CORRUGATED STEEL PIPE (ROUND)

PIPE DIAMETER (INCHES)	① MINIMUM COVER TOP OF PIPE TO TOP OF GROUND "H" (FEET)	MAX. FILL HEIGHT "H" ABOVE TOP OF PIPE (FEET)				
		METAL THICKNESS (INCHES)				
		0.064	0.079	0.109	0.138	0.168
2 3/8 INCH BY 1/2 INCH CORRUGATION RIVETED, WELDED, OR HELICAL LOCK-SEAM						
12	1	84	91			
15	1	67	73			
18	1	56	61			
24	1	42	46	59		
30	2	34	36	47		
36	2		30	39	41	
42	2		43	67	70	73
48	2		37	58	61	64
② 3 INCH BY 1 INCH OR 5 INCH BY 1 INCH CORRUGATION RIVETED, WELDED, BOLTED, OR HELICAL LOCK-SEAM						
36	1	48	60	88	111	118
42	1	41	51	72	90	102
48	1	36	45	64	77	85
54	2	32	40	59	71	79
60	2	29	36	53	64	71
66	2	26	33	47	58	64
72	2	24	30	44	53	59
78	2		28	41	49	54
84	2		26	38	45	51
90	2		24	35	43	45
96	2		22	33	40	44
102	2			31	38	42
108	2			30	35	39
114	2			28	34	37
120	2			27	32	35

CONSTRUCTION SEQUENCE

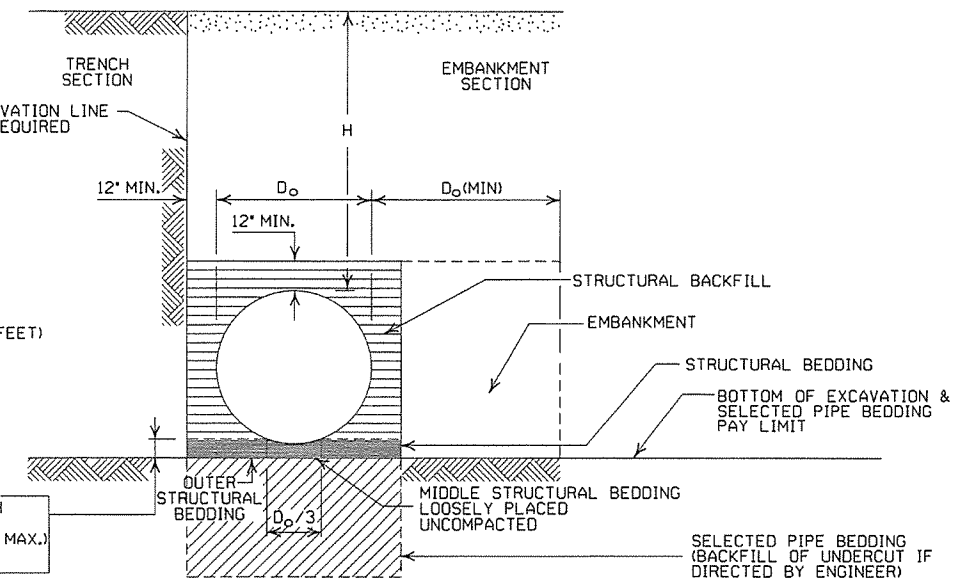
1. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
2. INSTALL PIPE TO GRADE.
3. COMPLETE STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
4. COMPLETE STRUCTURAL BACKFILL OPERATION BY WORKING FROM SIDE TO SIDE OF THE PIPE. THE SIDE TO SIDE STRUCTURAL BACKFILL DIFFERENTIAL SHALL NOT EXCEED 24 INCHES OR 1/3 THE SIZE OF THE PIPE, WHICHEVER IS LESS.

NOTE: STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF METAL PIPE.

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 1	AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7)
TYPE 2	SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4) OR TYPE 1 INSTALLATION MATERIAL ③

③ SM-3 WILL NOT BE ALLOWED.

- LEGEND -
- D_o = OUTSIDE DIAMETER OF PIPE
 - MAX. = MAXIMUM
 - MIN. = MINIMUM
 - [Hatched Pattern] = STRUCTURAL BACKFILL MATERIAL
 - [Dotted Pattern] = UNDISTURBED SOIL
 - [Diagonal Pattern] = EQUIV. DIA. = EQUIVALENT DIAMETER
 - H = FILL COVER HEIGHT OVER PIPE (FEET)



EMBANKMENT AND TRENCH INSTALLATIONS

1. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.
2. INSTALLATION TYPE 1 OR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE (ROUND).
3. INSTALLATION TYPE 1 SHALL BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 2 3/8" X 1/2" CORRUGATION.
4. INSTALLATION TYPE 1 OR 2 MAY BE USED FOR CORRUGATED STEEL OR ALUMINUM PIPE ARCHES WITH 3" X 1" OR 5" X 1" CORRUGATION.

CORRUGATED ALUMINUM PIPE (ROUND)

PIPE DIAMETER (INCHES)	① MINIMUM COVER TOP OF PIPE TO TOP OF GROUND "H" (FEET)	MAX. FILL HEIGHT "H" ABOVE TOP OF PIPE (FEET)				
		METAL THICKNESS IN INCHES				
		0.060	0.075	0.105	0.135	0.164
2 3/8 INCH BY 1/2 INCH CORRUGATION RIVETED OR HELICAL LOCK-SEAM						
12	1	45	45	52		
18	2	30	30	31		
24	2	22	22	39	41	
30	2		18	31	32	34
36	2.5		15	26	27	28
42	2			43	43	44
48	2			40	41	43
54	2			35	37	38
60	2				33	34
66	2					31
72	2					29

EQUIVALENT METAL THICKNESSES AND GAUGES

METAL THICKNESS IN INCHES			GAUGE NUMBER	
STEEL				
ZINC COATED	UNCOATED	ALUMINUM		
0.064	0.0598	0.060		16
0.079	0.0747	0.075		14
0.109	0.1046	0.105		12
0.138	0.1345	0.135		10
0.168	0.1644	0.164		8

GENERAL NOTES

1. METAL PIPE CULVERT CONSTRUCTION SHALL CONFORM TO ARKANSAS STATE HIGHWAY AND TRANSPORTATION DEPARTMENT STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION), WITH APPLICABLE SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS, UNLESS OTHERWISE NOTED IN THE PLANS, SECTION AND SUBSECTION REFER TO THE STANDARD CONSTRUCTION SPECIFICATIONS.
2. METAL PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
3. METAL PIPE CULVERT MATERIALS AND INSTALLATIONS SHALL CONFORM TO SECTION 606 AND JOB SPECIAL PROVISION "METAL PIPE".
4. ALL PIPE SHALL BE PROTECTED DURING CONSTRUCTION BY A COVER SUFFICIENT TO PREVENT DAMAGE FROM PASSAGE OF EQUIPMENT.
5. THE MINIMUM TRENCH WIDTH SHALL BE THE OUTSIDE DIAMETER OF THE PIPE PLUS 24 INCHES. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PRACTICABLE FOR WORKING CONDITIONS.
6. MULTIPLE PIPE CULVERTS SHALL BE INSTALLED WITH A MINIMUM CLEARANCE OF 24 INCHES BETWEEN STRINGS OF PIPE. REFER TO STD. DWG. FES-2 FOR MINIMUM CLEARANCE WHERE FLARED END SECTIONS ARE USED.
7. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
8. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
9. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE. IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."

CORRUGATED METAL PIPE ARCHES

EQUIV. DIA. (INCHES)	PIPE DIMENSION SPAN X RISE (INCHES)	MINIMUM CORNER RADIUS (INCHES)	STEEL				ALUMINUM			
			MIN. THICKNESS REQUIRED INCHES	① MIN. HEIGHT OF FILL, "H" (FT.)		MIN. THICKNESS REQUIRED INCHES	① MIN. HEIGHT OF FILL, "H" (FT.)			
				INSTALLATION	INSTALLATION		INSTALLATION	INSTALLATION		
			TYPE 1	TYPE 1	TYPE 1	TYPE 1				
2 3/8 INCH BY 1/2 INCH CORRUGATION RIVETED, WELDED, OR HELICAL LOCK-SEAM										
15	17x13	3	0.064	2	15	0.060	2	15		
18	21x15	3	0.064	2	15	0.060	2	15		
21	24x18	3	0.064	2,25	15	0.060	2,25	15		
24	28x20	3	0.064	2,5	15	0.075	2,5	15		
30	35x24	3	0.079	3	12	0.075	3	12		
36	42x29	3 1/2	0.079	3	12	0.105	3	12		
42	49x33	4	0.079	3	12	0.105	3	12		
48	57x38	5	0.109	3	13	0.135	3	13		
54	64x43	6	0.109	3	14	0.135	3	14		
60	71x47	7	0.138	3	15	0.164	3	15		
66	77x52	8	0.168	3	15					
72	83x57	9	0.168	3	15					
② 3 INCH BY 1 INCH OR 5 INCH BY 1 INCH CORRUGATION RIVETED, WELDED, OR HELICAL LOCK-SEAM										
			INSTALLATION		INSTALLATION					
			TYPE 2	TYPE 1	TYPE 2	TYPE 1				
36	40x31	5	0.079	3	2	12	15			
42	46x36	6	0.079	3	2	13	15			
48	53x41	7	0.079	3	2	13	15			
54	60x46	8	0.079	3	2	13	15			
60	66x51	9	0.079	3	2	13	15			
66	73x55	12	0.079	3	2	15	15			
72	81x59	14	0.079	3	2	15	15			
78	87x63	14	0.079	3	2	15	15			
84	95x67	16	0.109	3	2	15	15			
90	103x71	16	0.109	3	2	15	15			
96	112x75	18	0.109	3	2	15	15			
102	117x79	18	0.109	3	2	15	15			
108	128x83	18	0.138	3	2	15	15			

- ① FOR MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.
- ② WHERE THE STANDARD 2 2/3" X 1/2" CORRUGATION AND GAUGE IS SPECIFIED FOR A GIVEN DIAMETER, A PIPE OF THE SAME DIAMETER WITH A 3" X 1" OR 5" X 1" CORRUGATION MAY BE SUBSTITUTED, PROVIDING IT IS GAUGED FOR A FILL HEIGHT CONDITION EQUAL TO OR GREATER THAN THE MAXIMUM FILL HEIGHT CONDITION FOR THE SPECIFIED GAUGE AND CORRUGATION.

ARKANSAS STATE HIGHWAY COMMISSION		
METAL PIPE CULVERT FILL HEIGHTS & BEDDING		
STANDARD DRAWING PCM-1		
DATE	REVISION	DATE FILMED
2-27-14	REVISED GENERAL NOTE 1	
12-15-11	REVISED FOR LRFD DESIGN SPECS	
3-30-00	REVISED INSTALLATIONS	
11-06-97	ISSUED	

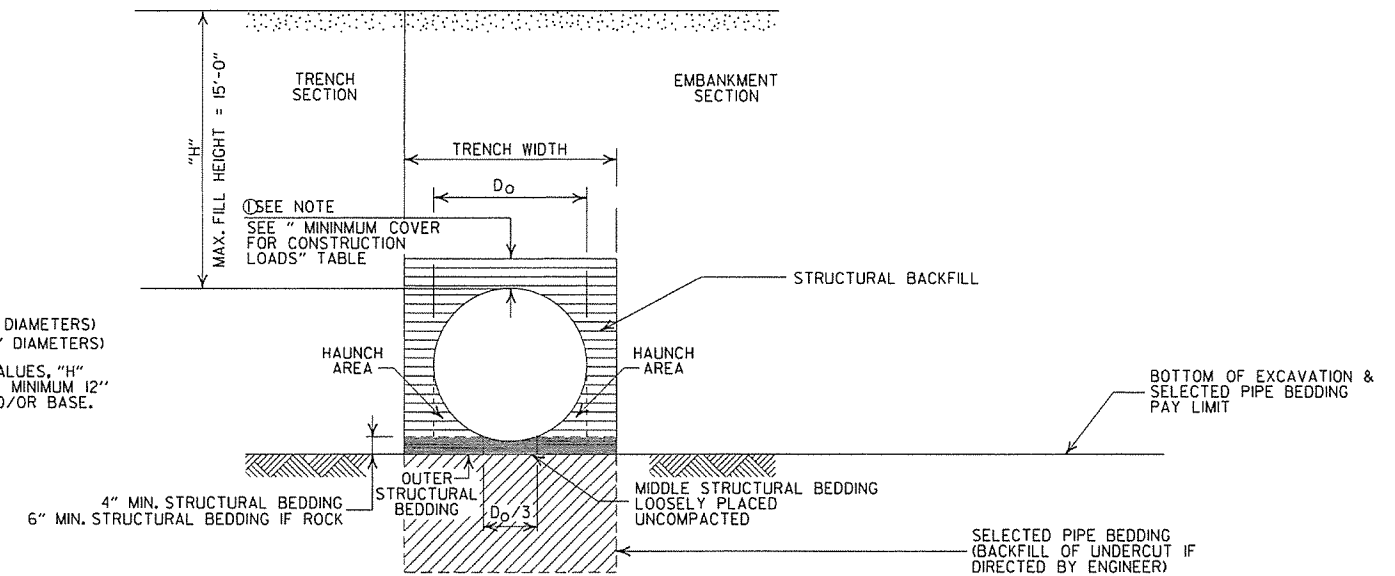
INSTALLATION TYPE	** MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 2	•SELECTED MATERIALS (CLASS SM-1, SM-2 OR SM-4)

- AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7) MAY BE USED IN LIEU OF SELECTED MATERIAL.
- SM3 WILL NOT BE ALLOWED.
- STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF 1/4 INCH. STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.
- STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF HDPE PIPE.

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

PIPE DIAMETER	TRENCH WIDTH (FEET)	
	"H" < 10'-0"	"H" > OR = 10'-0"
18"	4'-6"	4'-6"
24"	5'-0"	6'-0"
30"	5'-6"	7'-6"
36"	6'-0"	9'-0"
42"	7'-0"	10'-6"
48"	8'-0"	12'-0"

NOTE:
 18" MIN. (18" - 30" DIAMETERS)
 24" MIN. (36" - 48" DIAMETERS)
 MINIMUM COVER VALUES, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.



TYPE 2 EMBANKMENT AND TRENCH INSTALLATIONS
 I. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

MULTIPLE INSTALLATION OF HIGH DENSITY POLYETHYLENE PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	1'-6"
24"	2'-0"
30"	2'-6"
36"	3'-0"
42"	3'-6"
48"	4'-0"

MINIMUM COVER FOR CONSTRUCTION LOADS

PIPE DIAMETER	MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-110.0 (KIPS)	110.0-175.0 (KIPS)
36" OR LESS	2'-0"	2'-6"	3'-0"	3'-0"
42" OR GREATER	3'-0"	3'-0"	3'-6"	4'-0"

MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.

CONSTRUCTION SEQUENCE

1. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
2. INSTALL PIPE TO GRADE.
3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND ALIGNMENT.

GENERAL NOTES

1. PIPE SHALL CONFORM TO AASHTO M294, TYPE S. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE, IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
8. HIGH DENSITY POLYETHYLENE PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
9. JOINTS FOR HDPE PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN AASHTO SECTION 26.4.2.4 AND 30.4.2 "AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS." JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.

- LEGEND -

- H = FILL HEIGHT (FT.)
- D_o = OUTSIDE DIAMETER OF PIPE
- MAX. = MAXIMUM
- MIN. = MINIMUM
- ==== = STRUCTURAL BACKFILL MATERIAL
- ===== = UNDISTURBED SOIL

DATE	REVISION	DATE FILMED
2-27-14	REVISED GENERAL NOTE I.	
12-15-11	REVISED GENERAL NOTES & MINIMUM COVER NOTE	
11-17-10	ISSUED	

ARKANSAS STATE HIGHWAY COMMISSION

PLASTIC PIPE CULVERT
(HIGH DENSITY POLYETHYLENE)

STANDARD DRAWING PCP-1

INSTALLATION TYPE	MATERIAL REQUIREMENTS FOR STRUCTURAL BACKFILL AND STRUCTURAL BEDDING
TYPE 2	•SELECTED MATERIALS (CLASS SM-1, SM-2, OR SM-4)

- AGGREGATE BASE COURSE (CLASS 4, 5, 6, OR 7) MAY BE USED IN LIEU OF SELECTED MATERIAL. SM3 WILL NOT BE ALLOWED.
- STRUCTURAL BEDDING MATERIAL SHALL HAVE A MAXIMUM PARTICLE SIZE OF 1/2 INCH. STRUCTURAL BACKFILL MATERIAL SHALL BE FREE OF ORGANIC MATERIAL, STONES LARGER THAN 1.50 INCH IN GREATEST DIMENSION, OR FROZEN LUMPS.

STRUCTURAL BACKFILL AND STRUCTURAL BEDDING MATERIAL WILL NOT BE PAID FOR SEPARATELY, BUT COMPENSATION WILL BE CONSIDERED TO BE INCLUDED IN THE PRICE BID PER LINEAR FOOT OF PVC PIPE.

MAXIMUM FILL HEIGHT BASED ON STRUCTURAL BACKFILL

PIPE DIAMETER	"H"
18"	45'-0"
24"	45'-0"
30"	40'-0"
36"	40'-0"

① NOTE: 12" MIN. (18" - 36" DIAMETERS) MINIMUM COVER VALUE, "H" SHALL INCLUDE A MINIMUM 12" OF PAVEMENT AND/OR BASE.

MINIMUM TRENCH WIDTH BASED ON FILL HEIGHT "H"

PIPE DIAMETER	TRENCH WIDTH (FEET)	
	"H" < 10'-0"	"H" > OR = 10'-0"
18"	4'-6"	4'-6"
24"	5'-0"	6'-0"
30"	5'-6"	7'-6"
36"	6'-0"	9'-0"

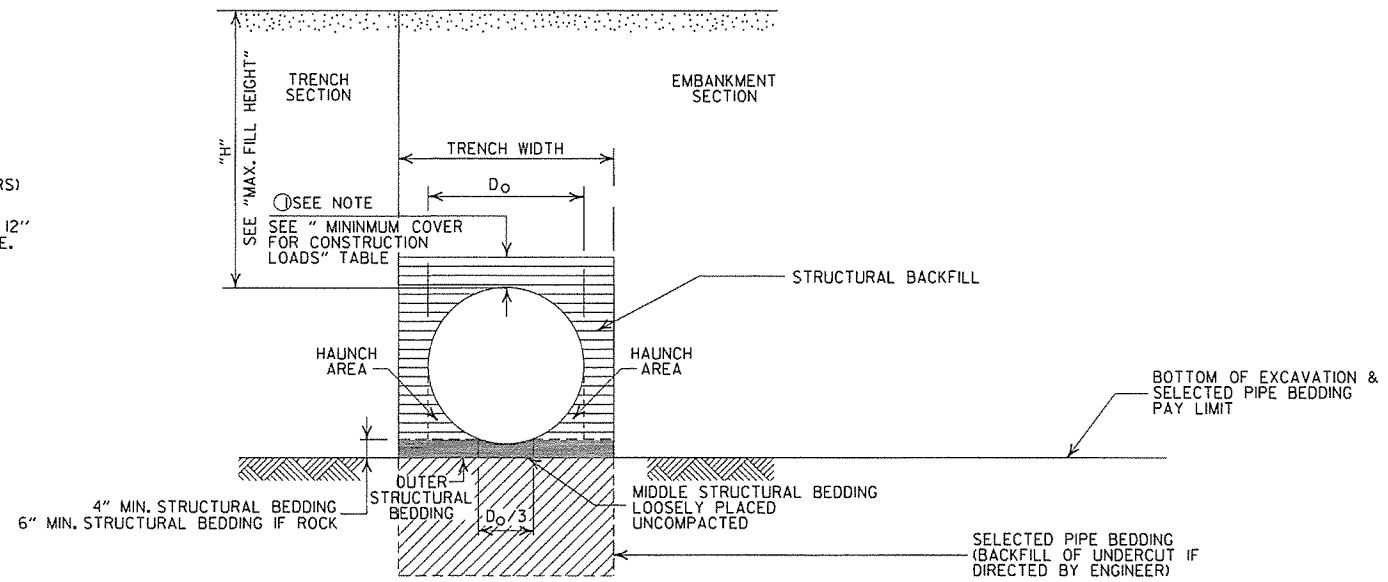
MULTIPLE INSTALLATION OF PVC PIPES

PIPE DIAMETER	CLEAR DISTANCE BETWEEN PIPES
18"	1'-6"
24"	2'-0"
30"	2'-6"
36"	3'-0"

MINIMUM COVER FOR CONSTRUCTION LOADS

PIPE DIAMETER	② MIN. COVER (FEET) FOR INDICATED CONSTRUCTION LOADS			
	18.0-50.0 (KIPS)	50.0-75.0 (KIPS)	75.0-110.0 (KIPS)	110.0-175.0 (KIPS)
18" THRU 36"	2'-0"	2'-6"	3'-0"	3'-0"

② MINIMUM COVER SHALL BE MEASURED FROM TOP OF PIPE TO TOP OF THE MAINTAINED CONSTRUCTION ROADWAY SURFACE. THE SURFACE SHALL BE MAINTAINED.



TYPE 2 EMBANKMENT AND TRENCH INSTALLATIONS

1. STRUCTURAL BACKFILL, EMBANKMENT, AND OUTER STRUCTURAL BEDDING MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DENSITY ACCORDING TO THE TYPE OR CLASS OF MATERIAL USED.

CONSTRUCTION SEQUENCE

1. PLACE STRUCTURAL BEDDING MATERIAL TO GRADE. DO NOT COMPACT.
2. INSTALL PIPE TO GRADE.
3. COMPACT STRUCTURAL BEDDING OUTSIDE THE MIDDLE THIRD OF THE PIPE.
4. THE STRUCTURAL BACKFILL SHALL BE PLACED AND COMPACTED IN LAYERS NOT EXCEEDING 8". THE LAYERS SHALL BE BROUGHT UP EVENLY AND SIMULTANEOUSLY TO THE ELEVATION OF THE MINIMUM COVER.
5. PIPE INSTALLATION MAY REQUIRE THE USE OF RESTRAINTS, WEIGHTING OR OTHER APPROVED METHODS IN ORDER TO HELP MAINTAIN GRADE AND ALIGNMENT.

- LEGEND -

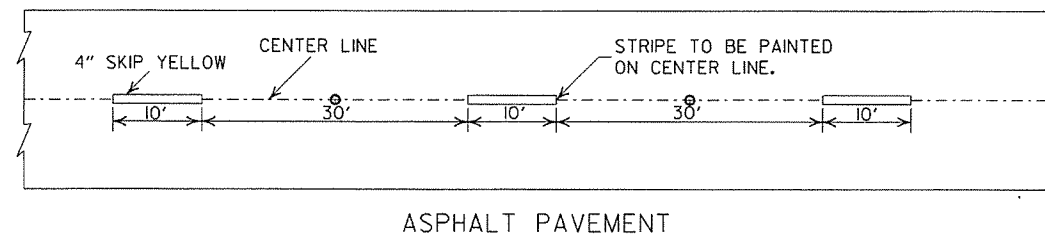
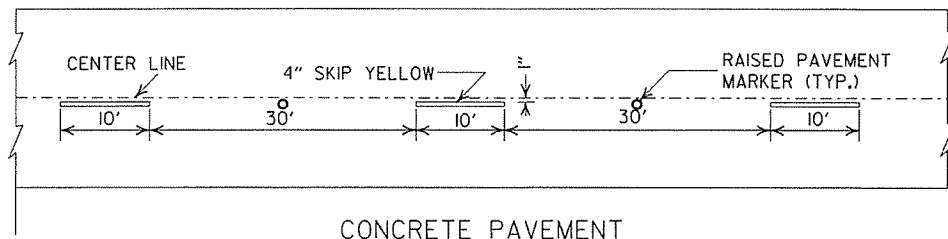
H = FILL HEIGHT (FT.)
 D_o = OUTSIDE DIAMETER OF PIPE
 MAX. = MAXIMUM
 MIN. = MINIMUM

==== = STRUCTURAL BACKFILL MATERIAL
 ===== = UNDISTURBED SOIL

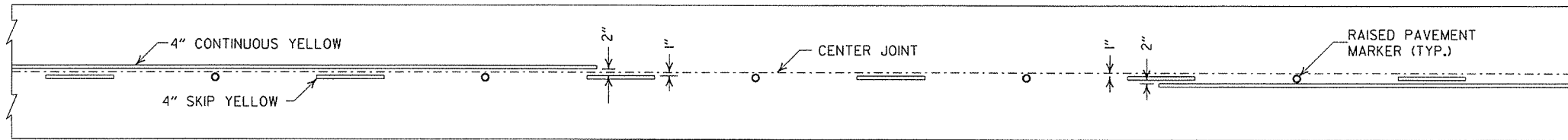
GENERAL NOTES

1. PIPE SHALL CONFORM TO ASTM F949, CELL CLASS 12454. INSTALLATION SHALL CONFORM TO JOB SPECIAL PROVISION "PLASTIC PIPE" AND SECTION 606 OF THE STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (CURRENT EDITION).
2. PLASTIC PIPE CULVERT DESIGN SHALL CONFORM TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, FIFTH EDITION (2010) WITH 2010 INTERIMS.
3. THE MAXIMUM ALLOWABLE TRENCH WIDTH SHALL BE THE MINIMUM WIDTH PLUS A SUFFICIENT WIDTH TO ENSURE WORKING ROOM TO PROPERLY AND SAFELY PLACE AND COMPACT HAUNCHING AND OTHER BACKFILL MATERIAL.
4. IMPERVIOUS MATERIAL SHOULD BE PLACED AS DIRECTED BY THE ENGINEER AT THE ENDS OF THE CULVERT TO PREVENT LOSS OF STRUCTURAL BEDDING WHEN PERVIOUS MATERIAL IS USED FOR STRUCTURAL BEDDING AND/OR BACKFILL.
5. WHEN DIRECTED BY THE ENGINEER, UNSUITABLE MATERIAL THAT IS ENCOUNTERED AT THE BOTTOM OF THE EXCAVATED TRENCH (BELOW THE AREA IDENTIFIED AS "STRUCTURAL BEDDING" ABOVE) WILL BE EXCAVATED AND REPLACED WITH SELECTED PIPE BEDDING. THE QUANTITY OF MATERIAL REQUIRED TO BACKFILL THE UNDERCUT AREA UP TO THE SELECTED PIPE BEDDING PAY LIMIT DESIGNATED ABOVE WILL BE MEASURED AND PAID FOR AS "SELECTED PIPE BEDDING."
6. WHEN THE EXISTING MATERIAL EXCAVATED FOR THE PIPE TRENCH IS DETERMINED BY THE ENGINEER TO BE UNSUITABLE FOR BACKFILLING THE PIPE (ABOVE THE AREA IDENTIFIED ABOVE AS STRUCTURAL BACKFILL), BORROW MATERIAL OR MATERIAL FROM THE ROADWAY EXCAVATION WILL BE USED TO BACKFILL THE PIPE, IF SUITABLE MATERIAL IS NOT AVAILABLE, THE ENGINEER MAY AUTHORIZE THE USE OF "SELECTED PIPE BACKFILL."
7. FOR PIPE TYPES THAT ARE NOT SMOOTH ON THE OUTSIDE (CORRUGATED OR PROFILE WALLS), BACKFILL GRADATIONS SHOULD BE SELECTED THAT WILL PERMIT THE FILLING OF THE CORRUGATION OR PROFILE VALLEY.
8. PVC PIPES OF DIAMETERS OTHER THAN SHOWN WILL NOT BE ALLOWED.
9. JOINTS FOR PVC PIPE SHALL MEET THE REQUIREMENTS FOR SOIL TIGHTNESS AS SPECIFIED IN AASHTO SECTION 26.4.2.4 AND 30.4.2 "AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS." JOINTS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS.

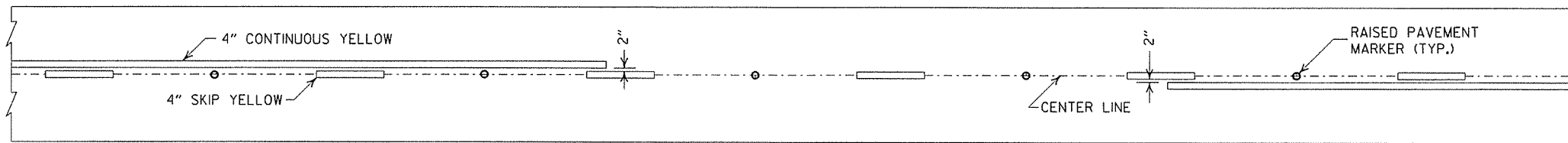
		ARKANSAS STATE HIGHWAY COMMISSION	
		PLASTIC PIPE CULVERT (PVC F949)	
		STANDARD DRAWING PCP-2	
2-27-14	REVISED GENERAL NOTE 1.		
12-15-11	REV GENERAL NOTES & MINIMUM COVER NOTE; DELETED SM3 MATERIAL		
11-17-10	ISSUED		
DATE	REVISION		DATE FILMED



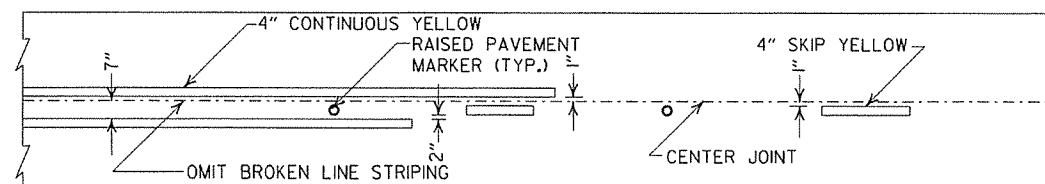
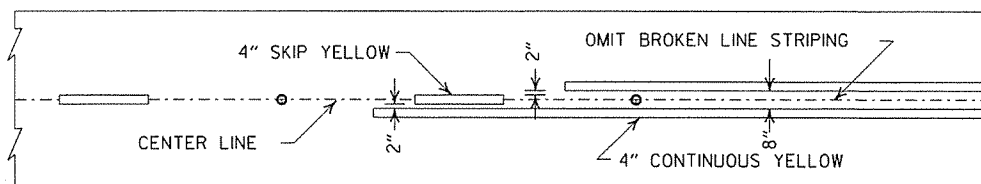
BROKEN LINE STRIPING



SOLID LINE STRIPING ON CONCRETE PAVEMENT



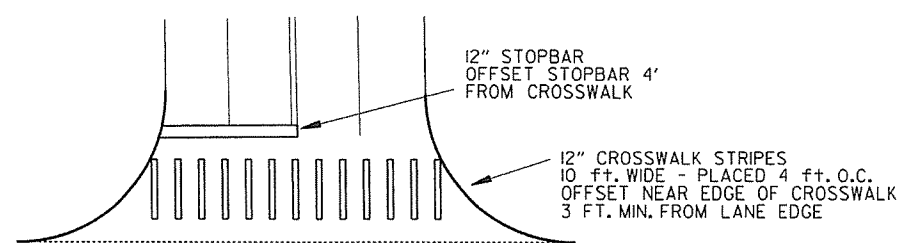
SOLID LINE STRIPING ON ASPHALT PAVEMENT



ASPHALT PAVEMENT

CONCRETE PAVEMENT

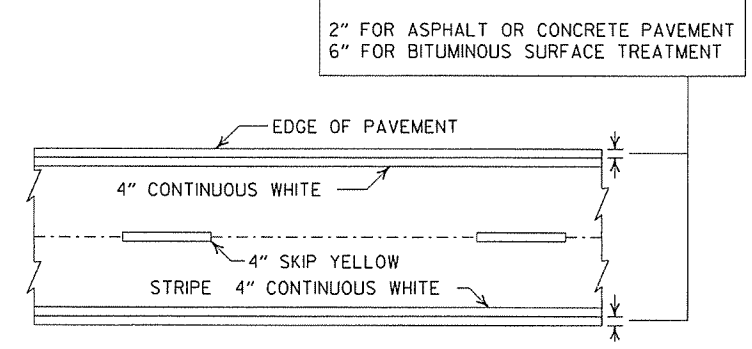
STRIPING AT ADJACENT NO PASSING LANES



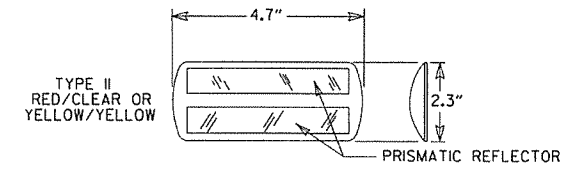
CROSSWALK AND STOPBAR DETAILS

NOTES:

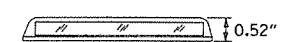
1. ALL LINES SHALL HAVE A WIDTH OF 4 INCHES.
2. THE THICKNESS AND RATE OF PAINT APPLICATION SHALL BE AS SPECIFIED IN SECTION 718 OF THE STANDARD SPECIFICATIONS.
3. THIS DRAWING SHALL BE USED IN CONJUNCTION WITH THE LATEST REVISED ADDITION OF THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES."
4. RAISED PAVEMENT MARKERS SHALL BE CENTERED BETWEEN SKIP LINES ON 40 FEET SPACING UNLESS OTHERWISE SHOWN ON THE PLANS.



PAVEMENT EDGE LINE MARKING



NOTE:
THE RED LENS OF THE TYPE II R.P.M. SHALL FACE THE INCORRECT TRAFFIC MOVEMENT.



DETAIL OF STANDARD RAISED PAVEMENT MARKERS

GENERAL NOTES:
THIS DRAWING SHOULD BE CONSIDERED AS TYPICAL ONLY AND THE FINAL LOCATION OF THE STRIPING AND RAISED PAVEMENT MARKERS SHALL BE DETERMINED BY THE ENGINEER.

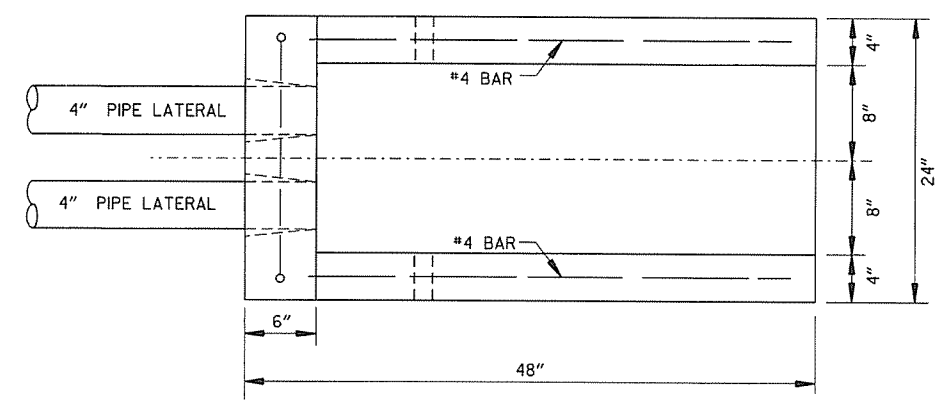
THIS DRAWING SHOULD BE USED IN CONJUNCTION WITH THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES", LATEST REVISION.

NOTE:
DIMENSIONS SHOWN FOR RAISED PAVEMENT MARKERS ARE TYPICAL. THE CONTRACTOR MAY SUBSTITUTE SIMILAR MARKERS WITH THE APPROVAL OF THE ENGINEER. REQUESTING APPROVAL FOR SIMILAR MARKERS MAY BE MADE BY REFERRING TO THE AHTD QUALIFIED PRODUCTS LIST.

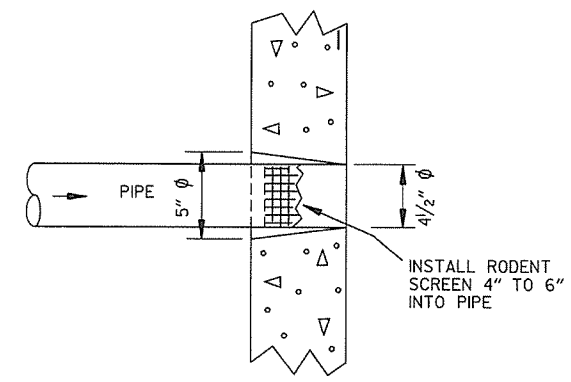
DATE	REVISION	FILMED
9-12-13	REVISED DETAIL OF STANDARD RAISED PAVEMENT MARKERS	
11-17-10	REVISED GENERAL NOTES & REMOVED PLOWABLE PVMT MRKRS	
11-18-04	REVISED NOTE 2 & GENERAL NOTES	
8-22-02	ADDED CROSSWALK & STOPBAR DTLS.	
7-02-98	ADDED DETAILS OF STD. RAISED PAV'T. MARKERS	
4-26-96	REV. NOTES 3&4; ADDED R.P.M.	
9-30-80	DRAWN	1-9-30-80

ARKANSAS STATE HIGHWAY COMMISSION	
PAVEMENT MARKING DETAILS	
STANDARD DRAWING PM-1	

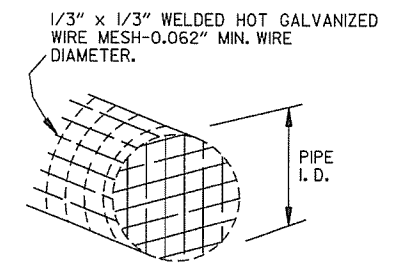
NOTE:
 1. GRANULAR BACKFILL TO BE SUBSIDIARY TO PIPE UNDERDRAIN.
 2. UNLESS OTHERWISE SPECIFIED ON THE PLANS, THE UNDERDRAIN COVER SHALL BE THOROUGHLY COMPACTED EARTH AND SHALL BE SUBSIDIARY TO PIPE UNDERDRAIN.
 3. GRANULAR MATERIAL SHALL BE WRAPPED WITH GEOTEXTILE FABRIC, LAP FABRIC 12" OR THE WIDTH OF THE TRENCH AT THE TOP.



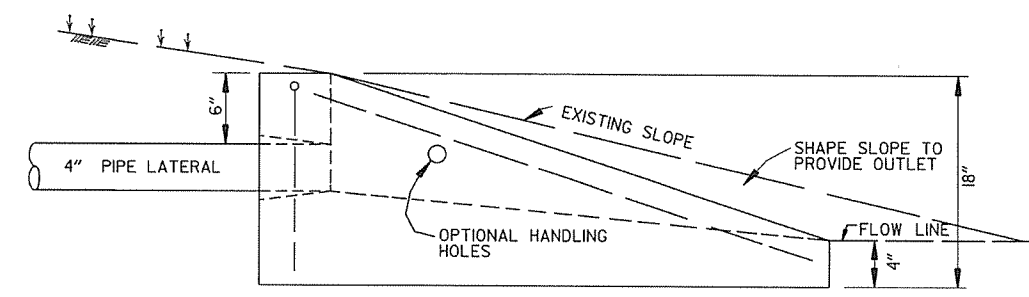
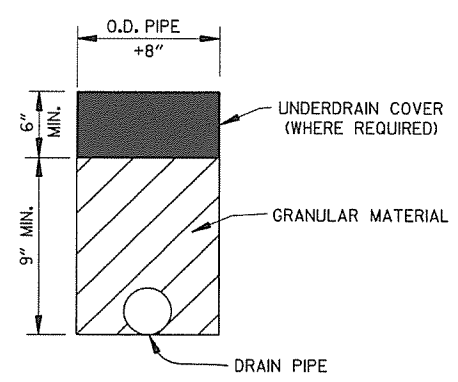
PLAN VIEW



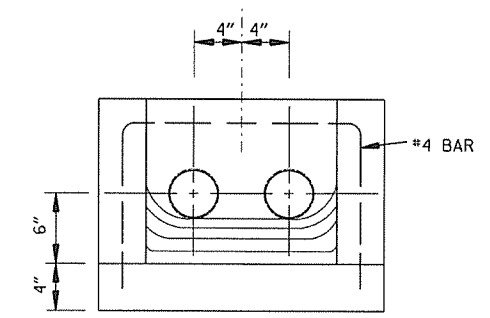
DETAIL OF HOLE FOR 4" PIPE



DETAIL OF RODENT SCREEN



SIDE VIEW

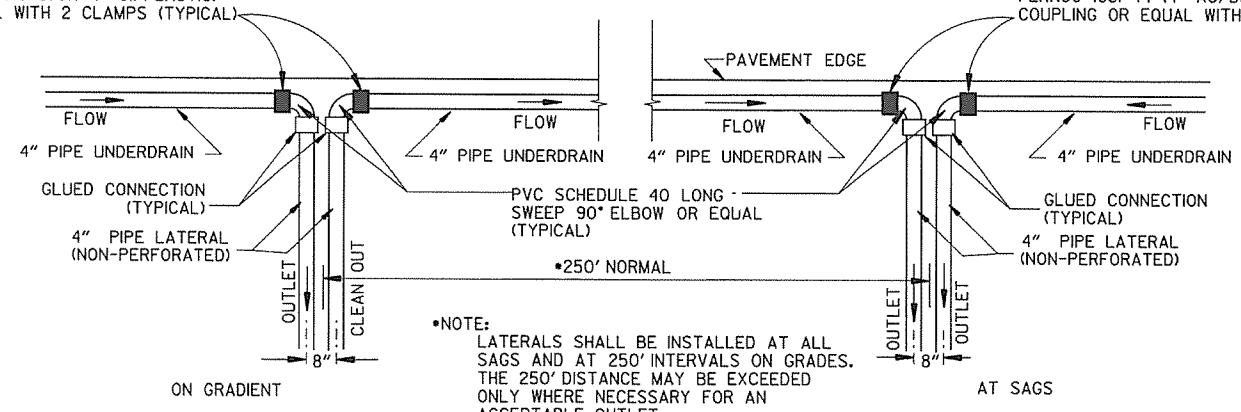


FRONT VIEW

FERNCO I056-44 (4" CI/PLASTIC) OR FERNCO I051-44 (4" AC/DI OR 4" CI/PLASTIC) COUPLING OR EQUAL WITH 2 CLAMPS (TYPICAL)

UNDERDRAIN OUTLET PROTECTORS

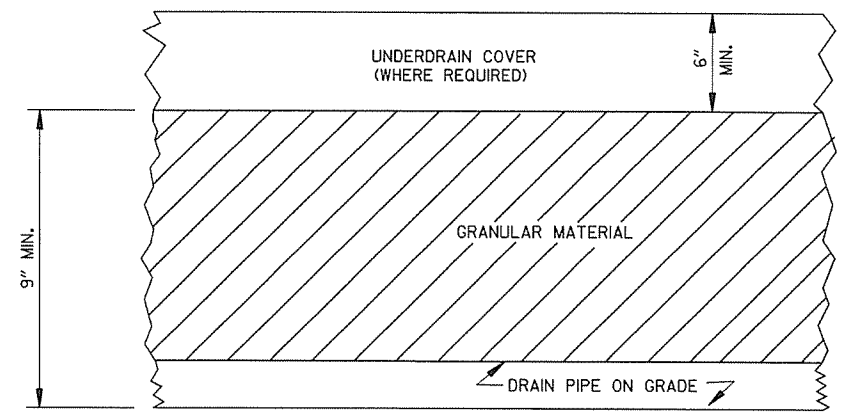
FERNCO I056-44 (4" CI/PLASTIC) OR FERNCO I051-44 (4" AC/DI OR 4" CI/PLASTIC) COUPLING OR EQUAL WITH 2 CLAMPS (TYPICAL)



NOTE:
 LATERALS SHALL BE INSTALLED AT ALL SAGS AND AT 250' INTERVALS ON GRADES. THE 250' DISTANCE MAY BE EXCEEDED ONLY WHERE NECESSARY FOR AN ACCEPTABLE OUTLET.

DETAIL OF PIPE UNDERDRAIN LATERALS WHEN PLACED ALONG PAVEMENT EDGE

NOTE: PVC PIPE FOR LATERALS SHALL MEET THE REQUIREMENTS OF ASTM D 1785 (LATEST REVISION) FOR SCHEDULE 40 PIPE.



DETAILS OF PIPE UNDERDRAIN

4-10-03	REVISED NOTE 3	
1-12-00	REVISED DETAIL OF UNDERDRAIN LATERALS	
11-18-98	REVISED NOTE	
10-18-96	REVISED MIN. DEPTH & GEOTEXTILE FABRIC	
4-26-96	ADDED LATERAL NOTE; 5 1/2" TO 5"	
11-22-95	REVISED LATERALS	
7-20-95	REVISED LATERALS & ADDED NOTE	
11-3-94	REVISED FOR DUAL LATERALS	11-3-94
10-1-92	SUBSTITUTED GEOTEXTILE	10-1-92
8-15-91	ADDED POLYETHYLENE PIPE	8-15-91
11-8-90	DELETED ALTERNATE NOTE	11-8-90
1-25-90	ADDED 4" SNAP ADAPTER	1-25-90
11-30-89	DEL. (SUBGRADE); ADDED (WHERE REQUIRED)	11-30-89
7-15-88	ISSUED P.L.M.	647-7-15-88
DATE	REVISION	DATE FILMED

ARKANSAS STATE HIGHWAY COMMISSION

DETAILS OF PIPE UNDERDRAIN

STANDARD DRAWING PU-1

SUPERELEVATION TABLE FOR TWO - WAY TRAFFIC

DEGREE OF CURVE	30 MPH		40 MPH		50 MPH		55 MPH		60 MPH		70 MPH	
	e	Ls (FT)	e	Ls (FT)	e	Ls (FT)	e	Ls (FT)	e	Ls (FT)	e	Ls (FT)
0° 15'	N.C.		N.C.		N.C.		N.C.		N.C.		N.C.	
0° 30'	N.C.		N.C.		N.C.		N.C.		N.C.		N.C.	
0° 45'	N.C.		N.C.		N.C.		N.C.		N.C.		N.C.	
1° 00'	N.C.		N.C.		0.021		0.022		0.023		0.028	
1° 15'	N.C.		N.C.		0.026		0.032		0.037		0.046	
1° 30'	N.C.		R.C.		0.031		0.037		0.043		0.054	
1° 45'	N.C.		0.021		0.036		0.043		0.049		0.062	
2° 00'	R.C.		0.025	175	0.040		0.048		0.055		0.070	
2° 15'	R.C.		0.031		0.045		0.053		0.061		0.078	
2° 30'	0.021		0.034		0.049		0.058		0.067		0.085	
2° 45'	0.023		0.037		0.053		0.063		0.072		0.091	
3° 00'	0.025	150	0.040		0.057		0.067	230	0.077	260	0.096	350
3° 15'	0.027		0.043		0.061		0.072	245	0.082	275	0.098	360
3° 30'	0.029		0.046		0.065	205	0.076	255	0.086	285	0.100	380
3° 45'	0.031		0.049		0.069	215	0.080	265	0.090	295		
4° 00'	0.033		0.051		0.072	225	0.083	270	0.093	305		
4° 30'	0.037		0.056		0.078	240	0.087	280	0.096	315		
5° 00'	0.040		0.061		0.083	250	0.091	295	0.098	320		
5° 30'	0.043		0.066	185	0.088	260	0.094	300				
6° 00'	0.046		0.070	190	0.092	270	0.096	305				
6° 30'	0.050		0.074	200	0.095	280	0.100	315				
7° 00'	0.053		0.078	210	0.098	285						
7° 30'	0.056		0.081	215	0.099	290						
8° 00'	0.058		0.084	220	0.100	290						
8° 30'	0.061		0.087	225								
9° 00'	0.063		0.089	230								
10° 00'	0.068	160	0.094	235								
11° 00'	0.072	170	0.097	250								
12° 00'	0.076	175	0.099	250								
13° 00'	0.080	180	0.100	250								
14° 00'	0.083	190										
15° 00'	0.086	195										
16° 00'	0.089	200										
17° 00'	0.091	200										
18° 00'	0.093	205										
19° 00'	0.095	210										
20° 00'	0.097	215										
21° 00'	0.098	215										
22° 00'	0.099	215										
23° 00'	0.099	215										
24° 00'	0.100	220										

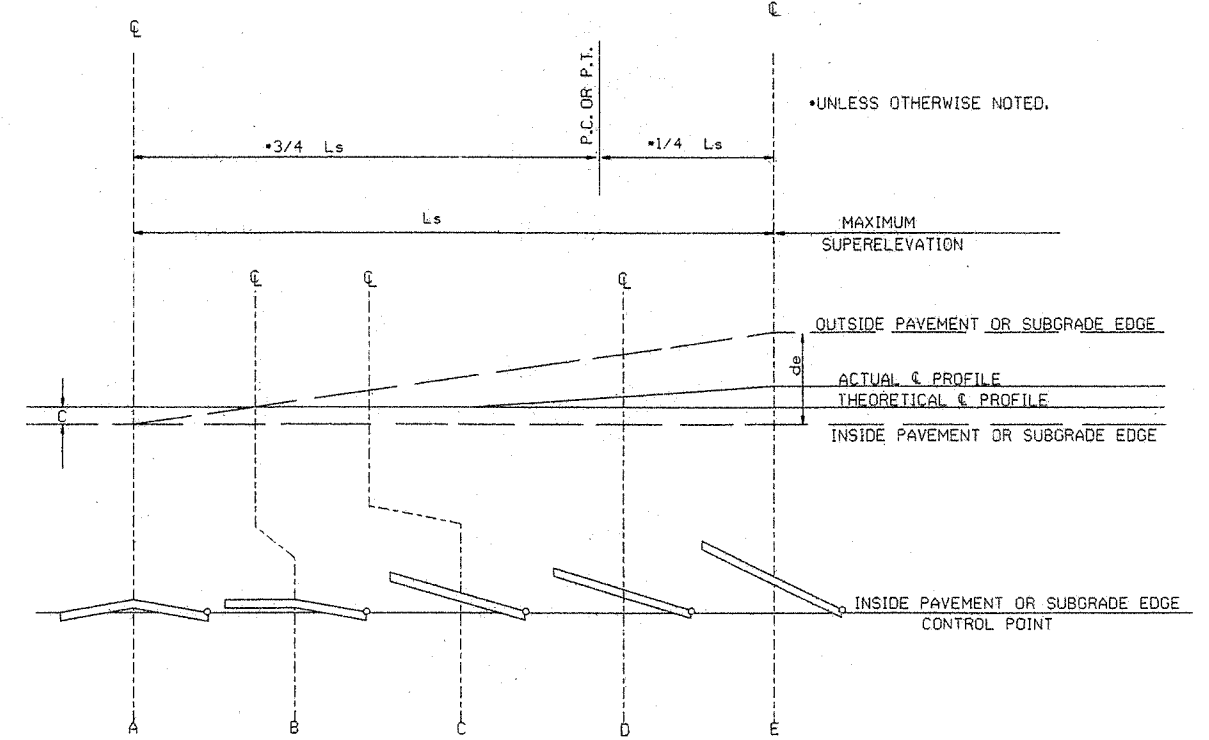
ABBREVIATIONS

- NC - NORMAL CROWN
- RC - REVERSE CROWN, SUPERELEVATION AT NORMAL CROWN SLOPE
- e - RATE OF SUPERELEVATION (FT. PER FT.)
- Ls - LENGTH OF SUPERELEVATION TRANSITION (FT.)
- L - DISTANCE FROM BEGINNING OF SUPERELEVATION TRANSITION TO ANY POINT (FT.)
- d - WIDTH OF PAVEMENT (FT.) OR WIDTH OF SUBGRADE (FT.)
- C - NORMAL CROWN (FT.)

GENERAL NOTES

- ON PAVEMENT WITH TWO-WAY TRAFFIC, THE SUPERELEVATION SHALL BE REVOLVED ON THE INSIDE PAVEMENT EDGE UNLESS OTHERWISE NOTED ON THE PLANS
- SUPERELEVATION VALUES SHOWN ON THE CROSS SECTIONS ARE VALUES (+) OR (-) TO BE ADDED TO OR SUBTRACTED FROM THE POINT OF CONTROL.
- LENGTHS FOR L MAY BE ROUNDED IN MULTIPLES OF 25 FT. OR 50 FT. TO PERMIT SIMPLER CALCULATIONS.
- PAVEMENTS WIDER THAN 2 LANES SHALL HAVE ADDITIONAL TRANSITION LENGTHS AS FOLLOWS:
 - 3 LANE UNDIVIDED ----- +20%
 - 4 LANE UNDIVIDED ----- +50%
 - 5 LANE UNDIVIDED ----- +80%
 - 6 LANE UNDIVIDED ----- +100%

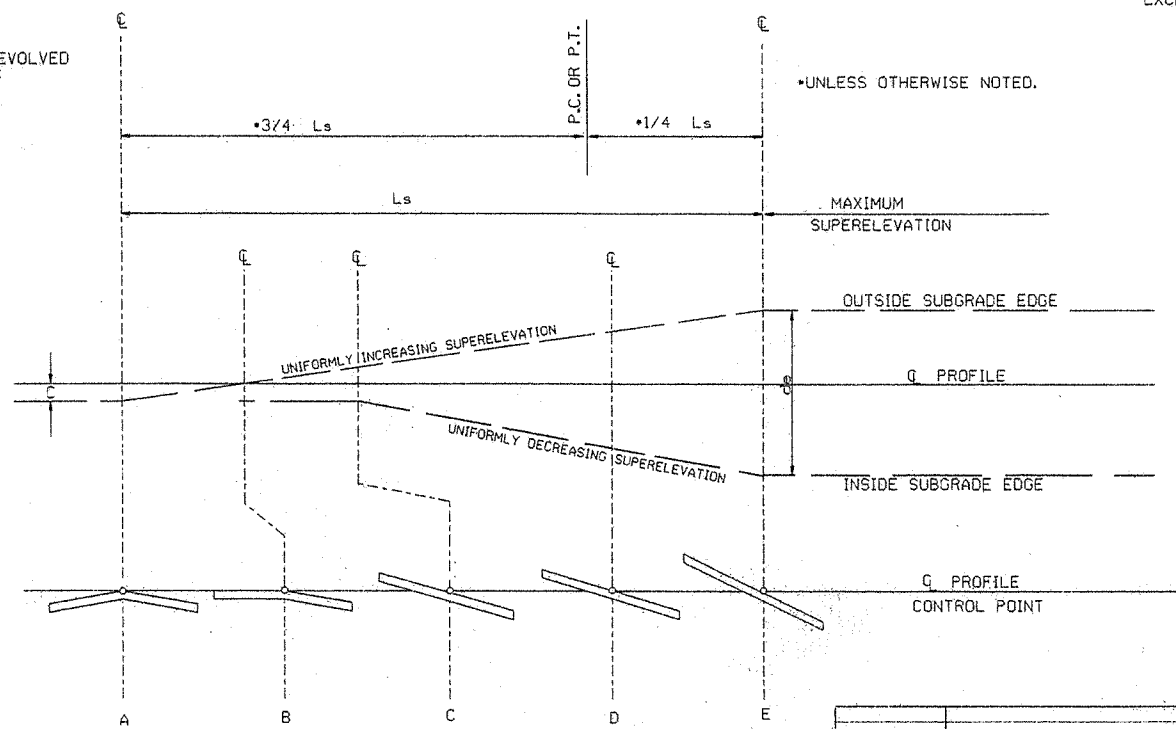
NOTE: MAINTAIN NORMAL CROWN ON INSIDE UNTIL SUPERELEVATION EXCEEDS 2C.
RATE OF SUPERELEVATION SHALL BE COMPUTED ON STRAIGHT LINE METHOD USING APPLICABLE Ls.



STANDARD METHOD WHEN SUPERELEVATION REVOLVES AROUND INNER SUBGRADE POINT OR INNER PAVEMENT EDGE

NOTE: MAINTAIN NORMAL CROWN ON INSIDE UNTIL SUPERELEVATION EXCEEDS 2C.

SUPERELEVATION FORMULA = $\frac{Lde}{Ls}$



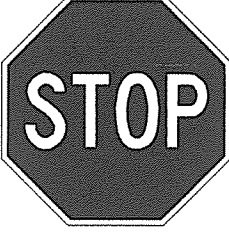
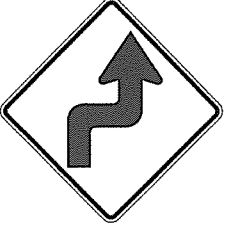
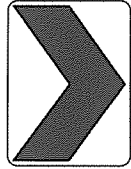

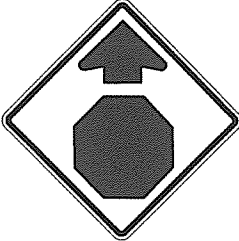

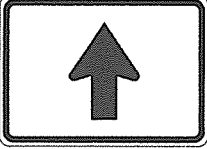
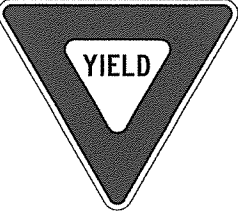

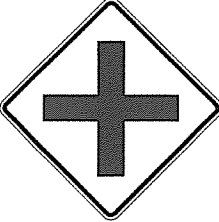

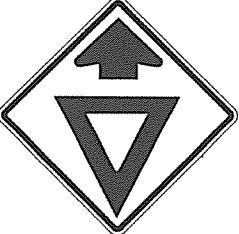

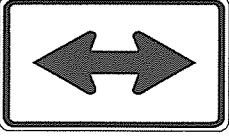
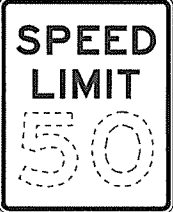
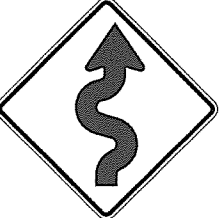
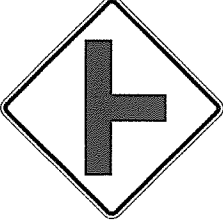




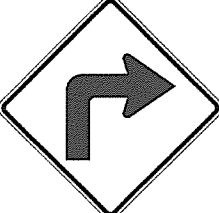
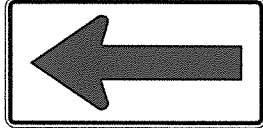
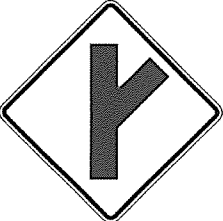

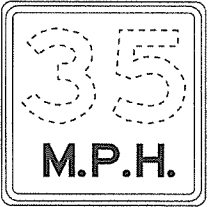
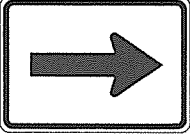
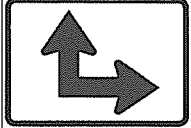
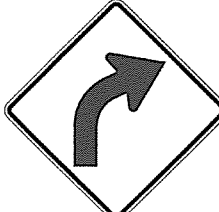
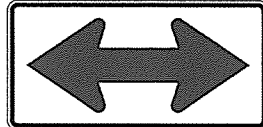
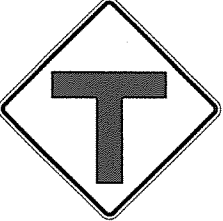
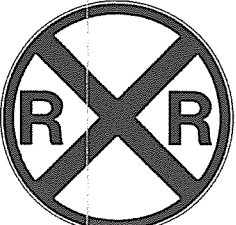
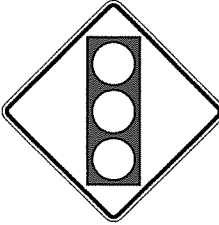

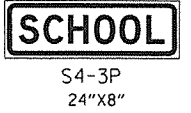

STANDARD METHOD WHEN SUPERELEVATION REVOLVES AROUND CENTER LINE

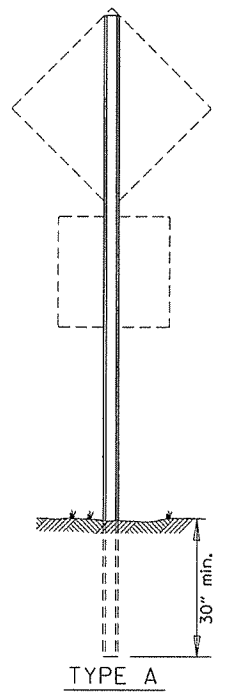
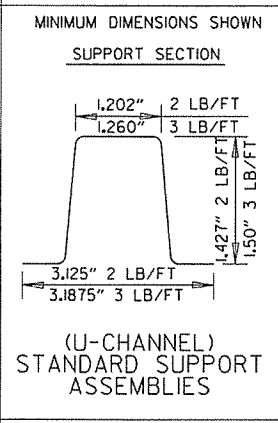
10-18-96	ADDED FORMULA	16-18-96
01-09-87	ISSUED	534-1-9-87
DATE	REVISION	DATE FILLED

ARKANSAS STATE HIGHWAY COMMISSION

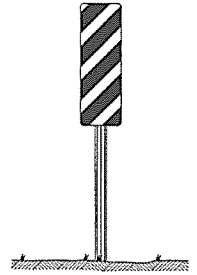
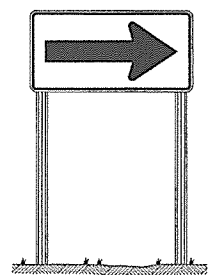
TABLES AND METHOD OF SUPERELEVATION FOR TWO-WAY TRAFFIC

STANDARD DRAWING SE-2

 RI-1 30"x30"	 WI-3 30"x30" (LT. OR RT.)	 WI-8 18"x24"	 W2-5 30"x30"	 W3-1 36"x36"	 W5-1 36"x36"	 M6-3 21"x15"
 RI-2 36"x36"x36"	 WI-4 30"x30" (LT. OR RT.)	 W2-1 30"x30"	 SI-1 36"x36"	 W3-2 36"x36"	 MI-6 24"x24" NOTE: REFLECTORIZED YELLOW LEGEND (COUNTY NAME, ROUTE LETTER & NUMBER) & BORDER ON A BLUE BACKGROUND.	 M6-4 21"x15"
 R2-1 24"x30"	 WI-5 30"x30" (LT. OR RT.)	 W2-2 30"x30"	 W5-2 36"x36"	 W8-3 36"x36"	 RI-3P 18"x6"	 M6-5 21"x15"
 WI-1 30"x30" (LT. OR RT.)	 WI-6 48"x24"	 W2-3 30"x30" (LT. OR RT.)	 W5-3 36"x36"	 W13-1P 18"x18"	 M6-1 21"x15"	 M6-6 21"x15"
 WI-2 30"x30" (LT. OR RT.)	 WI-7 48"x24"	 W2-4 30"x30"	 W10-1 36" DIAMETER	 W3-3 36"x36"	 M6-2 21"x15"	 S4-3P 24"x8"  S4-2P 24"x10"



NOTE: LENGTH OF SIGN POSTS SHALL BE DETERMINED SO AS TO PROVIDE FOR MINIMUM VERTICAL CLEARANCES AS CALLED FOR IN THE SPECIFICATIONS PLUS A MINIMUM VERTICAL PENETRATION OF 30" IN THE SOIL.

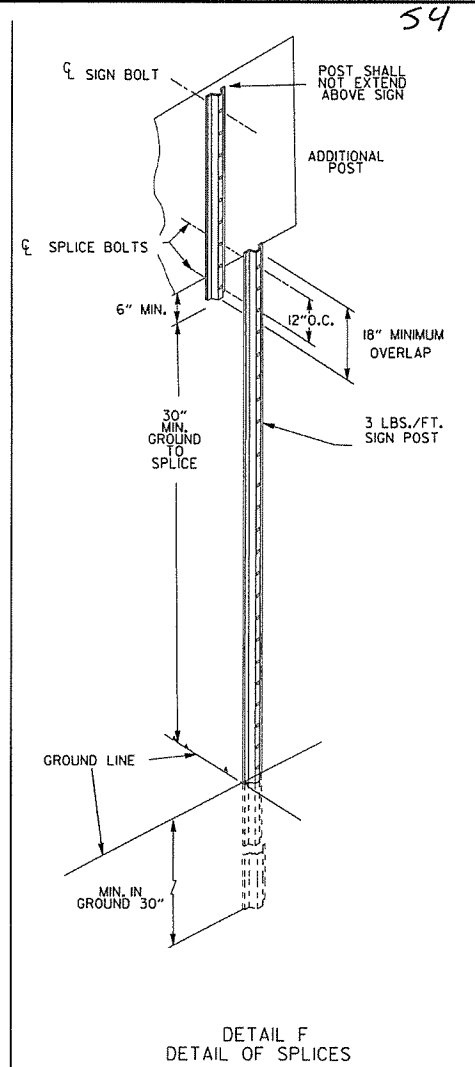
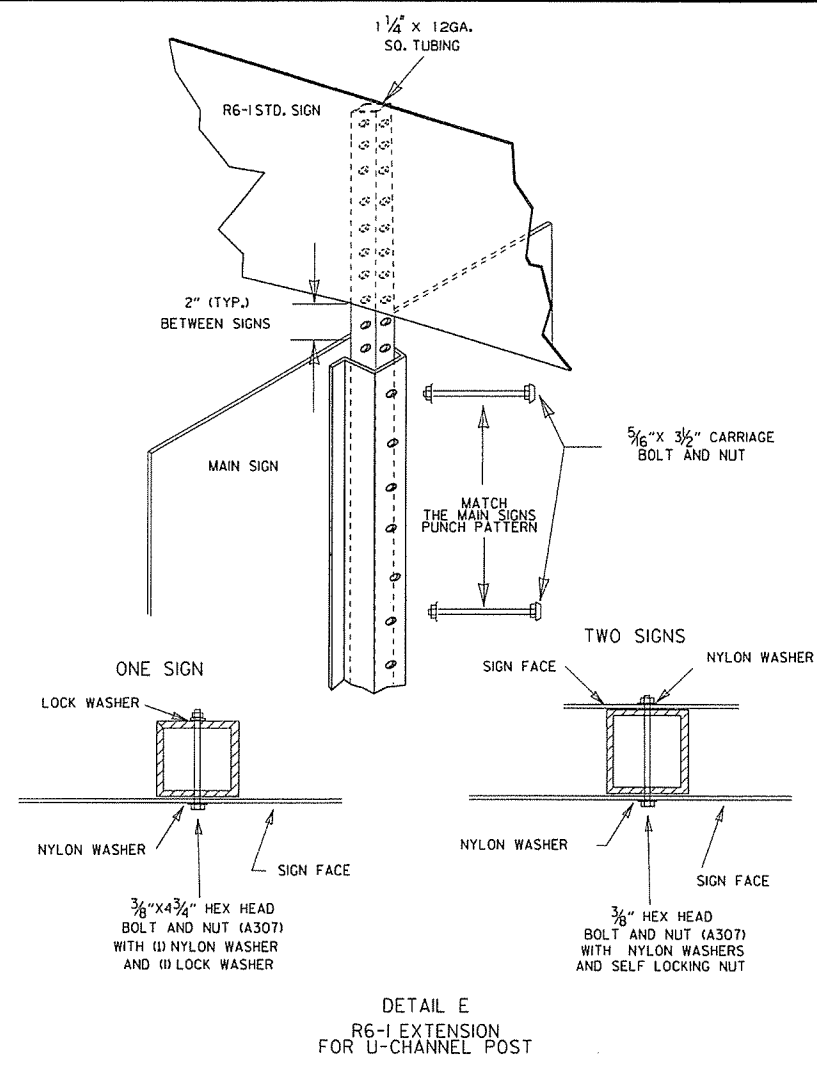
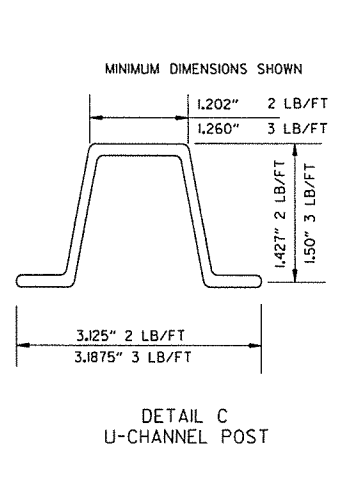
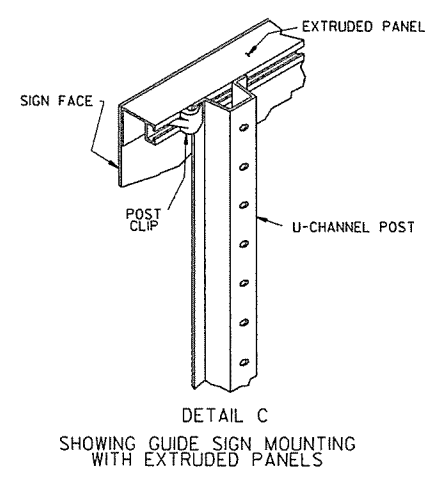
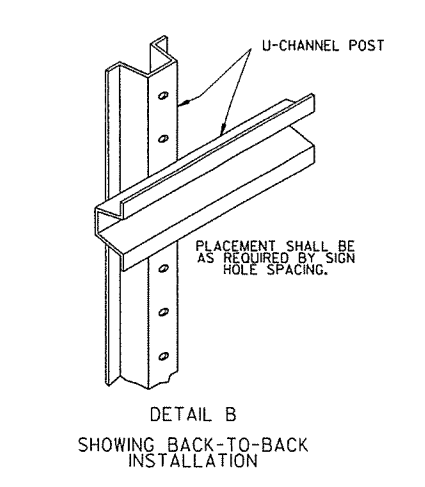
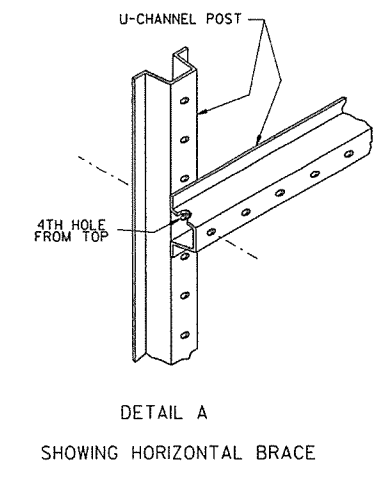
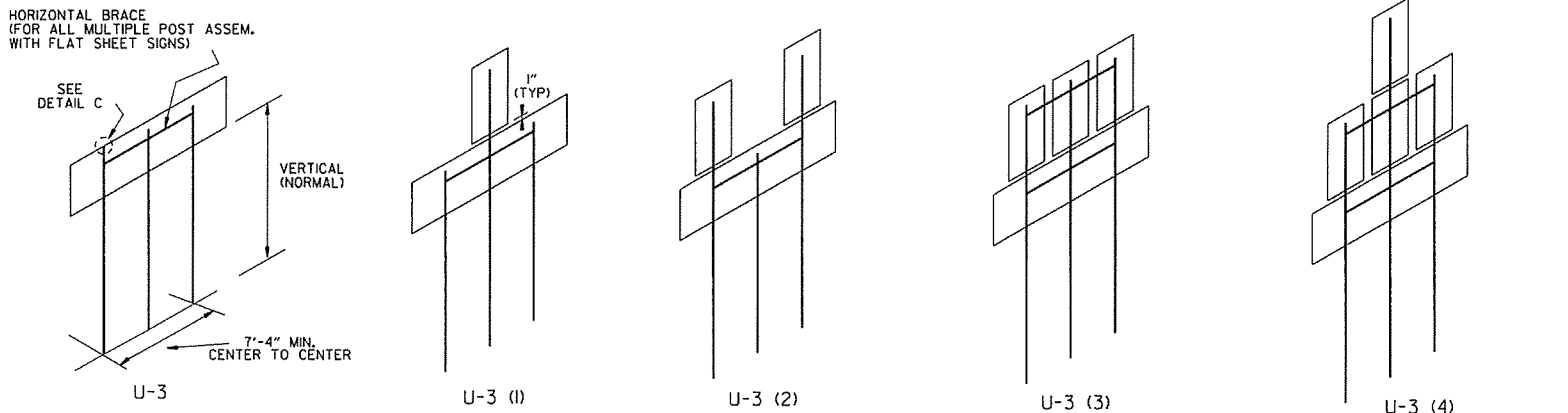
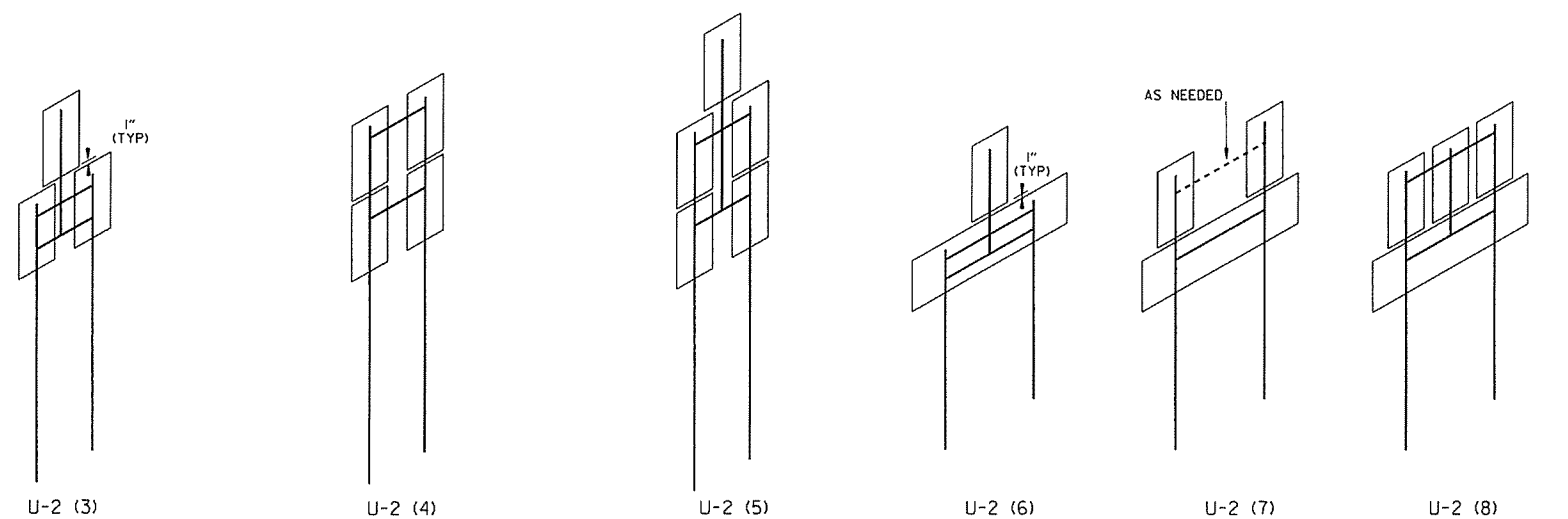
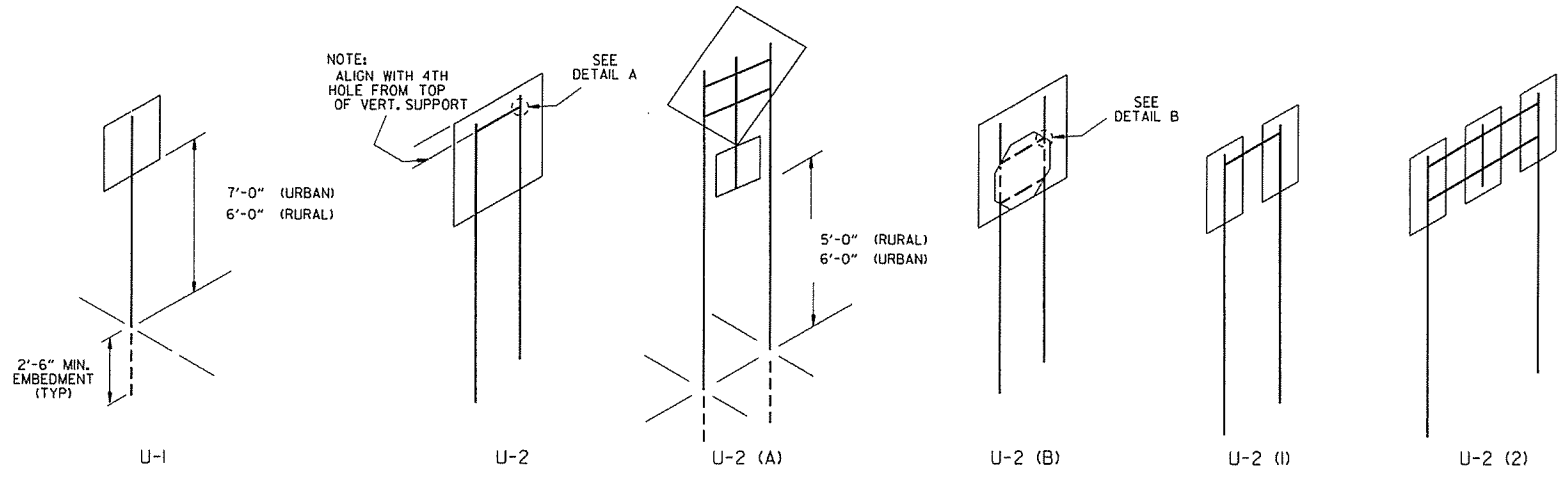


MINIMUM WEIGHT
TYPE A & B = 3 LBS./FT.
TYPE C = 2 LBS./FT.

STANDARD HIGHWAY SIGNS

SUPPORT ASSEMBLIES
ARKANSAS STATE HIGHWAY COMMISSION
STANDARD HIGHWAY SIGNS AND SUPPORT ASSEMBLIES
STANDARD DRAWING SHS-1

9-12-13	DELETED JOB NO. BLOCK; REVISED RI-3 TO RI-3P	
4-17-08	REVISED SIGN DESIGNATION - W3-1 & W3-2	
4-10-03	REVISED W5-2, W9-3, OM-3; ADDED WI-8	
1-5-81	REDRAWN	960-1-15-81
9-15-78	ADDED WI-4-3	877-9-15-78
9-2-76	POST WT.	623-9-3-76
5-3-76	STEEL POST WI FROM 2*-3*; ADDED S4-2 & S4-3	504-5-3-76
8-12-74	REV. HI. TYPE "C" ASSEMBLY	500-8-21-74
12-21-72	ADDED M6-2,3,4,5,6	500-12-21-72
12-1-72	ISSUED	562-12-1-72
DATE	REVISION	DATE FILMED



NOTES:

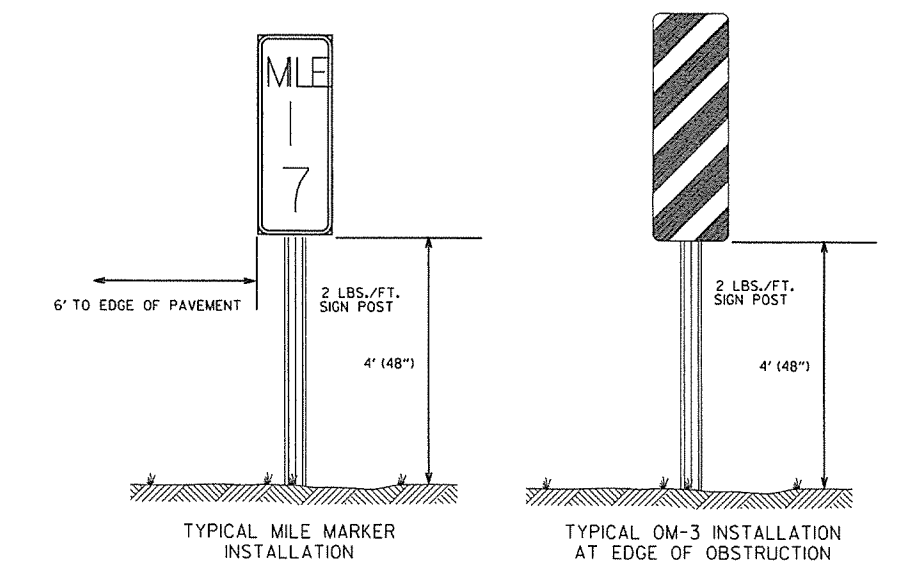
SIGNS AT LEAST 8' IN LENGTH MAY BE INSTALLED ON THREE 3 LB. POST. IN NO CASE SHALL THERE BE MORE THAN TWO 3 LB. POSTS WITHIN A 7' PATH.

SPLICES NECESSARY TO ATTAIN PROPER MOUNTING HEIGHT SHALL BE AS SHOWN IN DETAIL (F).


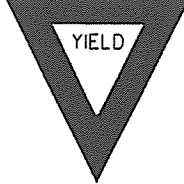
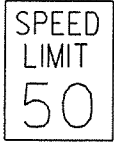





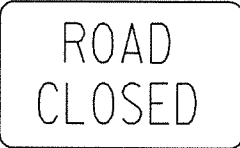
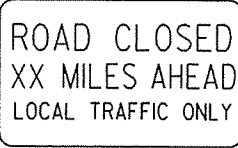
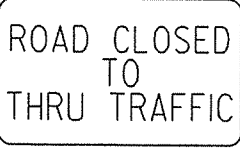
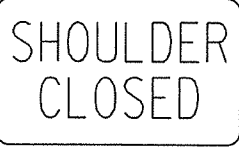
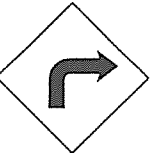

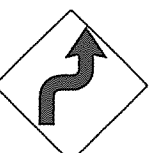

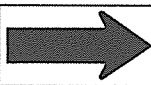
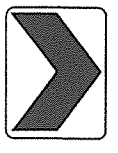
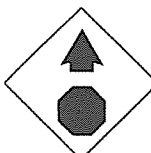
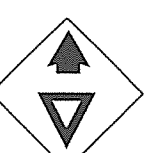
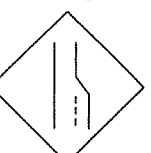

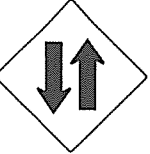




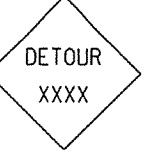



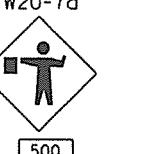

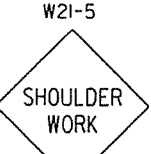
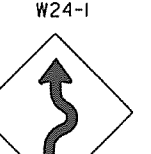

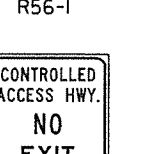
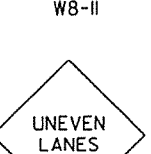
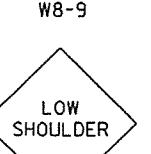
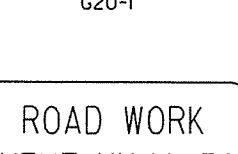
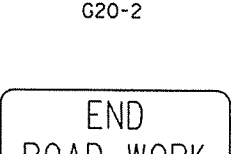
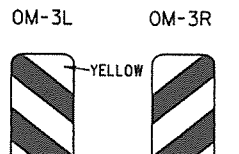
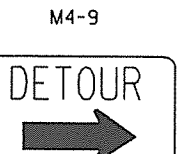
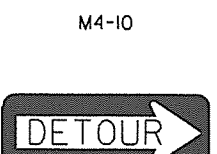
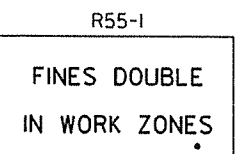
NORMAL INSTALLATIONS WILL REQUIRE 5/16" DIA. CARRIAGE BOLTS TO MOUNT SIGNS TO POST AND TO ASSEMBLE THE VARIOUS POST SUPPORTS.

ALL SIGN POSTS SHALL BE PLUMB.

THE POST FOR "TYPE U" SUPPORTS SHALL BE HOT DIP GALVANIZED.



ARKANSAS STATE HIGHWAY COMMISSION		
U-CHANNEL POST ASSEMBLIES		
STANDARD DRAWING SHS-2		
DATE	REVISION	FILED
9-12-13	REVISED U-2(3), U-2(6), U-3(1), DETAIL D; ADDED DETAILS E & F; ADDED TYPICAL MARKERS	
10-9-03	REMOVED ROUND POST & REVISED SPACING	
10-12-95	MOVED UPPER SPLICE	
6-8-95	REVISED SPLICE DETAIL	6-8-95
2-2-95	REDRAWN	2-2-95

<p>RI-1</p>  <p>STANDARD 30"x30" EXPRESSWAY 36"x36" SPECIAL 48"x48"</p>	<p>RI-2</p>  <p>STD. 36"x36"x36" EXPWY. 48"x48"x48" FWY. 60"x60"x60"</p>	<p>R2-1</p>  <p>STD. 24"x30" EXPWY. 36"x48" FWY. 48"x60"</p>	<p>W3-5</p>  <p>STD. 36"x36" EXPWY. 48"x48" FWY. 48"x48"</p>	<p>W3-5a</p>  <p>STD. 36"x36" EXPWY. 48"x48" FWY. 48"x48"</p>	<p>R4-1</p>  <p>STD. 24"x30" EXPWY. 36"x48" FWY. 48"x60"</p>	<p>R4-2</p>  <p>STD. 24"x30" EXPWY. 36"x48" FWY. 48"x60"</p>	
<p>R5-1</p>  <p>STD. 30"x30" EXPWY. 36"x36" SPECIAL 48"x48"</p>	<p>R11-2</p>  <p>48"x30"</p>	<p>R11-3A</p>  <p>60"x30"</p>	<p>R11-4</p>  <p>60"x30"</p>	<p>RSP-1</p>  <p>48"x30"</p>	<p>W1-1</p>  <p>STD. 36"x36" FWY. 48"x48"</p>	<p>W1-2</p>  <p>STD. 36"x36" FWY. 48"x48"</p>	
<p>W1-3</p>  <p>STD. 48"x48"</p>	<p>W1-4</p>  <p>STD. 48"x48"</p>	<p>W1-6</p>  <p>STD. 48"x24" SPECIAL 60"x30"</p>	<p>W1-8</p>  <p>STD. 18"x24" SPECIAL 24"x30" EXPWY. 30"x36" FWY. 36"x48"</p>	<p>W3-1</p>  <p>STD. 36"x36" SPECIAL 48"x48"</p>	<p>W3-2</p>  <p>STD. 36"x36" SPECIAL 48"x48"</p>	<p>W4-2</p>  <p>STD. 36"x36" FWY. 48"x48"</p>	
<p>W5-1</p>  <p>STD. 36"x36" SPECIAL 48"x48"</p>	<p>W6-3</p>  <p>EXPWY. 36"x36" SPECIAL 48"x48"</p>	<p>W8-7</p>  <p>EXPWY. 36"x36" FWY. 48"x48"</p>	<p>W9-2</p>  <p>STD. 36"x36" FWY. 48"x48"</p>	<p>W13-1</p>  <p>STD. 24"x24"</p>	<p>W20-1</p>  <p>STD. 48"x48"</p>	<p>W20-2</p>  <p>STD. 48"x48"</p>	<p>W20-3</p>  <p>STD. 48"x48"</p>
<p>W20-4</p>  <p>STD. 48"x48"</p>	<p>W20-5</p>  <p>STD. 48"x48"</p>	<p>W20-7a</p>  <p>STD. 36"x36" FWY. 48"x48"</p>	<p>W21-2</p>  <p>STD. 30"x30" SPECIAL 36"x36"</p>	<p>W21-5</p>  <p>STD. 30"x30" SPECIAL 36"x36"</p>	<p>W24-1</p>  <p>STD. 36"x36"</p>	<p>W1-4b</p>  <p>STD. 48"x48"</p>	<p>R56-1</p>  <p>STD. 18"x18"</p>
<p>W8-11</p>  <p>STD. 36"x36" FWY. 48"x48"</p>	<p>W8-9</p>  <p>STD. 36"x36" FWY. 48"x48"</p>	<p>G20-1</p>  <p>60"x24"</p>	<p>G20-2</p>  <p>48"x24"</p>	<p>OM-3L OM-3R</p>  <p>12"x36"</p>	<p>M4-9</p>  <p>STD. 30"x24" SPECIAL 48"x36" SPECIAL 60"x48"</p>	<p>M4-10</p>  <p>48"x18"</p>	<p>R55-1</p>  <p>36"x60"</p> <p>WHEN WORKERS ARE PRESENT **</p> <p>• USE 6" C LETTERS •• USE 4" D LETTERS</p>

55

ADVANCE DISTANCES (XXXX)

500 FT	1/2 MILE
1000 FT	3/4 MILE
1500 FT	1 MILE AHEAD

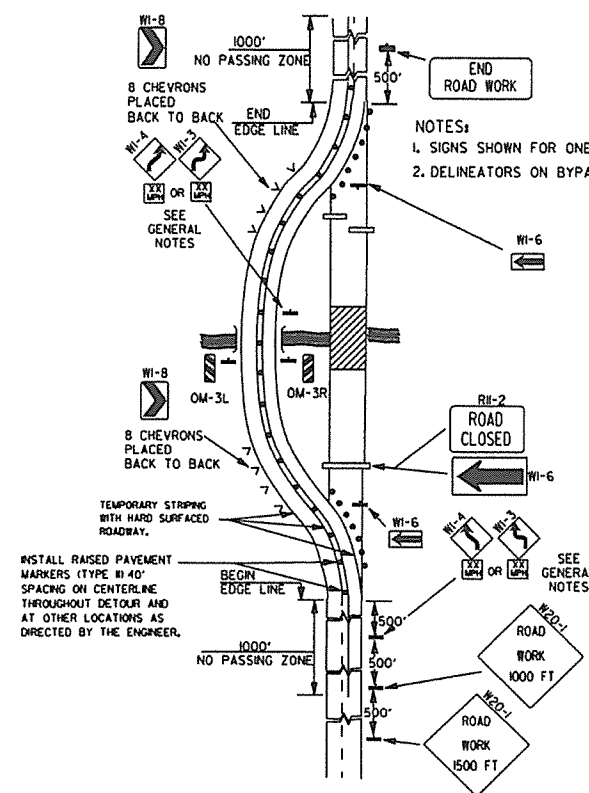
GENERAL NOTES:

- ALL TRAFFIC CONTROL DEVICES USED ON ROAD CONSTRUCTION SHALL CONFORM TO THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES, LATEST EDITION, AND TO THE STANDARD HIGHWAY SIGNS, LATEST EDITION, OR AS APPROVED BY THE FEDERAL HIGHWAY ADMINISTRATION.
- TRAFFIC CONTROL DEVICES SHALL BE SET UP JUST BEFORE THE START OF CONSTRUCTION OPERATIONS AND SHALL BE PROPERLY MAINTAINED DURING THE TIME SUCH CONDITIONS EXIST. THEY SHALL REMAIN IN PLACE ONLY AS LONG AS NEEDED AND REMOVED THEREAFTER.
- EXISTING SIGNS AND CONSTRUCTION SIGNS SHALL BE KEPT IN PROPER POSITION, AND BE CLEAN AND LEGIBLE AT ALL TIMES. SIGNS THAT DO NOT APPLY TO EXISTING CONDITIONS SHALL BE REMOVED. SIGNS THAT ARE DAMAGED, DEFACED, OR THAT ACCUMULATE DIRT DURING CONSTRUCTION SHALL BE CLEANED, REPAIRED, OR REPLACED.
- SIGNS ARE USUALLY MOUNTED ON A SINGLE POST, ALTHOUGH THOSE WIDER THAN 36" OR LARGER THAN 10 SQ. FT. SHALL BE MOUNTED ON TWO POSTS OR ABOVE A TYPE III BARRICADE.
- SIGN POSTS DIRECT BURIED IN SOIL SHALL BE 2 LB. MINIMUM CHANNEL POST OR 4"x4" WOOD POSTS. CHANNEL POSTS SHALL BE PAINTED GREEN. WOOD POSTS SHALL BE PAINTED WHITE. ALL POSTS SHALL BE NEATLY CONSTRUCTED, AND SHALL BE REPLUMBED, CLEANED, OR REPAIRED AS NEEDED FOR THE DURATION OF THE JOB. THERE SHALL NOT BE MORE THAN 2 POSTS IN A 7' PATH FOR WOOD OR CHANNEL POSTS. ANY CHANNEL POST SPLICE SHALL BE IN ACCORDANCE WITH STANDARD DRAWING TC-3.
- POST MOUNTED SIGNS IN RURAL AREAS SHALL BE CONSTRUCTED WITH THE NEAR EDGE OF THE SIGN FROM 6 TO 12 FEET FROM THE PAVEMENT EDGE. SIGNS IN URBAN AREAS AND BARRICADE MOUNTED SIGNS SHALL BE MOUNTED A MINIMUM OF 2 FEET FROM THE PAVEMENT EDGE.
- ALL POST AND BARRICADE MOUNTED SIGNS MOUNTED IN URBAN AREAS SHALL BE MOUNTED A MINIMUM DISTANCE OF 7' FROM THE BOTTOM OF THE SIGN TO THE ROADWAY SURFACE. ALL POST AND BARRICADE MOUNTED SIGNS MOUNTED IN RURAL AREAS SHALL BE MOUNTED A MINIMUM DISTANCE OF 7' FROM THE BOTTOM OF THE SIGN TO THE ROADWAY SURFACE, EXCEPT A MINIMUM OF 6' SHALL BE USED WHEN MOUNTING AN ADVISORY SIGN BELOW A WARNING SIGN. TEMPORARY SIGNS MAY BE MOUNTED ON PORTABLE SUPPORTS FOR INTERMEDIATE TERM STATIONARY WORK CONDITIONS. THE SIGNS MINIMUM MOUNTING HEIGHT SHALL BE 5'. RETROREFLECTIVE DEVICES SHALL BE USED. TEMPORARY SIGNS MAY BE MOUNTED ON PORTABLE SUPPORTS FOR SHORT-TERM, SHORT DURATION, AND MOBILE CONDITIONS. THEY SHALL BE NO LESS THAN ONE (1) FOOT ABOVE THE TRAVELED WAY. LONG-TERM STATIONARY SIGNS SHALL BE DIRECT BURIED IN SOIL, UNLESS CONDITIONS NECESSITATE THE USE OF PORTABLE SIGNS, OR AS APPROVED BY THE ENGINEER. CONCRETE PADS, CONCRETE OR ROCK BALLAST, OR OTHER SOLID MATERIALS SHALL NOT BE UTILIZED WITH PORTABLE SIGN SUPPORTS.
- FLAGGERS SHALL USE REFLECTORIZED STOP-SLOW PADDLES. FLAGS MAY BE USED ONLY FOR EMERGENCY SITUATIONS.
- MOST OF THE SIGNS SHOWN ARE ORIENTED TO THE RIGHT. HOWEVER, THIS DOES NOT PRECLUDE THE USE OF MIRROR IMAGES OF THESE SIGNS WHERE THE REVERSE ORIENTATION MIGHT BETTER CONVEY TO MOTORISTS THE PROPER DIRECTION OF MOVEMENT.
- R55-1 SIGNS SHALL BE PLACED AT LEAST 1500' BUT NOT MORE THAN 1 MILE IN ADVANCE OF THE WORK ZONE. IF A SPEED LIMIT REDUCTION IS IN EFFECT, THE SIGN SHALL BE PLACED A MINIMUM OF 500' IN ADVANCE OF THE "REDUCED SPEED AHEAD" SIGN.

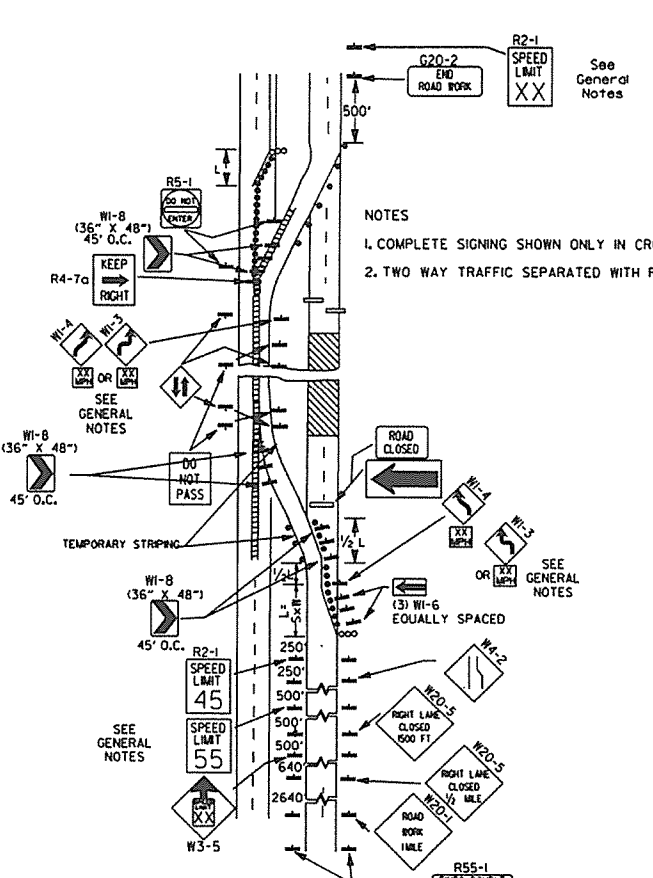
* NOTE: SUPPORTS FOR SIGNS, BARRICADES, AND VERTICAL PANELS THAT ARE DIFFERENT FROM THE REQUIREMENTS SHOWN IN NOTES 4 & 5, BUT MEET THE REQUIREMENTS OF NCHRP-350 OR MANUAL FOR ASSESSING SAFETY HARDWARE (MASH), WILL BE ACCEPTED. COMPLIANCE WITH THE REQUIREMENTS OF NCHRP-350 OR MANUAL FOR ASSESSING SAFETY HARDWARE (MASH) IS REQUIRED FOR ALL PROJECTS.

9-2-15	REVISED REDUCED SPEED LIMIT AHEAD SIGNS REVISED ROAD WORK NEXT XX MILES	
12-15-1	REVISED W24-1	
1-17-10	DELETED W8-9a & ADDED W8-9	
10-15-09	ADDED REFERENCE TO MASH & ADDED SIGN W24-1	
4-17-08	REVISED SIGN DESIGNATIONS	
1-18-04	REVISED NOTES	
10-9-03	REVISED NOTE 1	
1-16-01	REVISED NOTE 7	
9-28-00	REVISED NOTE	
1-18-98	ADDED NOTE	
6-26-97	REVISED NOTE 5	
4-03-97	REVISED NOTE 5	
10-18-96	ADDED CONTROLLED ACCESS HWY. SIGN & TO NOTE 7	
10-12-95	ADDED R55-1	
6-8-95	REVISED TO CORRECT SIGN ILLUSTRATIONS	6-8-95
2-2-95	REVISED PER PART VI, MUTCD SEPT. 3, 1993	
8-15-91	DRAWN AND PLACED IN USE	
DATE	REVISION	FILMED

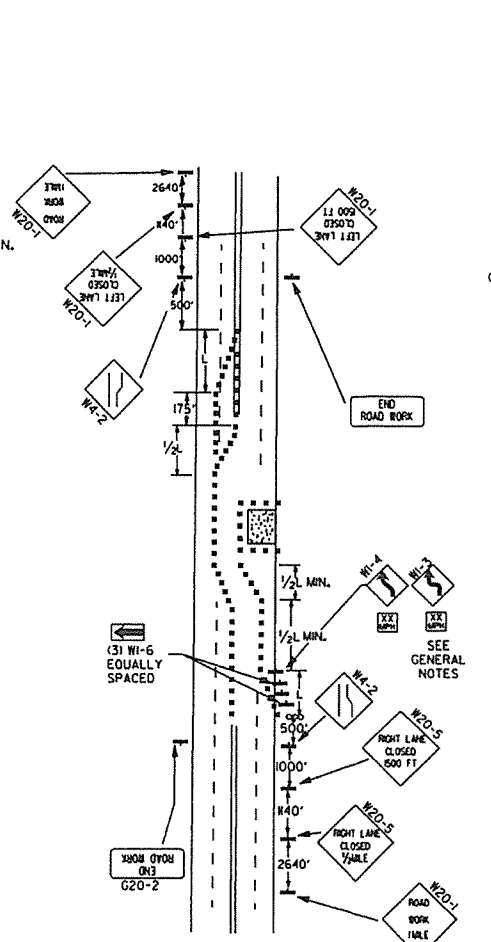
ARKANSAS STATE HIGHWAY COMMISSION
STANDARD TRAFFIC CONTROLS
FOR HIGHWAY CONSTRUCTION
STANDARD DRAWING TC-1



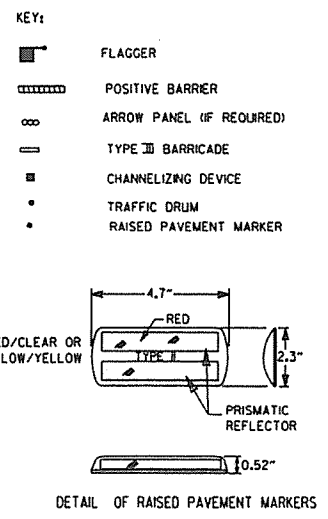
(A) TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES ON A 2-LANE HIGHWAY WHERE THE ENTIRE ROADWAY IS CLOSED AND A BYPASS DETOUR IS PROVIDED.



(B) TYPICAL APPLICATION - 4-LANE DIVIDED ROADWAY WHERE ONE ROADWAY IS CLOSED.



(C) TYPICAL APPLICATION - 4-LANE UNDIVIDED ROADWAY WHERE HALF OF THE ROADWAY IS CLOSED.



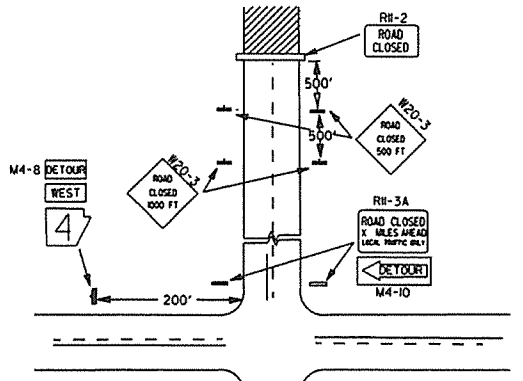
TYPICAL ADVANCE WARNING SIGN PLACEMENT

TAPER FORMULAE:
 L= SXW FOR SPEEDS OF 45MPH OR MORE.
 L= $\frac{WS^2}{60}$ FOR SPEEDS OF 40MPH OR LESS.
 WHERE:
 L= MINIMUM LENGTH OF TAPER.
 S= NUMERICAL VALUE OF POSTED SPEED LIMIT PRIOR TO WORK OR 85TH PERCENTILE SPEED.
 W= WIDTH OF OFFSET.

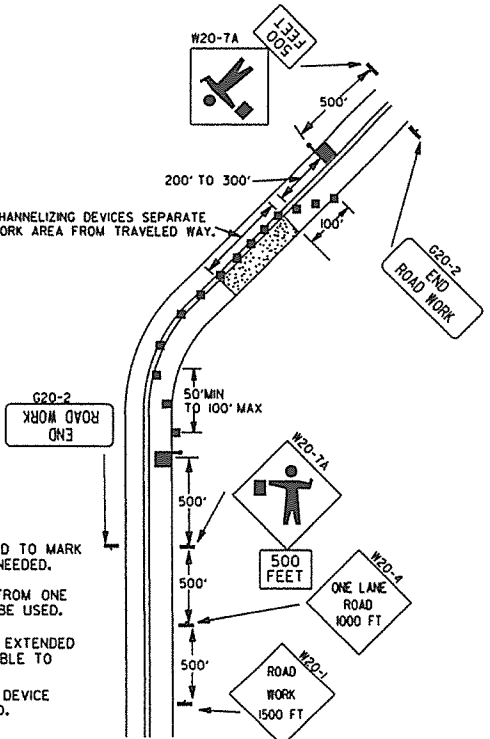
- GENERAL NOTES:
- ADVISORY SPEED POSTED ON W1-3 OR W1-4 CURVE WARNING SIGNS TO BE DETERMINED AT SITE. USE W1-4 WHEN SPEED IS GREATER THAN 30MPH AND W1-3 WHEN 30MPH OR LESS.
 - WHEN THE EXISTING SPEED LIMIT IS 55MPH AND THE PLANS REQUIRE A SPEED LIMIT OF 45MPH, THE R2-(K55) SHALL BE OMITTED AND THE W3-5 SHALL BE INSTALLED AT THAT LOCATION. ADDITIONAL R2-145MPH SPEED LIMIT SIGNS SHALL BE INSTALLED AT A MAXIMUM OF 1MILE INTERVALS. AT THE END OF THE WORK AREA A R2-(KXX) SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.
 - WHEN THE EXISTING SPEED LIMIT IS 65MPH AND THE PLANS REQUIRE A SPEED LIMIT OF 55MPH, THE R2-(K45) SHALL BE OMITTED. ADDITIONAL R2-155MPH SPEED LIMIT SIGNS SHALL BE INSTALLED AT A MAXIMUM OF 1MILE INTERVALS. AT THE END OF THE WORK AREA A R2-(KXX) SHALL BE INSTALLED TO MATCH ORIGINAL SPEED LIMIT.
 - THE MAXIMUM SPACING BETWEEN CHANNELIZING DEVICES IN A TAPER SHOULD BE APPROXIMATELY EQUAL IN FEET TO THE SPEED LIMIT. BEYOND THE TAPER, MAXIMUM SPACING SHALL BE TWO TIMES THE SPEED LIMIT, OR AS DIRECTED BY THE ENGINEER.
 - WARNING LIGHTS AND/OR FLAGS MAY BE MOUNTED TO SIGNS OR CHANNELIZING DEVICES AT NIGHT AS NEEDED.
 - PAVEMENT MARKINGS NO LONGER APPLICABLE WHICH MIGHT CREATE CONFUSION IN THE MINDS OF VEHICLE OPERATORS SHALL BE REMOVED OR OBLITERATED AS SOON AS PRACTICABLE.
 - TRAILER MOUNTED DEVICES SUCH AS ARROW PANELS AND PORTABLE CHANGEABLE MESSAGE SIGNS SHALL BE DELINEATED BY AFFIXING CONSPICUITY MATERIAL IN A CONTINUOUS LINE ON THE FACE OF THE TRAILER. WHEN PLACED ON OR ADJACENT TO THE SHOULDER AND NOT BEHIND A POSITIVE BARRIER, THESE DEVICES SHALL BE DELINEATED BY PLACING FIVE (5) TRAFFIC DRUMS, EQUALLY SPACED ALONG THE TRAFFIC SIDE OF THE DEVICE.
 - DIMENSIONS SHOWN FOR RAISED PAVEMENT MARKERS ARE TYPICAL. THE CONTRACTOR MAY SUBSTITUTE SIMILAR MARKERS WITH THE APPROVAL OF THE ENGINEER. REQUESTING APPROVAL FOR SIMILAR MARKERS MAY BE MADE BY REFERRING TO THE AHTD QUALIFIED PRODUCTS LIST.

9-2-15	REVISED NOTE 2, ADDED NOTE 8, REVISED DRAWING (A) & REPLACED R2-5A WITH W3-5	
9-12-13	REVISED DETAIL OF RAISED PAVEMENT MARKERS	
3-4-10	ADDED (AFAD)	
4-20-08	REVISED SIGN DESIGNATIONS	
4-18-04	ADDED GENERAL NOTE	
10-18-96	ADDED R55-1	
4-26-96	CORRECTED (C) BEHIND G20-2	
6-8-95	CORRECTED SIGN IDENT. ON W1-4A	6-8-95
2-2-95	REVISED PER PART VI, MUTCD, SEPT. 3, 1993	
8-15-91	DRAWN AND PLACED IN USE	
DATE	REVISION	FILED

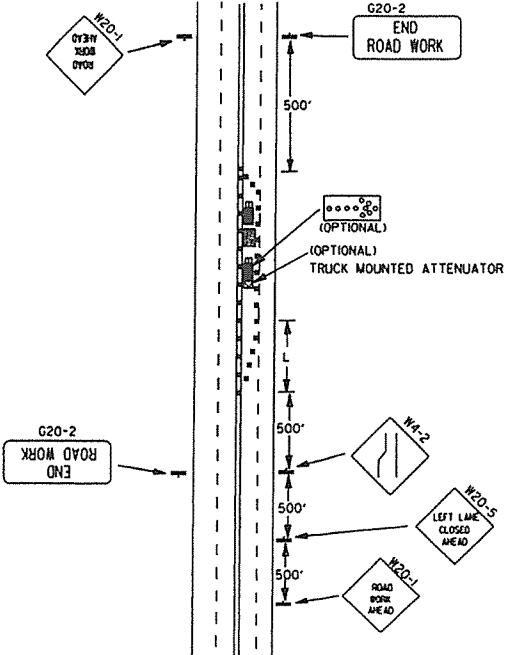
ARKANSAS STATE HIGHWAY COMMISSION
 STANDARD TRAFFIC CONTROLS
 FOR HIGHWAY CONSTRUCTION
 STANDARD DRAWING TC-2



(D) TYPICAL APPLICATION - ROADWAY CLOSED BEYOND DETOUR POINT.

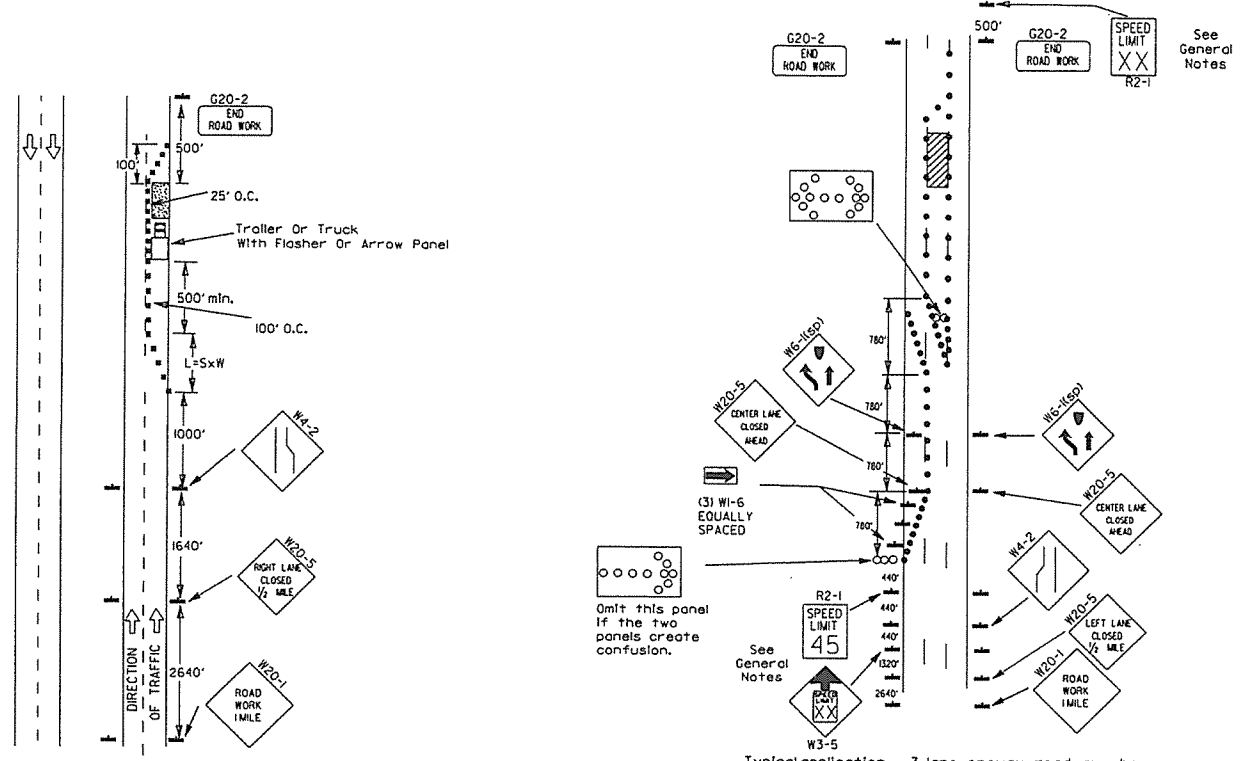


(E) TYPICAL APPLICATION OF TRAFFIC CONTROL DEVICES ON 2-LANE HIGHWAY WHERE ONE LANE IS CLOSED AND FLAGGING IS PROVIDED.

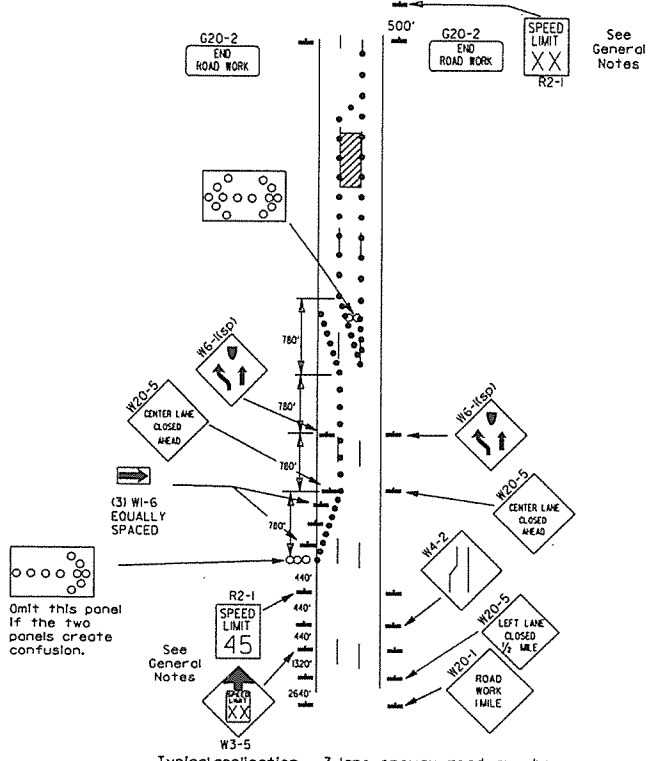


(F) TYPICAL APPLICATION - 4-LANE UNDIVIDED ROADWAY WITH INSIDE LANE CLOSED.

Channelizing devices



(A) Typical application - daytime maintenance operations of short duration on a 4-lane divided roadway where half of the roadway is closed.



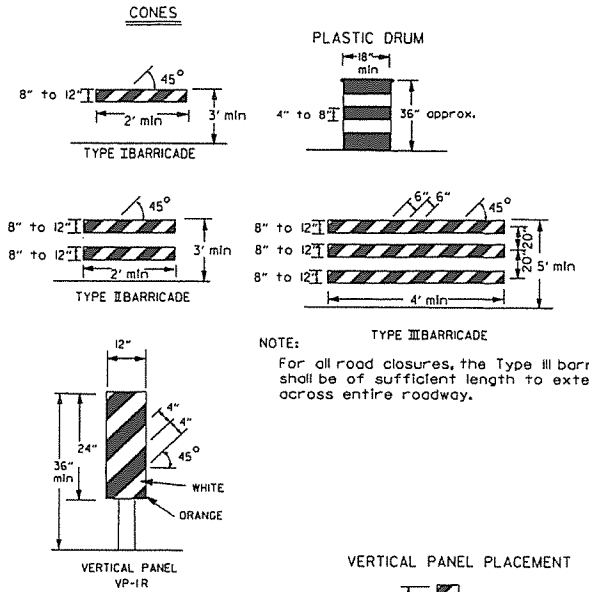
(B) Typical application - 3-lane one-way roadway where center lane is closed.

- KEY:
- Arrow Panel (if Required)
 - Channelizing Device
 - Traffic drum

GENERAL NOTES:

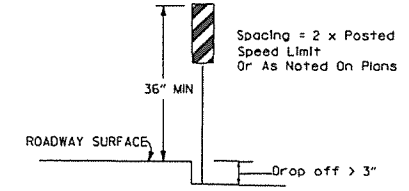
1. A speed limit reduction may be implemented ONLY when designated in the plan or when recommended by the Roadway Design Division.
2. When the existing speed limit is 55mph and the plans require a speed limit of 45mph, the R2-1(55) shall be omitted and the W3-5 shall be installed at that location. Additional R2-145mph speed limit signs shall be installed at a maximum of 1/2 mile intervals. At the end of the work area a R2-1(XX) shall be installed to match original speed limit.
3. When the existing speed limit is 65mph and the plans require a speed limit of 55mph, the R2-1(65) shall be omitted. Additional R2-155mph speed limit signs shall be installed at a maximum of 1/2 mile intervals. At the end of the work area a R2-1(XX) shall be installed to match original speed limit.
4. The maximum spacing between channelizing devices in a taper should be approximately equal in feet to the speed limit. Beyond the taper, maximum spacing shall be two times the speed limit or as directed by the Engineer.
5. Warning lights and/or flags may be mounted to signs or channelizing devices at night as needed.
6. Pavement markings no longer applicable which might create confusion in the minds of vehicle operators shall be removed or obliterated as soon as practicable.
7. The G20-1 sign will be required on jobs of over two miles in length. When the lane closure is not at the beginning of the project, the G20-1 sign shall be erected 125' in advance of the job limit. Additional W20-1 (1/2 MILE) signs are not required in advance of lane closures that begin inside the project limits.
8. Flaggers shall use STOP/SLOW paddles for controlling traffic through work zones. Flags may be used only for emergency situations.
9. All plastic drums and cones shall meet the requirements of NCHRP-350 or Manual For Assessing Safety Hardware (MASH).
10. Trailer mounted devices such as arrow panels and portable changeable message signs shall be delineated by affixing conspicuity material in a continuous line on the face of the trailer. When placed on or adjacent to the shoulder and not behind a positive barrier, these devices shall be delineated by placing five (5) traffic drums, equally spaced along the traffic side of the device.

When cones are used on freeways and multi-lane highways, they shall be 28" min. During hours of darkness, 28" cones shall be used on all roadways, and shall be reflectorized in accordance with the M.U.T.C.D.



NOTE: For all road closures, the Type III barricades shall be of sufficient length to extend across entire roadway.

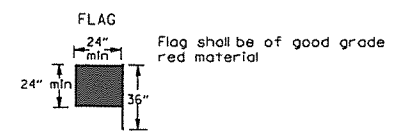
VERTICAL PANEL PLACEMENT



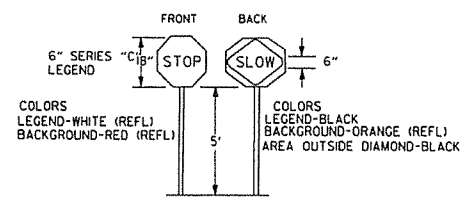
TRAFFIC CONTROL DEVICES FOR VERTICAL PAVEMENT DIFFERENTIALS

VERTICAL DIFFERENTIAL	LOCATIONS	TRAFFIC CONTROL
1" to 3"	Centerline, lane lines	WB-11
1" to 3"	Edge of shoulder	WB-9
Greater than 3"	Lane lines	Standard lane closure required
Greater than 3"	Edge of traveled lane	*RSP-land vertical panels, drums or concrete barrier
Greater than 3"	Edge of shoulder	*Vertical panels, drums or concrete barrier

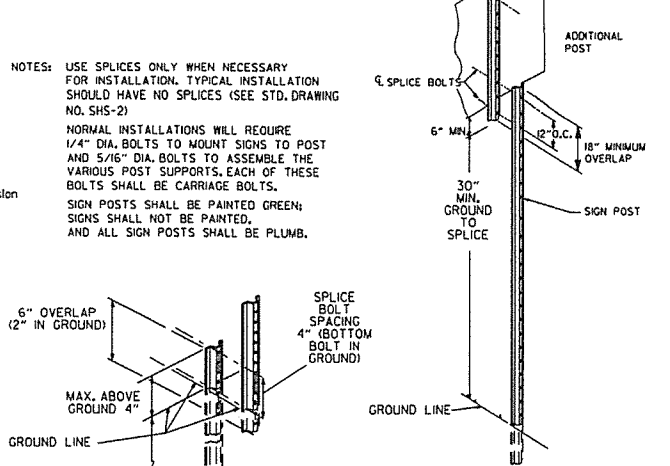
* When shown on the plans concrete barrier will be used. When the shoulder area is used as part of the traveled lane and there is insufficient width to place drums on the remaining shoulder width, then vertical panels shall be used.



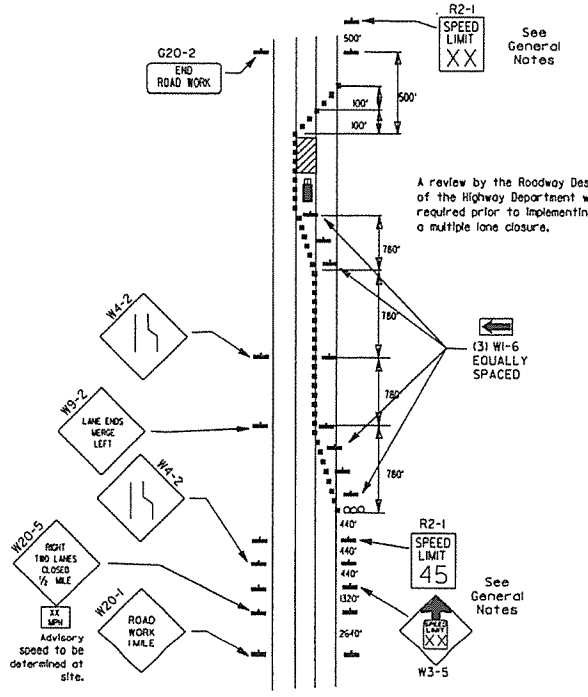
STOP SLOW PADDLE



DETAIL OF SPLICES



- NOTES:
- USE SPLICES ONLY WHEN NECESSARY FOR INSTALLATION. TYPICAL INSTALLATION SHOULD HAVE NO SPLICES (SEE STD. DRAWING NO. SHS-2)
 - NORMAL INSTALLATIONS WILL REQUIRE 1/4" DIA. BOLTS TO MOUNT SIGNS TO POST AND 5/16" DIA. BOLTS TO ASSEMBLE THE VARIOUS POST SUPPORTS. EACH OF THESE BOLTS SHALL BE CARRIAGE BOLTS.
 - SIGN POSTS SHALL BE PAINTED GREEN; SIGNS SHALL NOT BE PAINTED, AND ALL SIGN POSTS SHALL BE PLUMB.



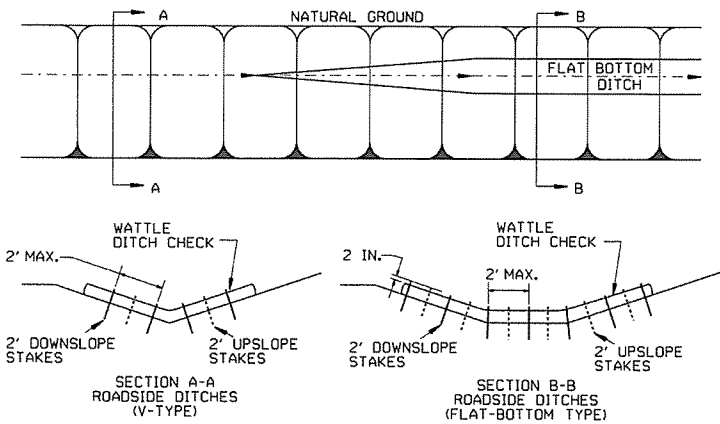
(D) Typical application - closing multiple lanes of a multi-lane highway.

(C) Typical application - construction operations of intermediate to long term duration on a 4-lane divided roadway where half of the roadway is closed.

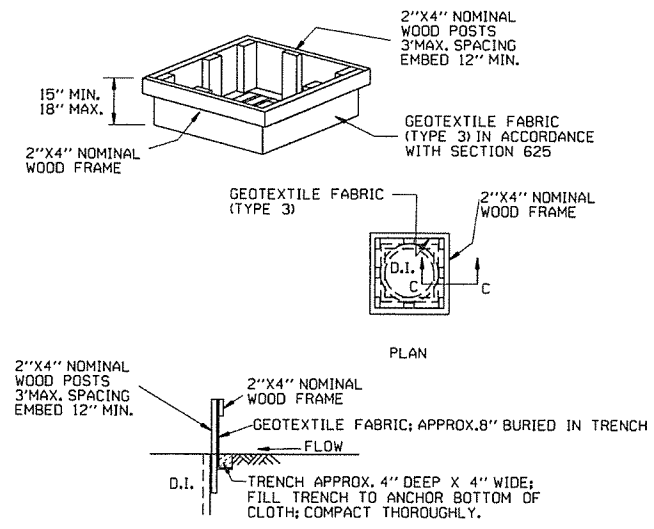
DATE	REVISION	FILED
9-2-15	REVISED NOTE 2 & REPLACED R2-5A WITH W3-5	
10-15-09	ADDED REFERENCE TO MASH	
11-20-08	REVISED SIGN DESIGNATIONS	
11-18-04	ADDED NOTE	
10-1-98	ADDED NOTE	
4-03-97	ADDED (SPI) TO W6-1 & REVISED TRAFFIC CONTROL DEVICES NOTE	
10-18-96	ADDED R55-1	
10-12-95	MOVED UPPER SPLICE	
6-8-95	REVISED SPLICE DETAIL, TEXT	6-8-95
2-2-95	REVISED PER PART VI, MUTCD, SEPT. 3, 1993	
8-15-91	DRAWN AND PLACED IN USE	

ARKANSAS STATE HIGHWAY COMMISSION
STANDARD TRAFFIC CONTROLS FOR HIGHWAY CONSTRUCTION
STANDARD DRAWING TC-3

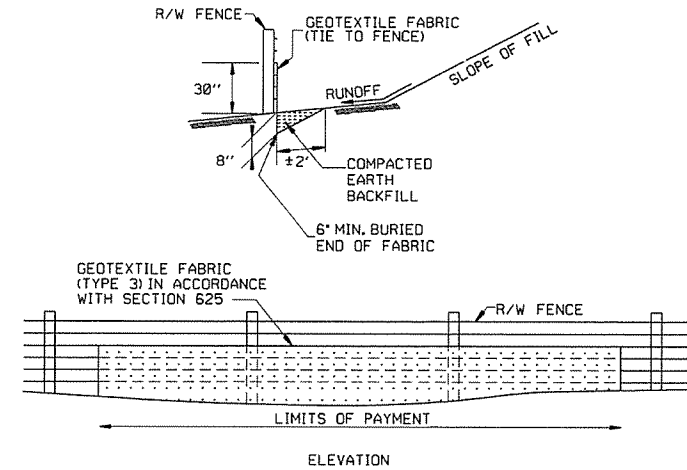
GENERAL NOTES
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.



WATTLE DITCH CHECK (E-1)



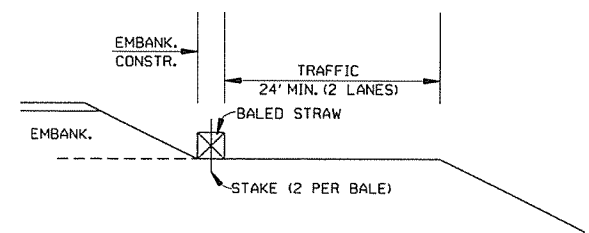
DROP INLET SILT FENCE (E-7)



SILT FENCE ON R/W FENCE (E-4)

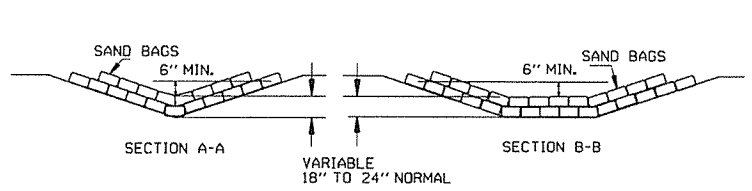
GENERAL NOTES
GEOTEXTILE FABRIC SHALL BE SPICED TOGETHER WITH A SEWN SEAM ONLY AT A SUPPORT POST, OR TWO SECTIONS OF FENCE MAY BE OVERLAPPED INSTEAD. PAYMENT OF ADDITIONAL MATERIAL FOR OVERLAP WILL NOT BE MADE.

GENERAL NOTES
1. STRAW BALES SHALL BE INSTALLED SO THAT THE BINDINGS ARE ORIENTED AROUND THE SIDES RATHER THAN ALONG THE TOPS AND BOTTOMS OF THE BALES. THE BALES SHALL BE A MINIMUM OF 30 INCHES IN LENGTH.
2. NO GAPS SHALL BE LEFT BETWEEN BALES.
3. BALED STRAW FILTER BARRIERS COMPLETED AND ACCEPTED WILL BE MEASURED BY THE BALE IN PLACE AS AUTHORIZED BY THE ENGINEER AND WILL BE PAID FOR AT THE CONTRACT UNIT PRICE BID PER BALE FOR BALED STRAW DITCH CHECKS.

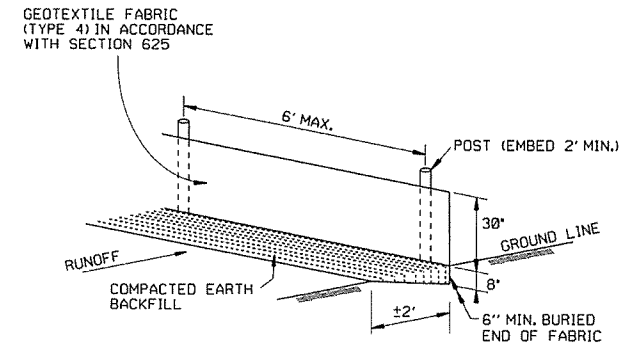


BALED STRAW FILTER BARRIER (E-2)

NUMBER OF SAND BAGS AND ARRANGEMENT VARIABLE WITH ON-SITE CONDITIONS. PLACE SAND BAGS AT BASE OF DITCH CHECK IN AREA OF OVERFLOW.

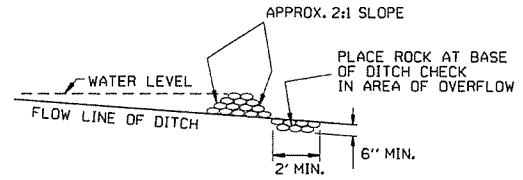


SAND BAG DITCH CHECK (E-5)



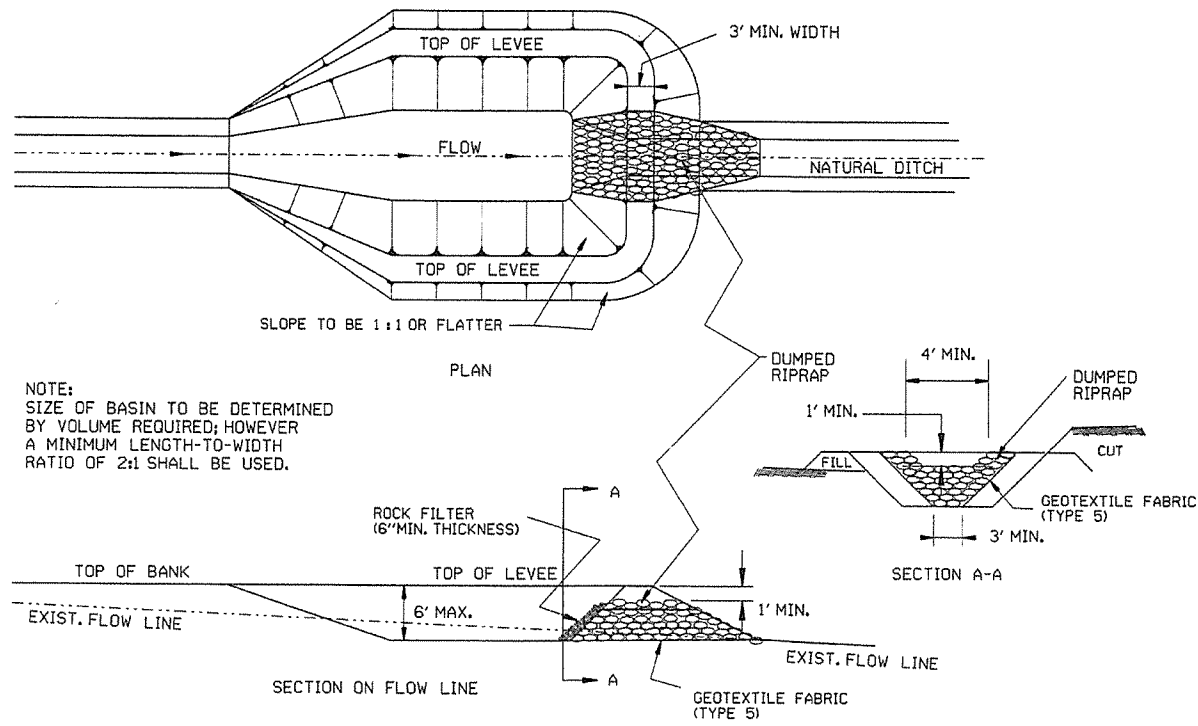
SILT FENCE (E-11)

GENERAL NOTES
GEOTEXTILE FABRIC SHALL BE SPICED TOGETHER WITH A SEWN SEAM ONLY AT A SUPPORT POST, OR TWO SECTIONS OF FENCE MAY BE OVERLAPPED INSTEAD. PAYMENT OF ADDITIONAL MATERIAL FOR OVERLAP WILL NOT BE MADE.



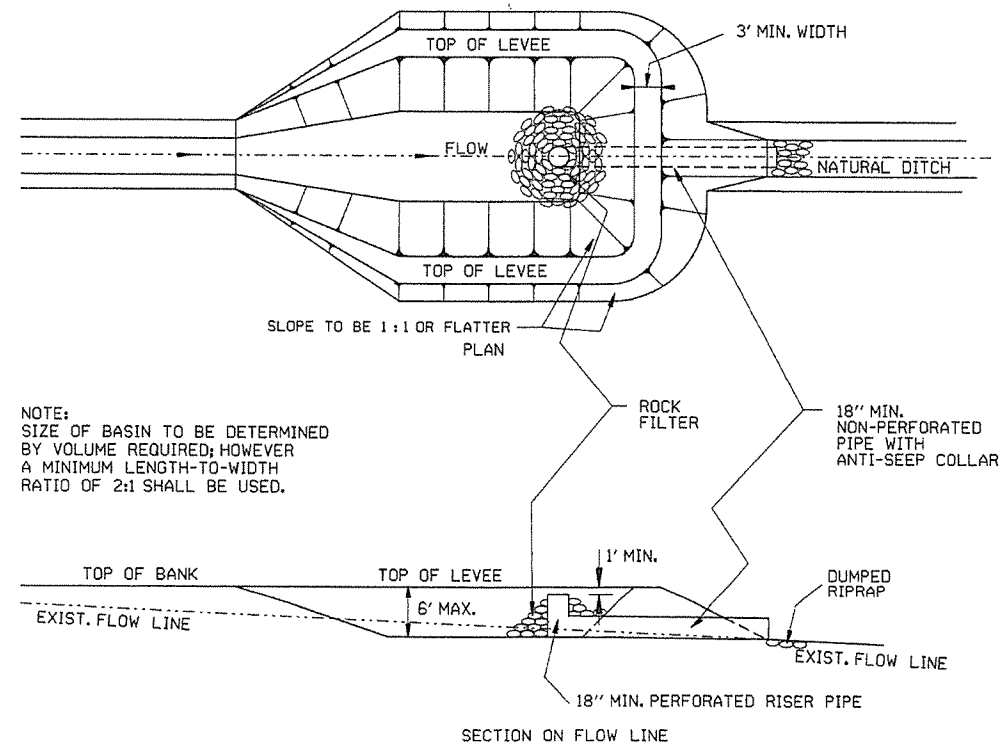
ROCK DITCH CHECK (E-6)

12-15-11	DELETED BALED STRAW DITCH CHECK & ADDED WATTLE DITCH CHECK		ARKANSAS STATE HIGHWAY COMMISSION
11-18-98	ADDED NOTES		TEMPORARY EROSION CONTROL DEVICES
7-02-98	ADDED BALED STRAW FILTER BARRIER (E-2)		
7-20-95	REVISED SILT FENCE E-4 AND E-11	7-20-95	STANDARD DRAWING TEC-1
7-15-94	REV. E-4 & E-11 MIN. 13" BURIED END OF FABRIC		
6-2-94	REVISED E-1, 4, 7 & 11; DELETED E-2 & 3	6-2-94	
4-1-93	REDRAWN		
10-1-92	REDRAWN		
8-2-76	ISSUED R.D.M.	298-7-28-76	
DATE	REVISION	FILMED	



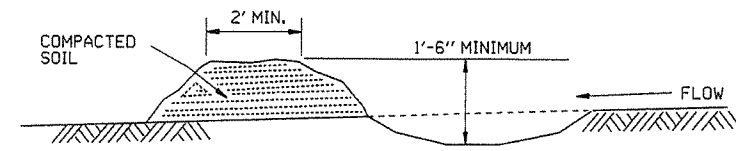
NOTE:
SIZE OF BASIN TO BE DETERMINED
BY VOLUME REQUIRED; HOWEVER
A MINIMUM LENGTH-TO-WIDTH
RATIO OF 2:1 SHALL BE USED.

SEDIMENT BASIN WITH RIPRAP OUTLET (E-9)

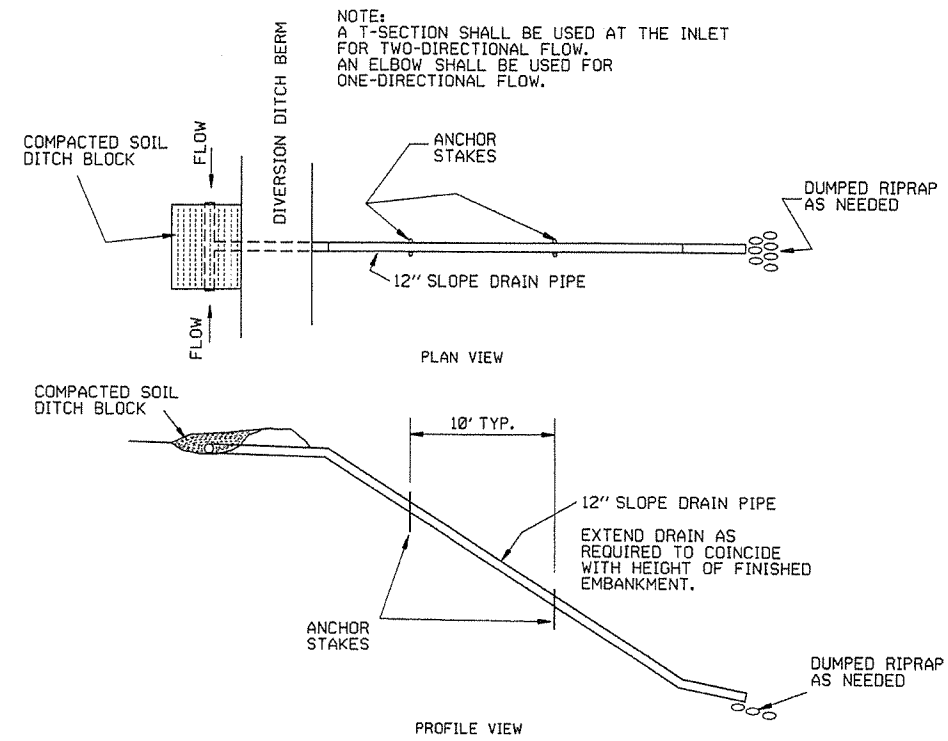


NOTE:
SIZE OF BASIN TO BE DETERMINED
BY VOLUME REQUIRED; HOWEVER
A MINIMUM LENGTH-TO-WIDTH
RATIO OF 2:1 SHALL BE USED.

SEDIMENT BASIN WITH PIPE OUTLET (E-10)

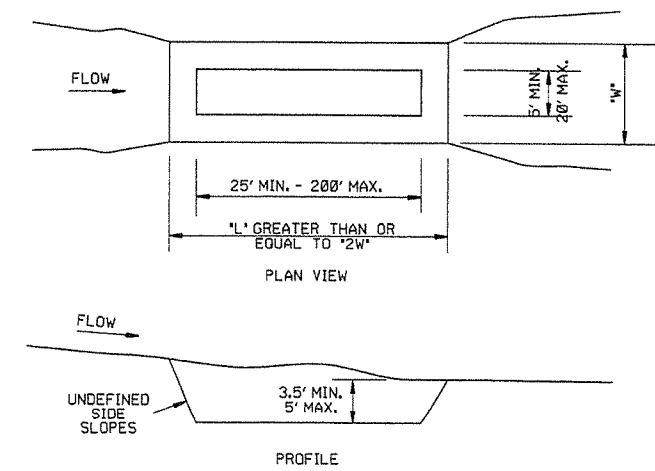


DIVERSION DITCH (E-8)



NOTE:
A T-SECTION SHALL BE USED AT THE INLET
FOR TWO-DIRECTIONAL FLOW.
AN ELBOW SHALL BE USED FOR
ONE-DIRECTIONAL FLOW.

SLOPE DRAIN (E-12)



SEDIMENT BASIN (E-14)

6-2-94	Revised E-8 & E-12; Added E-14 & Deleted E-13		
4-1-93	ISSUED		
DATE	REVISION		FILMED

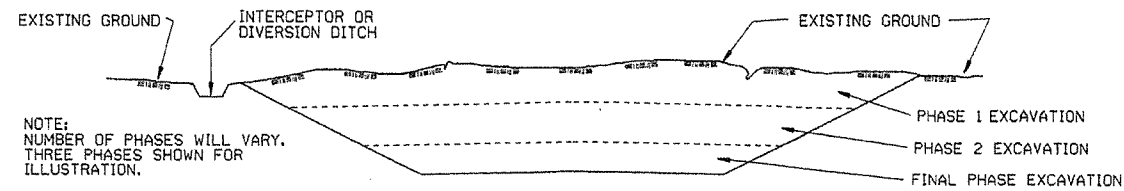
ARKANSAS STATE HIGHWAY COMMISSION
 TEMPORARY EROSION
 CONTROL DEVICES
 STANDARD DRAWING TEC-2

CLEARING AND GRUBBING

CONSTRUCTION SEQUENCE

1. PLACE PERIMETER CONTROLS (I.E. SILT FENCES, DIVERSION DITCHES, SEDIMENT BASINS, ETC.)
2. PERFORM CLEARING AND GRUBBING OPERATION.

EXCAVATION



NOTE:
NUMBER OF PHASES WILL VARY.
THREE PHASES SHOWN FOR
ILLUSTRATION.

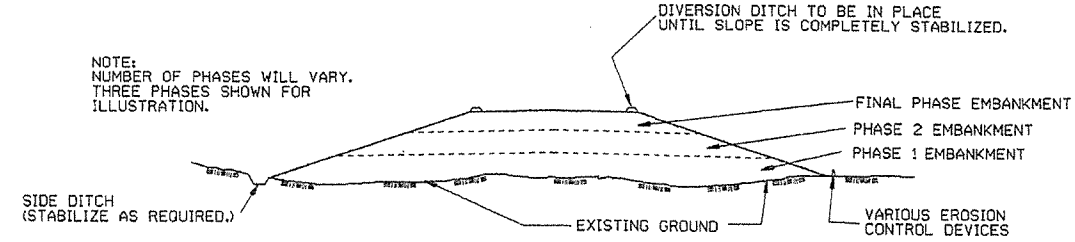
GENERAL NOTE

ALL CUT SLOPES SHALL BE DRESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE EXCAVATED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

CONSTRUCTION SEQUENCE

1. EXCAVATE AND STABILIZE INTERCEPTOR AND/OR DIVERSION DITCHES.
2. PERFORM PHASE 1 EXCAVATION. PLACE PERMANENT OR TEMPORARY SEEDING.
3. PERFORM PHASE 2 EXCAVATION. PLACE PERMANENT OR TEMPORARY SEEDING.
4. PERFORM FINAL PHASE OF EXCAVATION. PLACE PERMANENT OR TEMPORARY SEEDING. STABILIZE DITCHES. CONSTRUCT DITCH CHECKS, DIVERSION DITCHES, SEDIMENT BASINS, OR OTHER EROSION CONTROL DEVICES AS REQUIRED.

EMBANKMENT



NOTE:
NUMBER OF PHASES WILL VARY.
THREE PHASES SHOWN FOR
ILLUSTRATION.

GENERAL NOTE

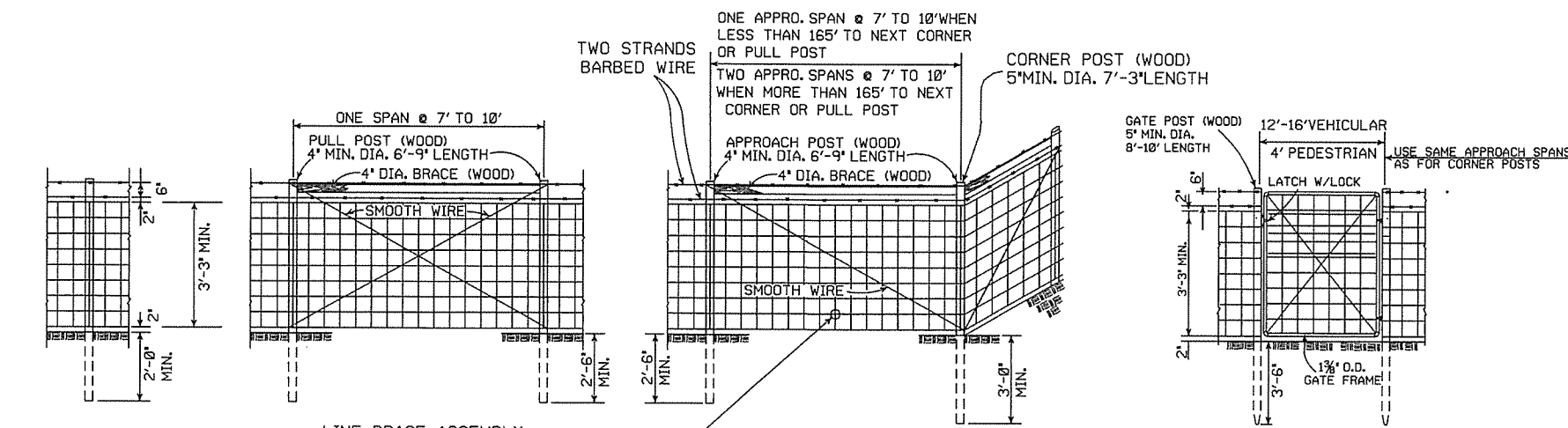
ALL EMBANKMENT SLOPES SHALL BE DRESSED, PREPARED, SEEDED, AND MULCHED AS THE WORK PROGRESSES. SLOPES SHALL BE CONSTRUCTED AND STABILIZED IN EQUAL INCREMENTS NOT TO EXCEED 25 FEET, MEASURED VERTICALLY.

CONSTRUCTION SEQUENCE

1. CONSTRUCT DIVERSION DITCHES, DITCH CHECKS, SEDIMENT BASINS, SILT FENCES, OR OTHER EROSION CONTROL DEVICES AS SPECIFIED.
2. PLACE PHASE 1 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.
3. PLACE PHASE 2 EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PROVIDE DIVERSION DITCHES AND SLOPE DRAINS IF EMBANKMENT CONSTRUCTION IS TO BE TEMPORARILY ABANDONED FOR A PERIOD OF GREATER THAN 21 DAYS.
4. PLACE FINAL PHASE OF EMBANKMENT WITH PERMANENT OR TEMPORARY SEEDING. PLACE DIVERSION DITCHES AND SLOPE DRAINS AND MAINTAIN UNTIL ENTIRE SLOPE IS STABILIZED.

60

ARKANSAS STATE HIGHWAY COMMISSION			
TEMPORARY EROSION CONTROL DEVICES			
STANDARD DRAWING TEC-3			
11-03-94	CORRECTED SPELLING		
6-2-94	Drawn & Issued		6-2-94
DATE	REVISION		FILMED

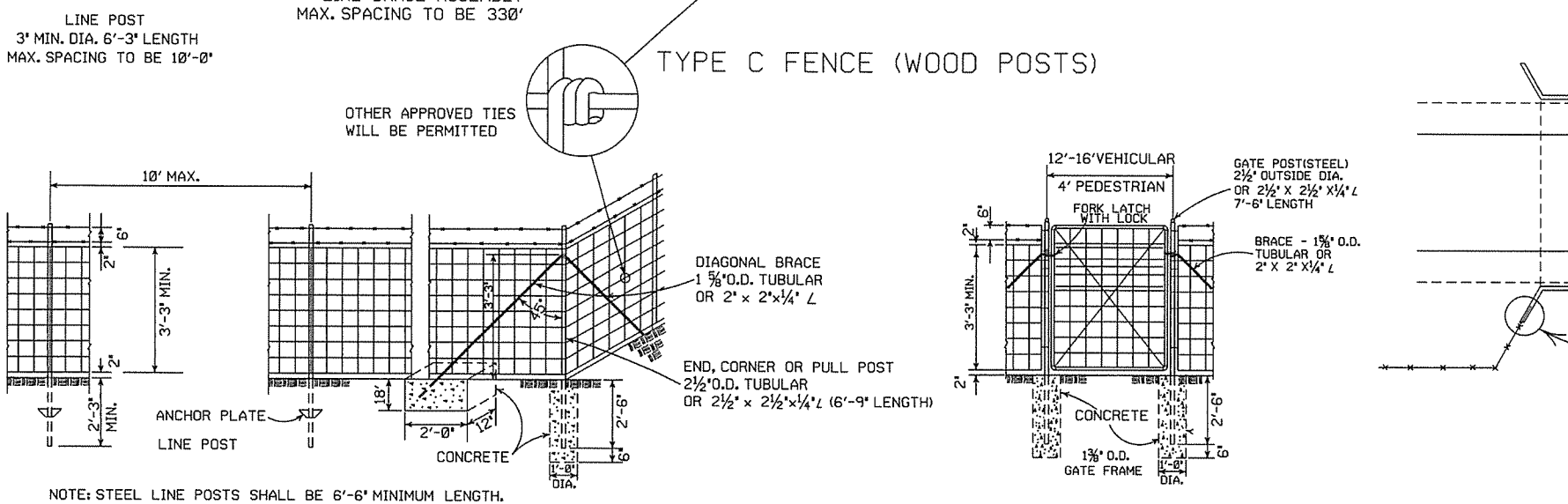


GENERAL NOTES:
 STEEL LINE POSTS SHALL BE PAINTED OR GALVANIZED. TUBULAR END, CORNER, PULL, OR DIAGONAL BRACES MUST CONFORM TO THE DIMENSIONS AND WEIGHTS SPECIFIED ON STANDARD DRAWING WF-3 (CHAIN LINK). APPROVED ALTERNATES ARE ACCEPTABLE.
 AN ACCEPTABLE TOLERANCE IN LENGTH OF TUBULAR OR WOODEN POSTS SHALL BE - 1" TO +2".
 TUBULAR POSTS MUST BE PAINTED OR GALVANIZED.

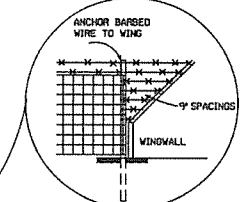
THE CONTRACTOR SHALL FURNISH AT LEAST 25% OF TIMBER LINE POSTS OF 7 FOOT LENGTHS IN ORDER TO PROVIDE SUFFICIENT SET IN SOFT GROUND OR SMALL DEPRESSIONS.

DRIVEWAY GATES, EITHER SINGLE 12' TO 16' OR DOUBLE 6' TO 8' OPENING OF THE SAME TYPE AS THE PEDESTRIAN GATE, SHALL BE INSTALLED ON THE RIGHT SIDE OF EACH THROUGH LANE ROAD AT LARGE CULVERTS OR BRIDGE CROSS FENCE, FOR USE OF MAINTENANCE EQUIPMENT. LOCATION OF GATES TO BE SHOWN ON PLANS OR AS DESIGNATED BY THE ENGINEER.

AT STREAM CROSSINGS, THE FENCE SHALL NOT BE CONSTRUCTED ACROSS LARGE STREAMS. WHERE CLEARANCE IS SUFFICIENT FROM THE TOP OF THE BANK TO THE BRIDGE STRUCTURE A CROSS CONNECTION SHALL BE CONSTRUCTED BETWEEN THE FENCE ON EACH SIDE OF THE ROAD. WHERE THE CLEARANCE IS NOT SUFFICIENT, THE FENCE SHALL BE TERMINATED WITH CROSS CONNECTIONS AND END POSTS ADJACENT TO BRIDGE ABUTMENTS OR CULVERT WINGWALLS.



NOTE: USE 3/8" x 1 1/2" LAG BOLT & SHIELD OR AS APPROVED BY THE ENGINEER.



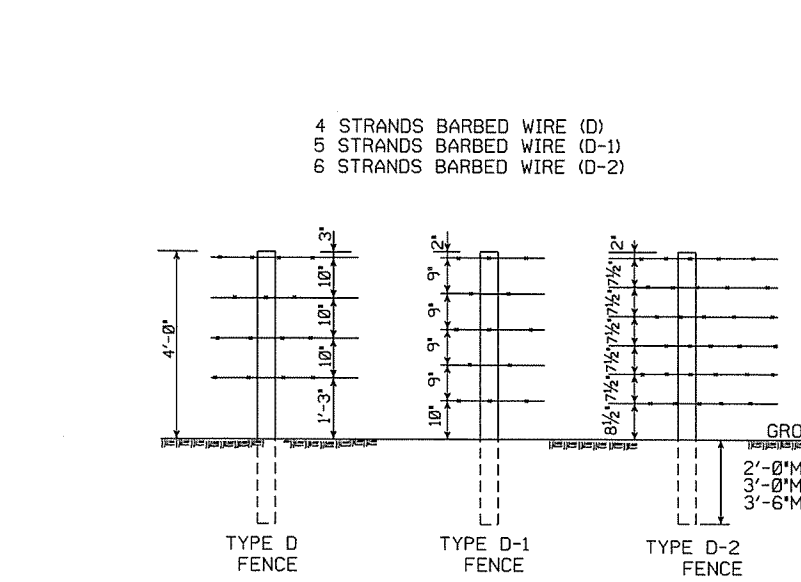
DETAIL OF FENCE CONSTRUCTION AT LARGE CULVERTS (5' IN HEIGHT AND OVER)

SPLICE FOR BARBED WIRE BETWEEN PULL POST ASSEMBLY SHALL BE BY THE 'EYE METHOD' AS DESCRIBED AS FOLLOWS: THE ENDS OF THE BARBED WIRE SHALL BE BENT TO FORM A LOOP. THE LOOPS SHALL BE CONNECTED. AFTER THE LOOPS ARE CONNECTED THE ENDS OF THE WIRE SHALL BE WRAPPED AROUND THE PROJECTING WIRES A MINIMUM OF 4 TIMES FOR EACH WIRE LOOP.

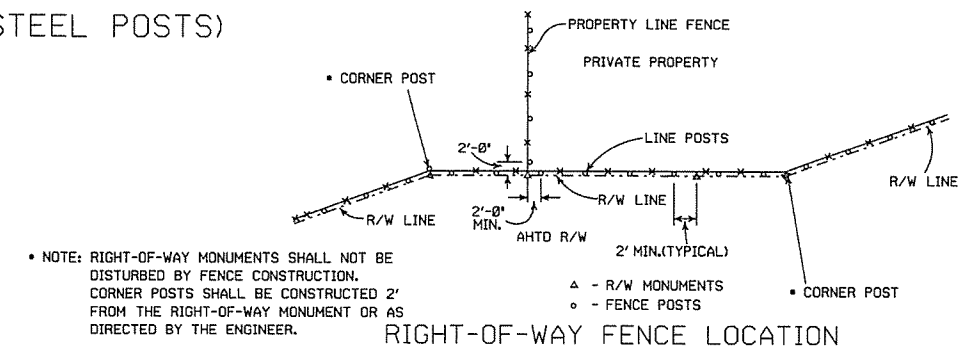
SPLICE FOR WOVEN WIRE BETWEEN PULL POST SHALL BE BY THE 'WESTERN UNION METHOD' AS DESCRIBED AS FOLLOWS: THE VERTICAL WIRES FOR EACH END OF THE FENCE FABRIC SHALL BE PLACED SIDE BY SIDE AND THE PROJECTING HORIZONTAL WIRES SHALL BE WRAPPED A MINIMUM OF 4 TIMES AROUND THE HORIZONTAL WIRES OF THE FIRST WEB.

STAPLE AT LEAST TOP, BOTTOM AND ALTERNATE WIRES OF WOVEN FABRIC FOR WOOD LINE POSTS.

TYPE C FENCE (STEEL POSTS)

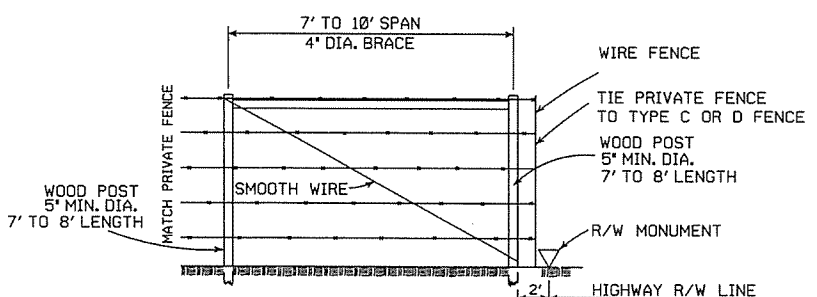


NOTE: SPACING AND SIZE (EXCEPT LENGTH) OF POSTS, APPROACH SPANS, PULL POST ASSEMBLIES, AND CORNER BRACING FOR TYPE D FENCE SHALL CONFORM TO TYPE C FENCE. USE GALVANIZED STAPLES ON WOOD POSTS AND APPROVED FASTENERS ON STEEL POSTS.

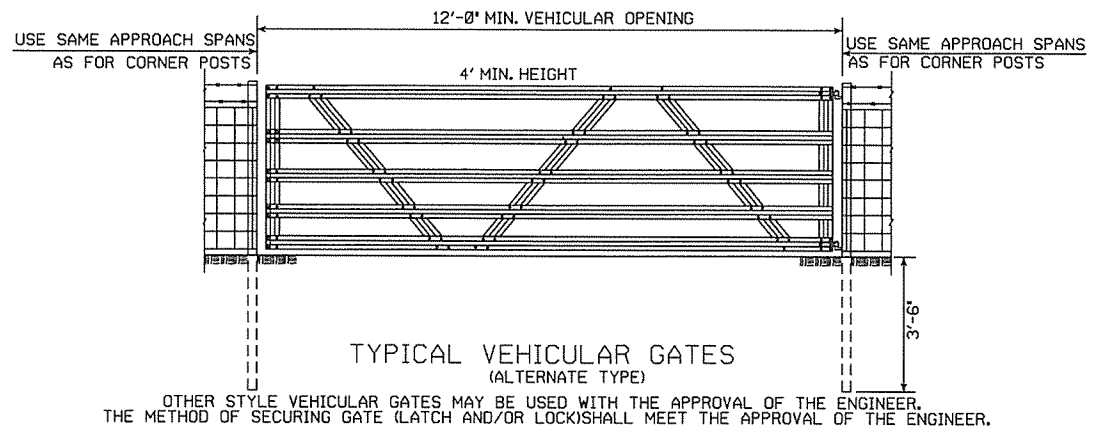


NOTE: RIGHT-OF-WAY MONUMENTS SHALL NOT BE DISTURBED BY FENCE CONSTRUCTION. CORNER POSTS SHALL BE CONSTRUCTED 2' FROM THE RIGHT-OF-WAY MONUMENT OR AS DIRECTED BY THE ENGINEER.
 ▲ - R/W MONUMENTS
 ○ - FENCE POSTS

RIGHT-OF-WAY FENCE LOCATION



PRIVATE FENCE TERMINAL INSTALLATION
 WHERE EXISTING FENCE CONSISTS OF STEEL POSTS, USE END POST ASSEMBLY AS SHOWN IN TYPE C FENCE OR OTHER END POST ASSEMBLY AS APPROVED BY THE ENGINEER.



8-22-02	REVISED GENERAL NOTES	
10-18-96	REVISED AASHTO	
11-22-95	REVISED R-O-W LOCATION DETAIL	
6-2-94	REVISED BARB WIRE AND ADDED CORNER POST NOTES	6-2-94
8-5-93	REVISED R/W INSTALLATION FENCE	8-5-93
10-1-92	ADDED STAPLE NOTE	10-1-92
8-15-91	ADDED TYPE D-2 FENCE	8-15-91
11-30-89	DELETED CLASS CONCRETE	11-30-89
7-15-88	ADDED SPLICE NOTE	700-7-15-88
10-30-87	GENERAL REVISIONS	549-10-30-87
11-1-84	MAX. POST SPACING MIN. WIRE GAUGE	507-11-1-84
1-4-83	MIN. DIA. LINE POST	648-1-4-83
3-2-81	TOLERANCE FOR POST LENGTH	722-3-2-81
12-1-72	ADDED D-1 & FENCE INSTALLATION	564-12-1-72
10-2-72	REVISED AND REDRAWN	540-10-2-72
DATE	REVISION	FILMED

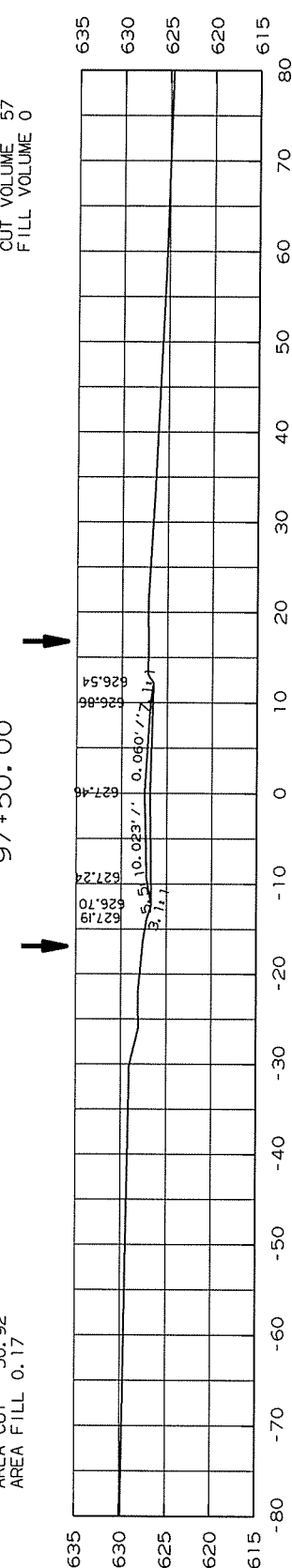
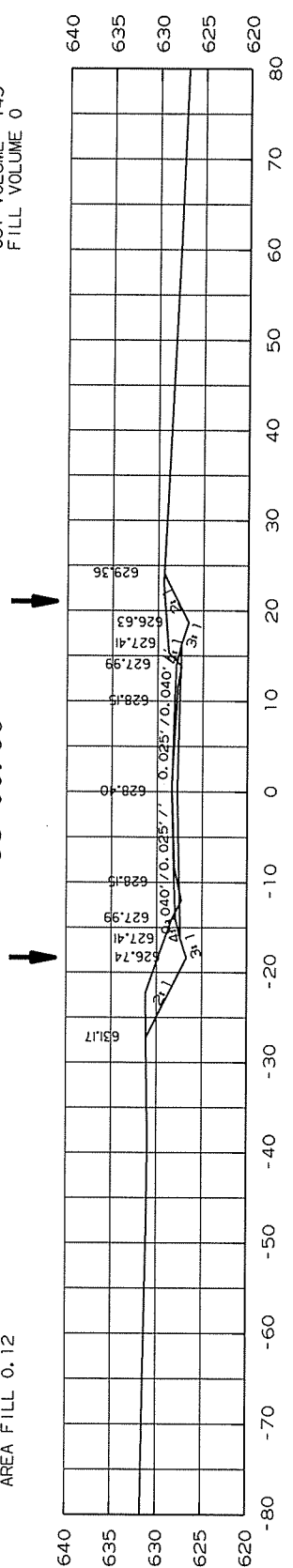
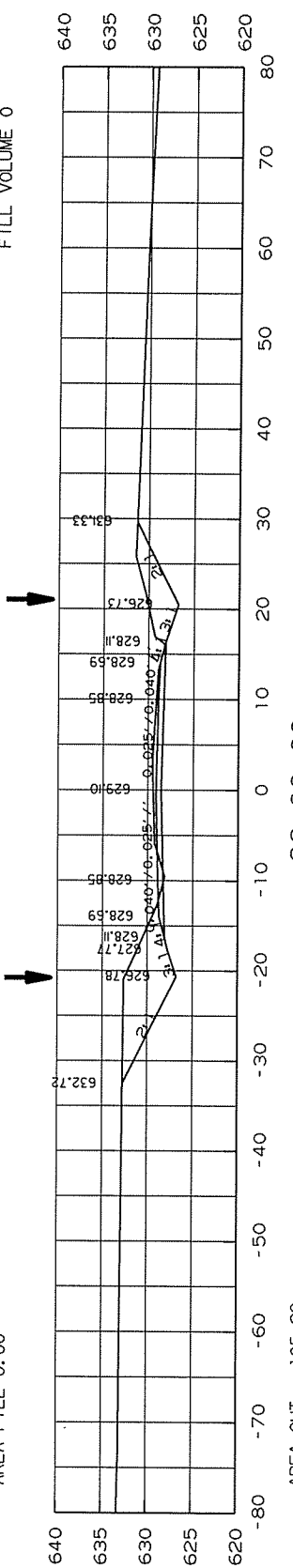
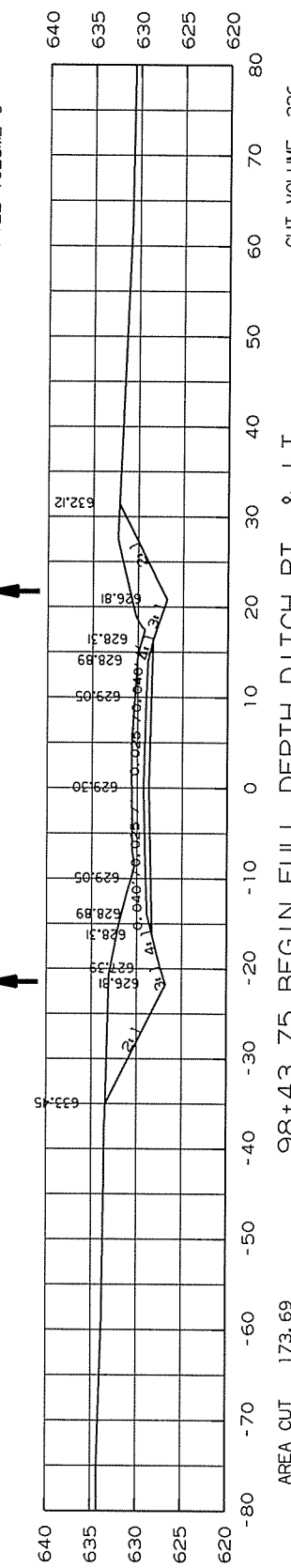
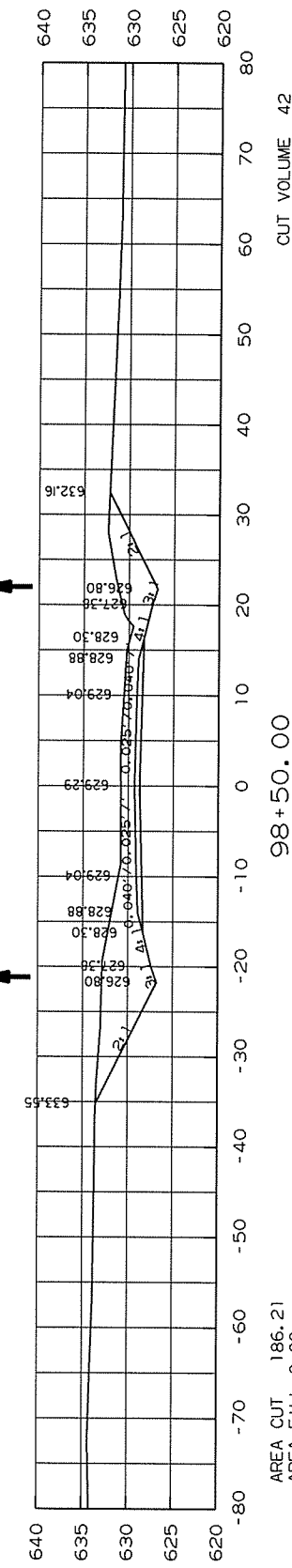
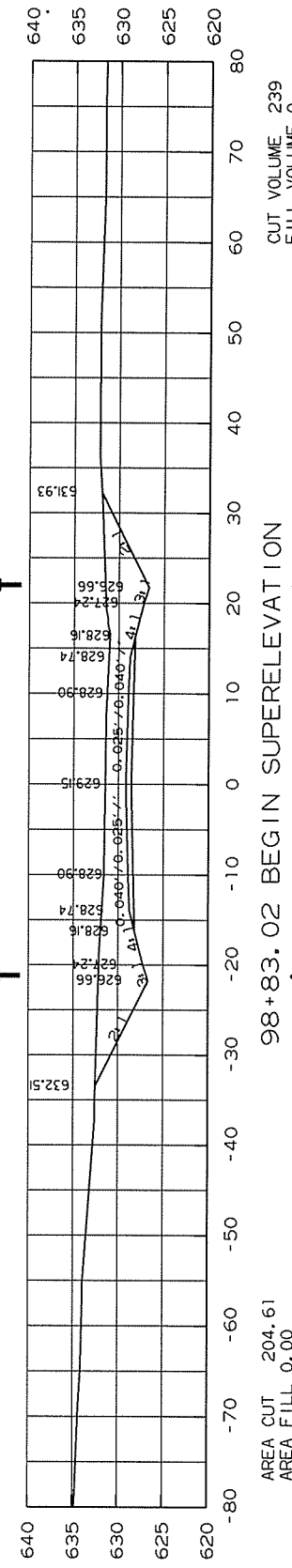
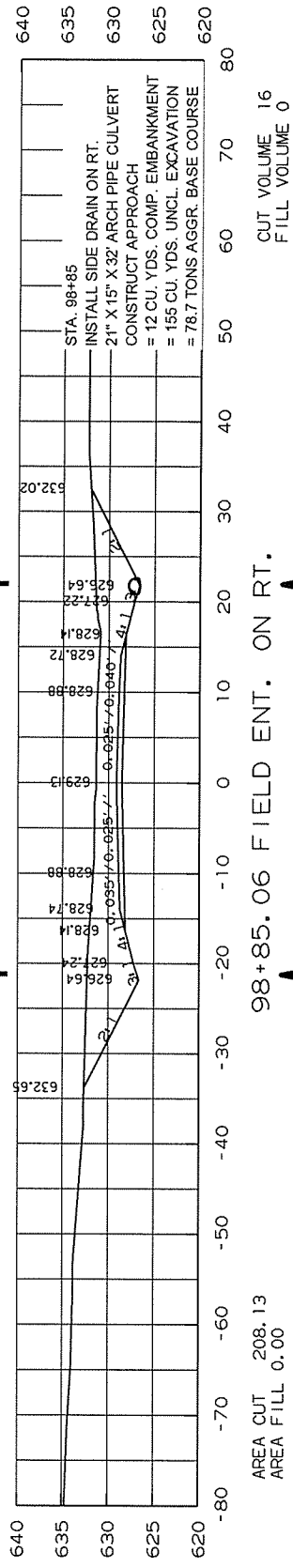
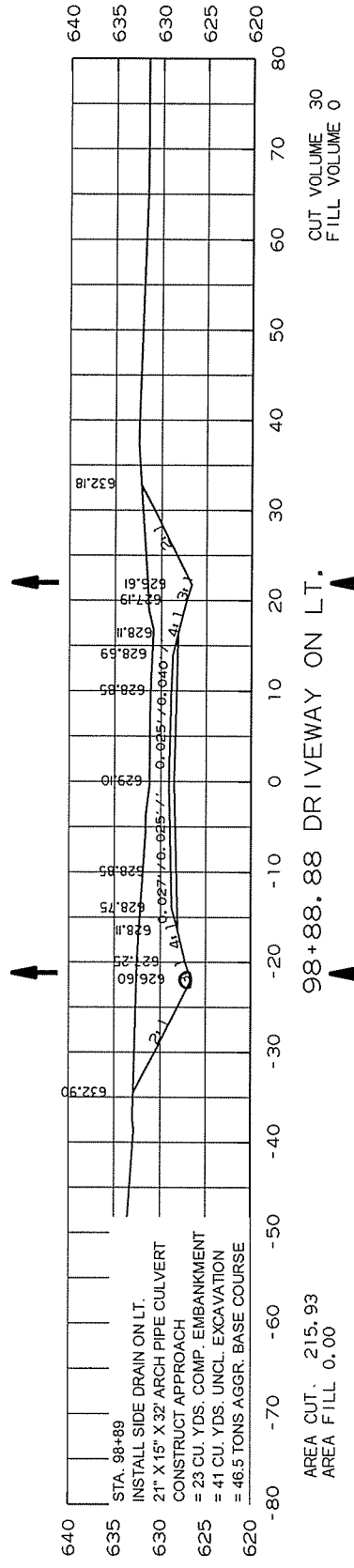
ARKANSAS STATE HIGHWAY COMMISSION

**WIRE FENCE
 TYPE C AND D**

STANDARD DRAWING WF-4

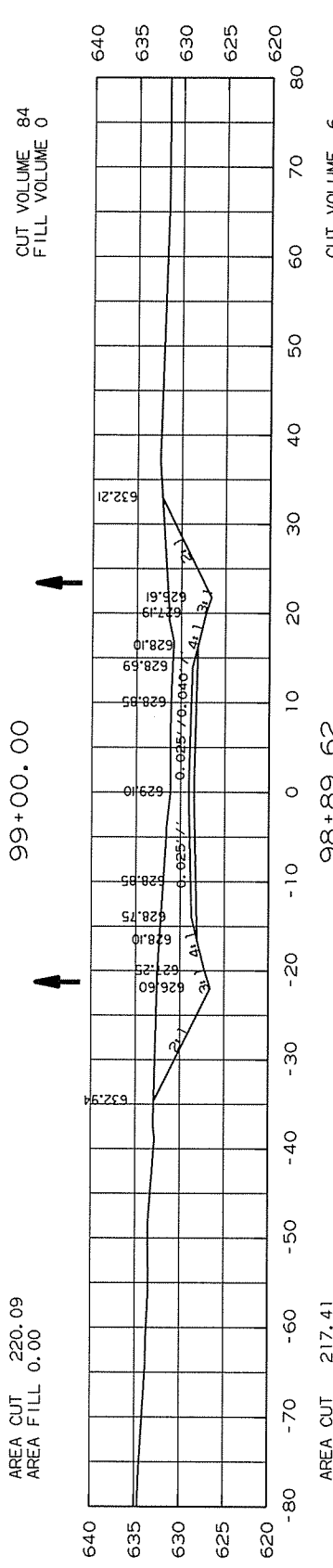
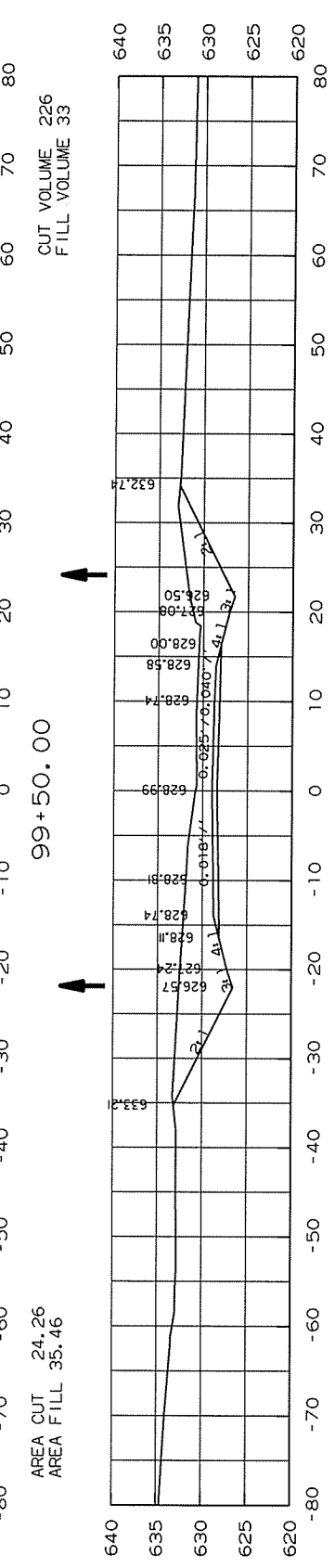
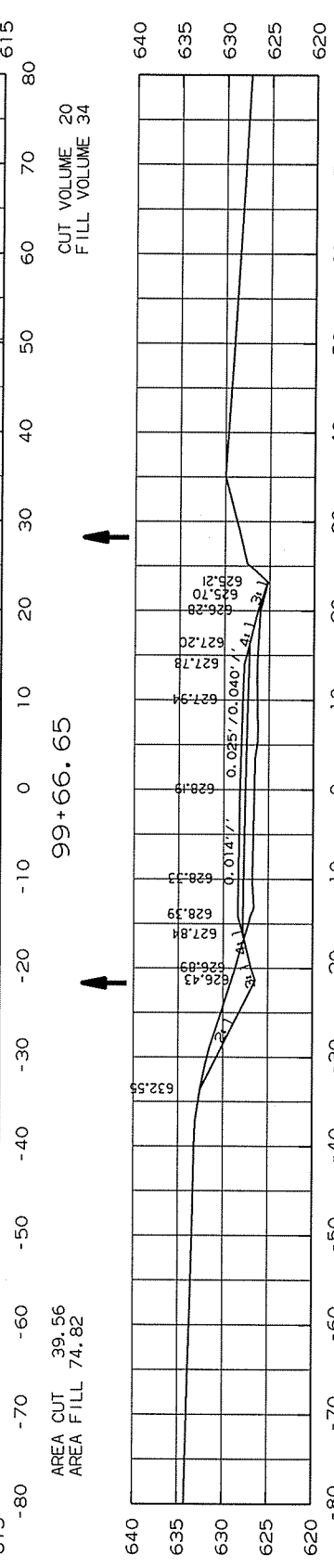
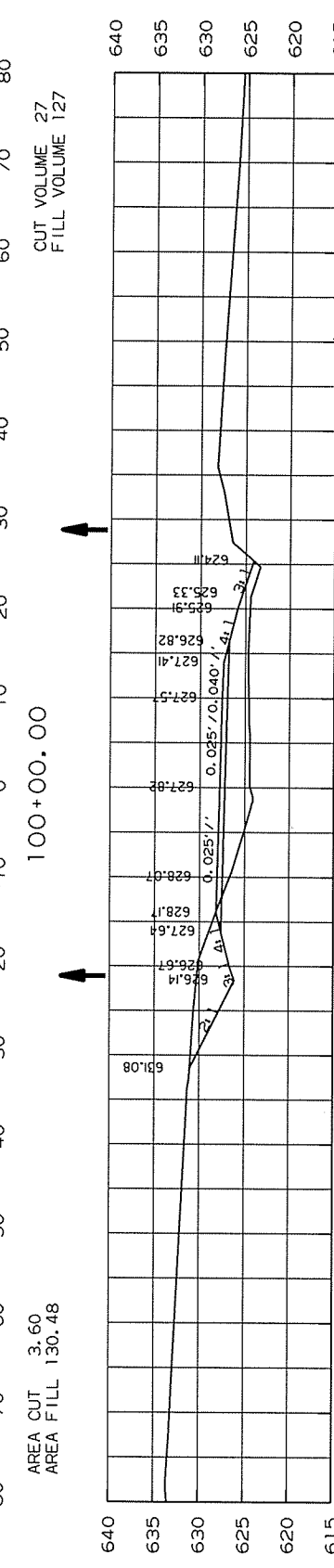
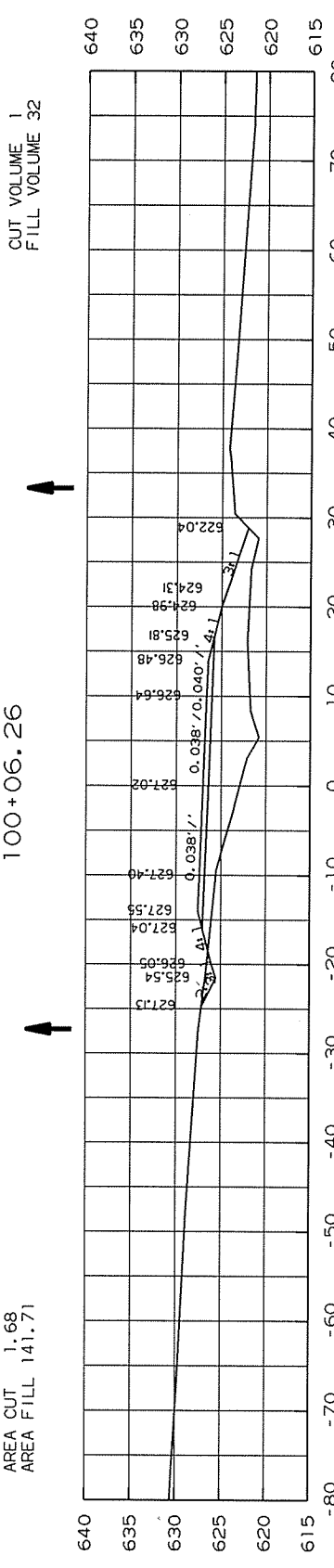
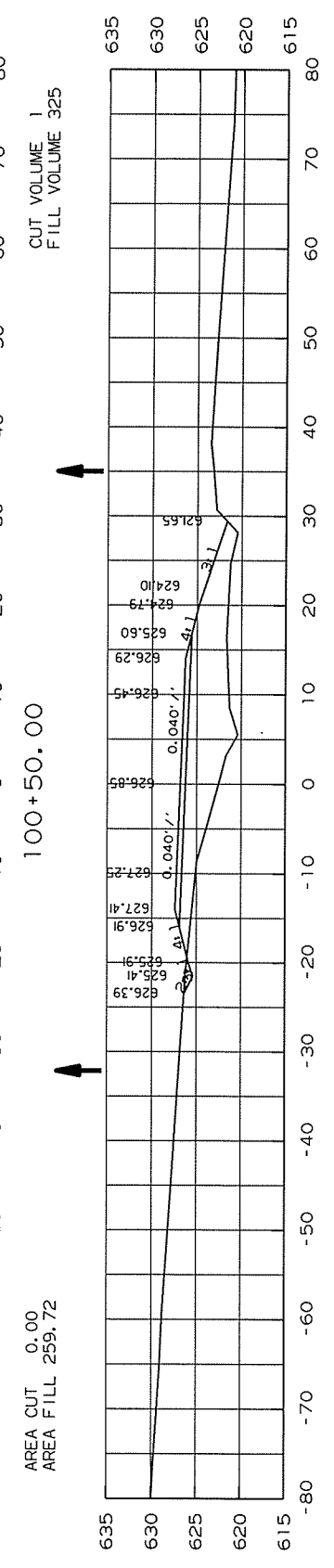
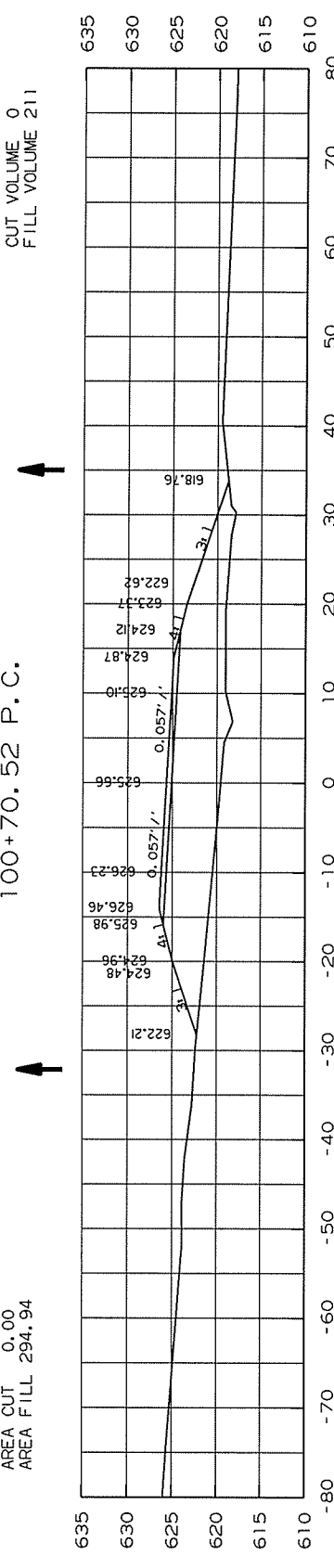
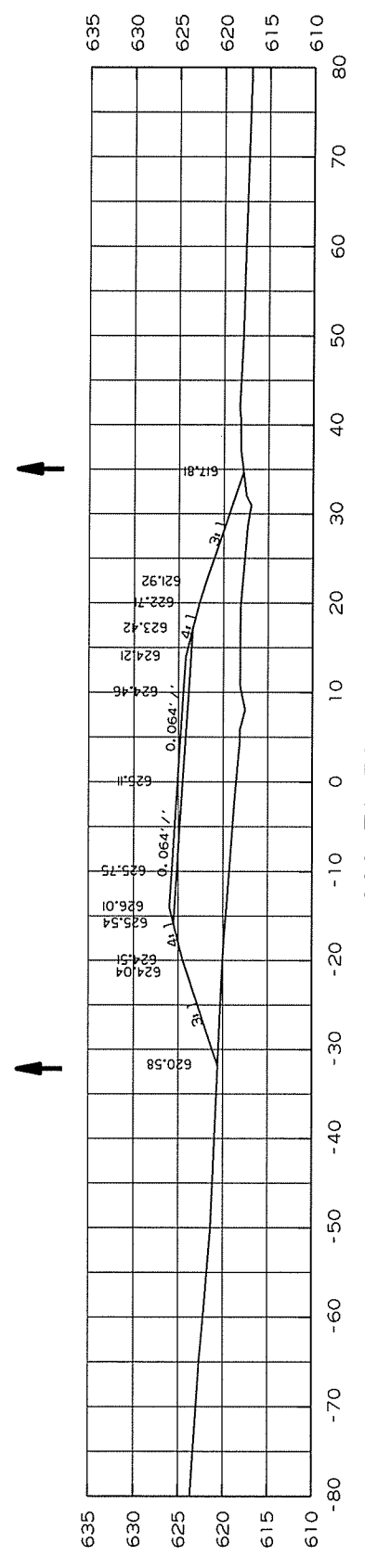
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	62	85	

4 CROSS SECTIONS STA. 97+00.00 TO 98+88.88



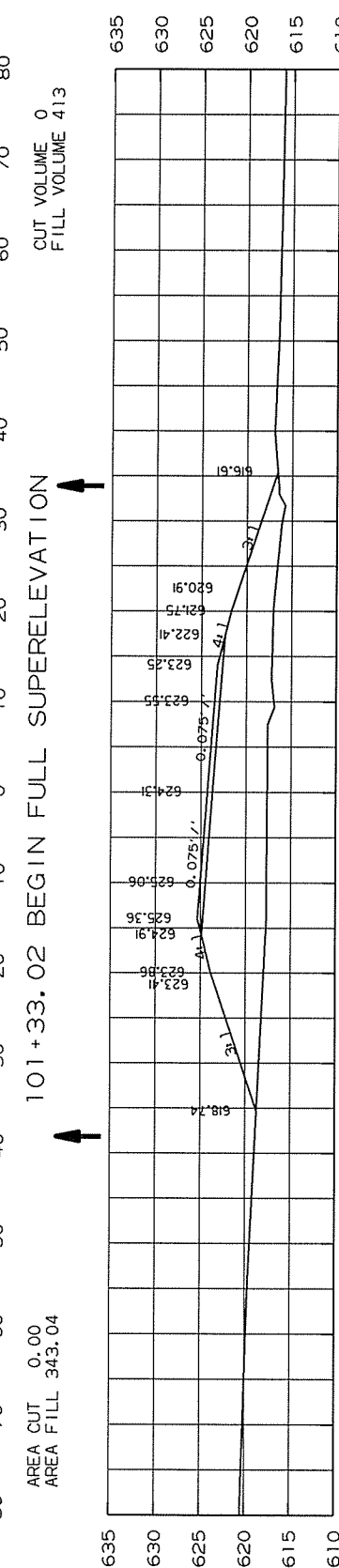
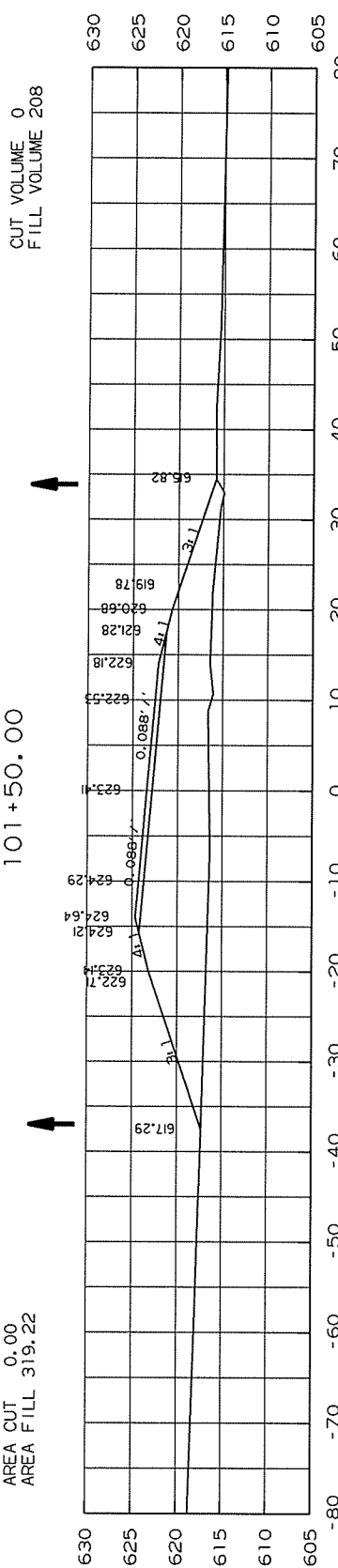
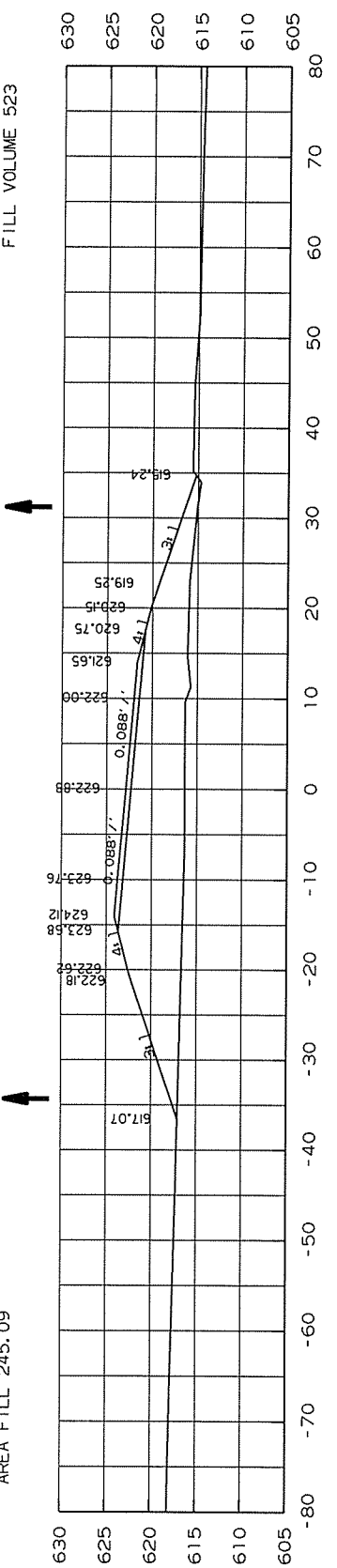
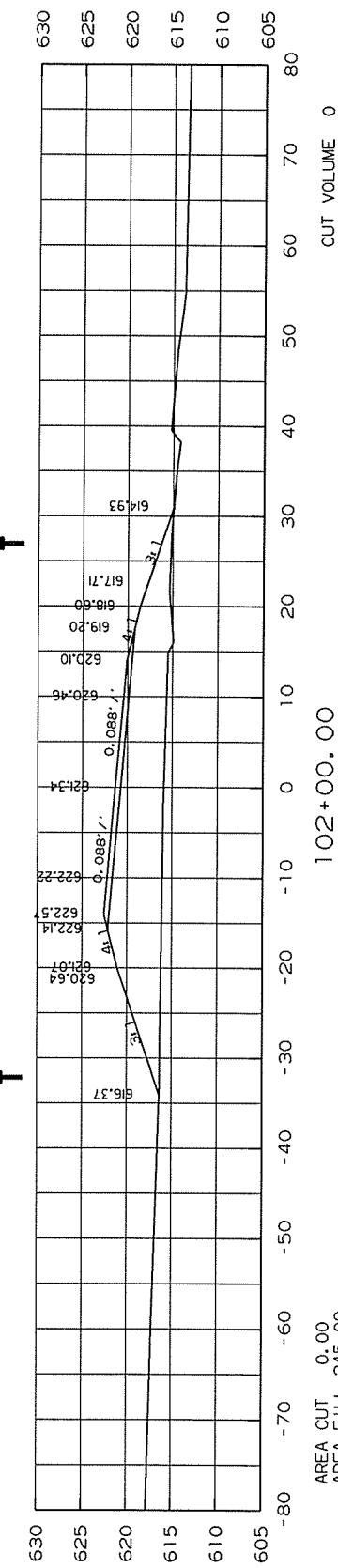
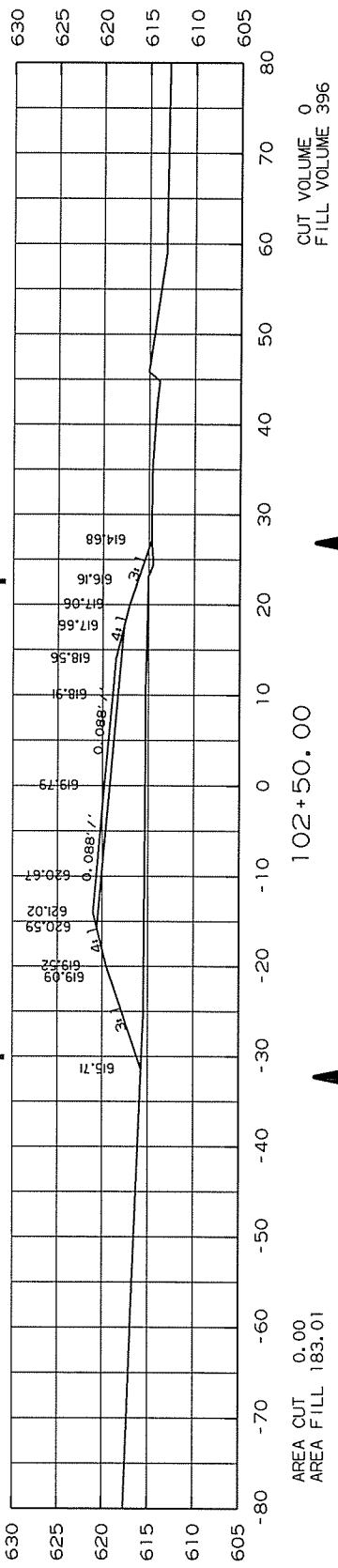
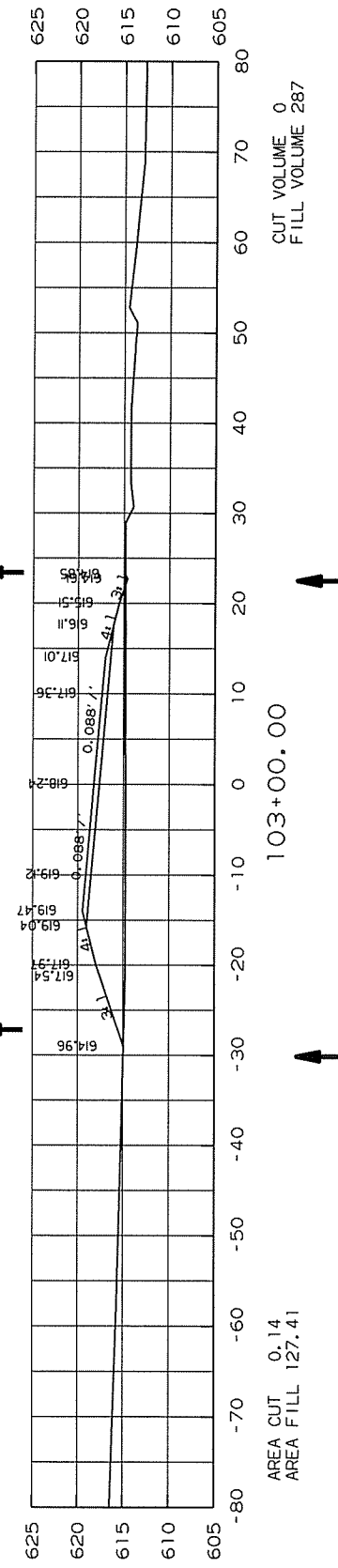
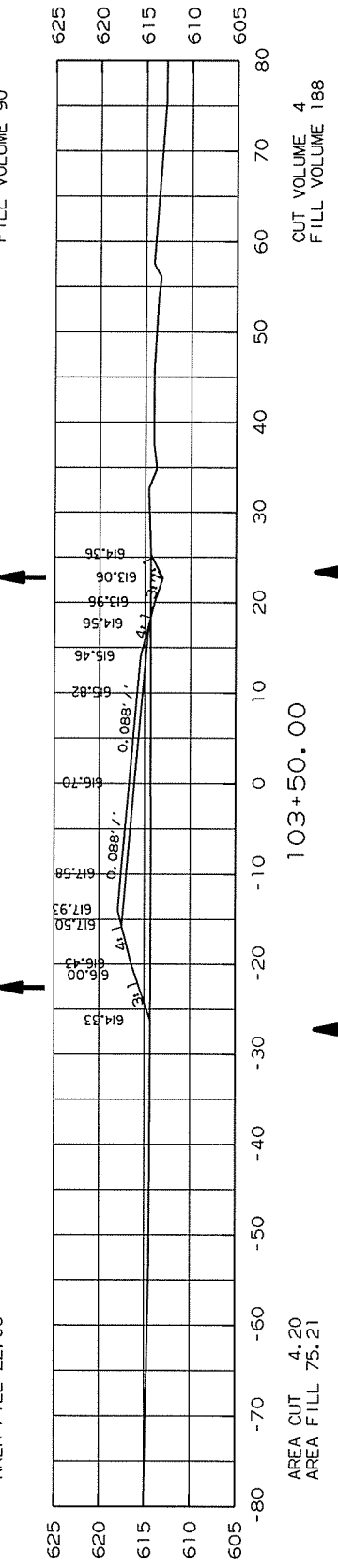
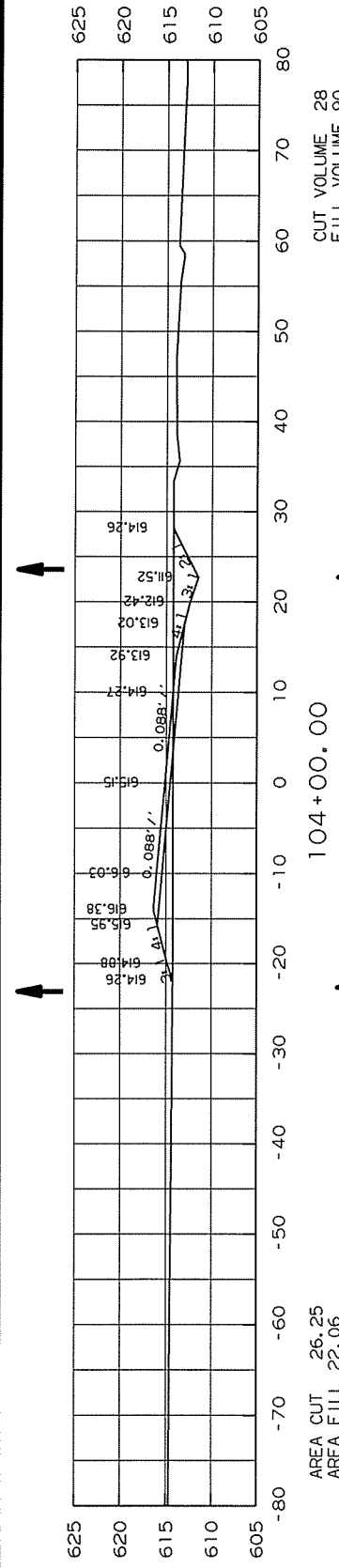
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	63	85	

4 CROSS SECTIONS STA. 98+89.62 TO 100+70.52



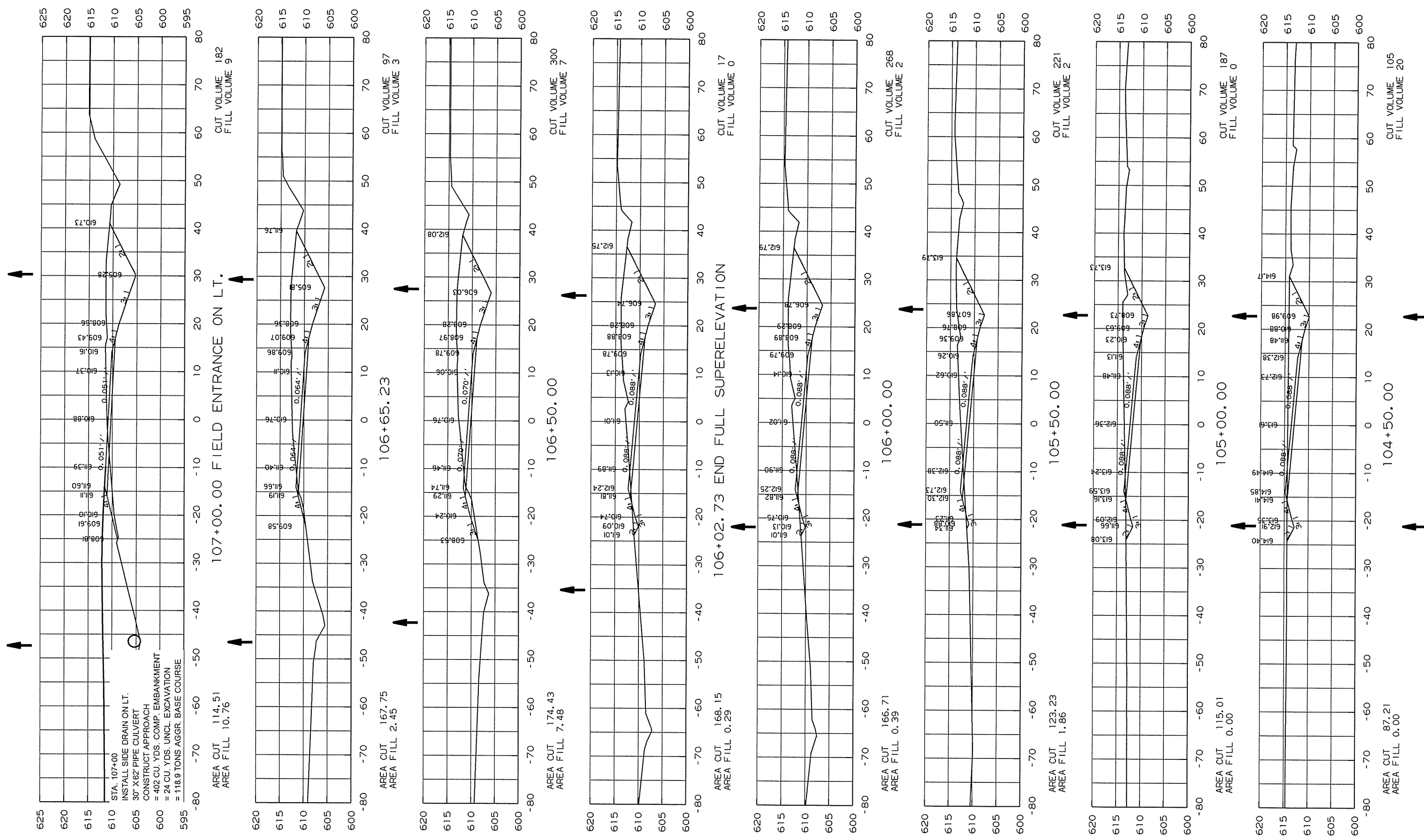
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		64	85

4 CROSS SECTIONS STA. 101+00.00 TO 104+00.00



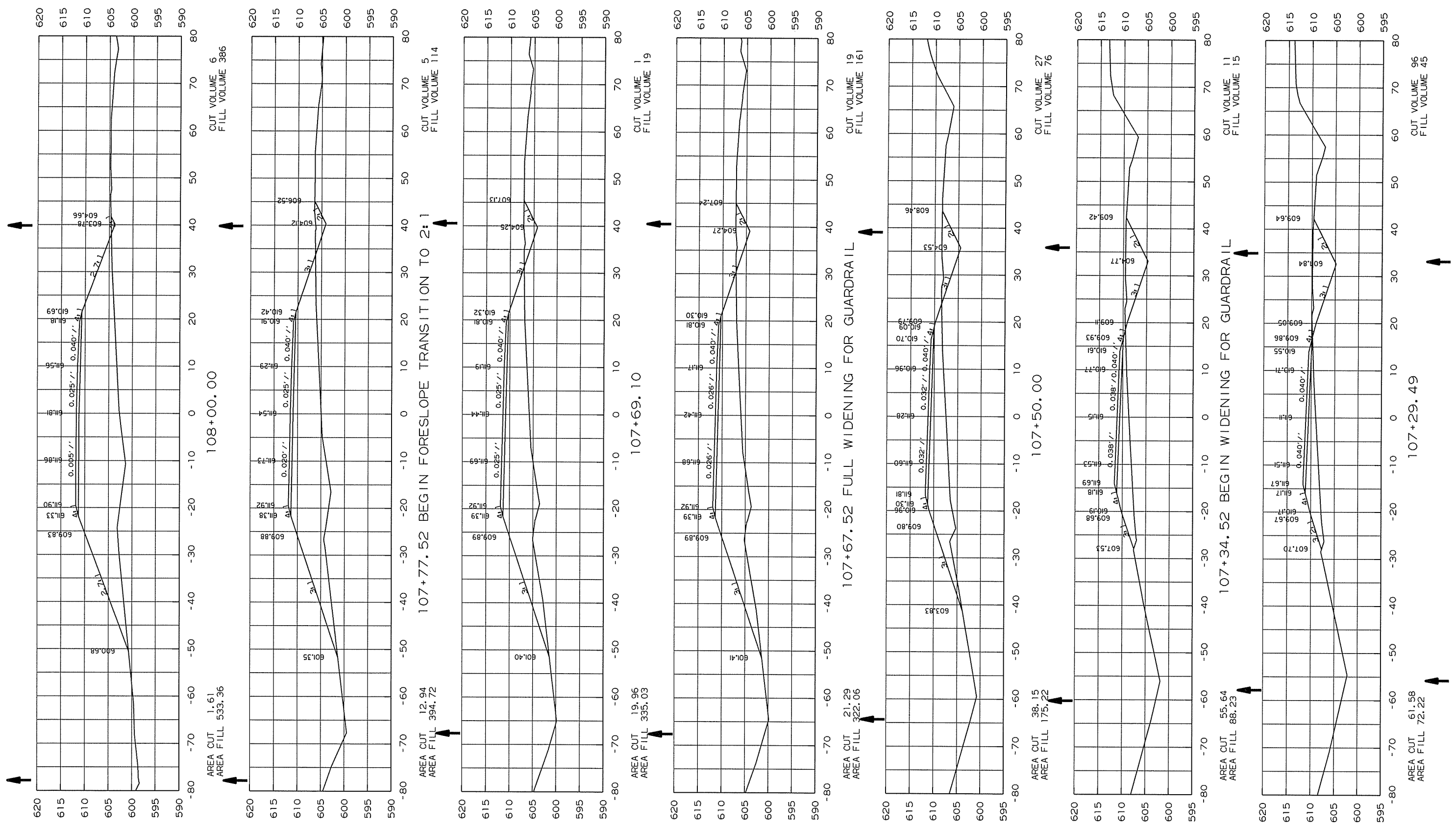
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		65	85

4 CROSS SECTIONS STA. 104+50.00 TO 107+00.00



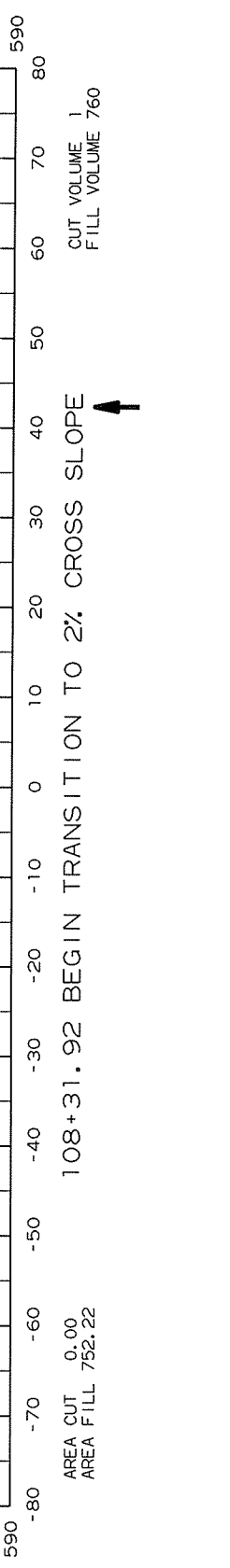
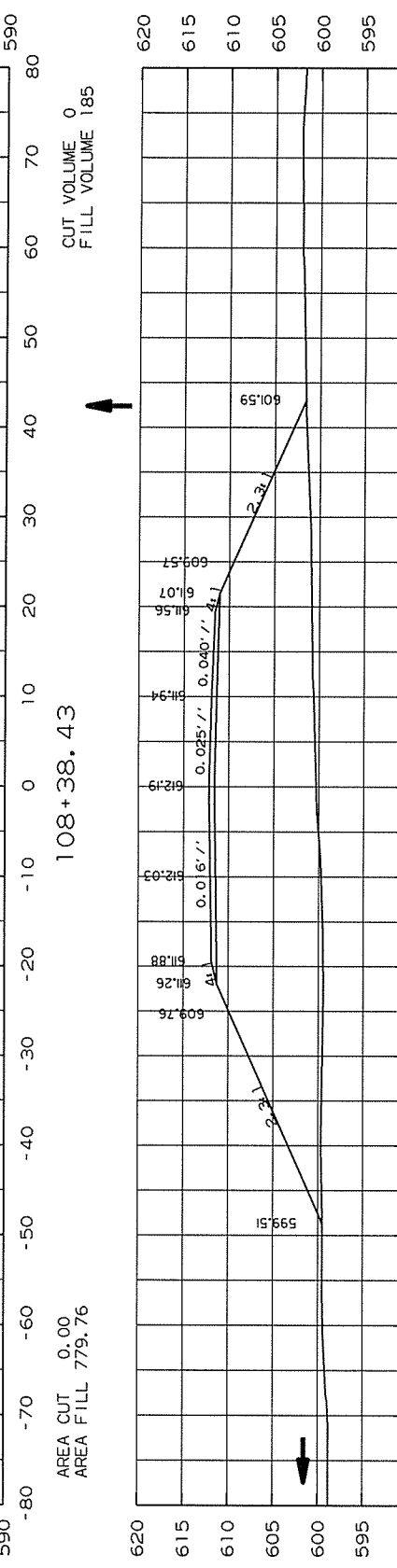
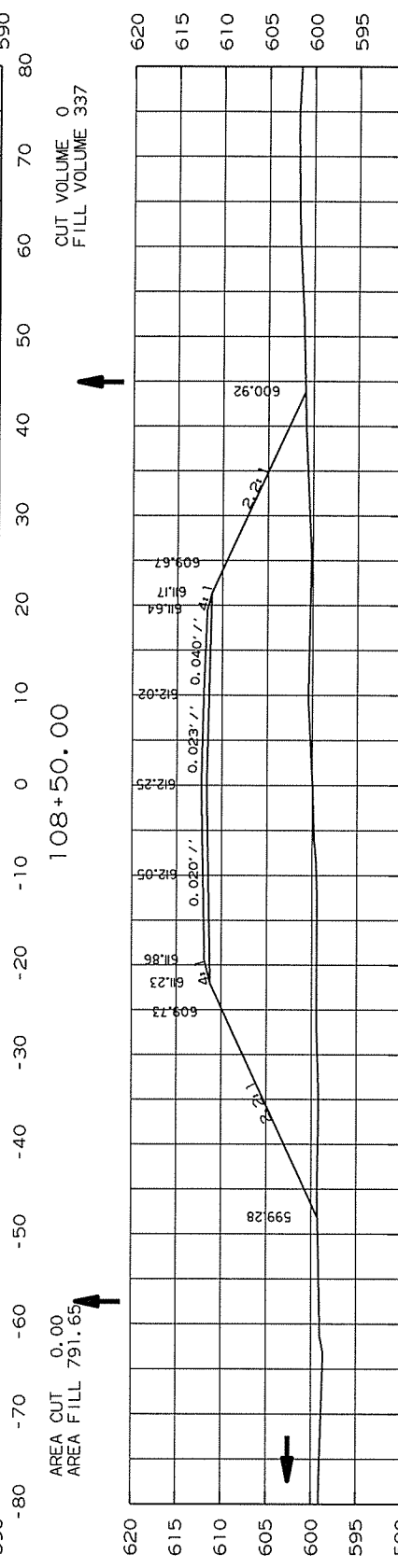
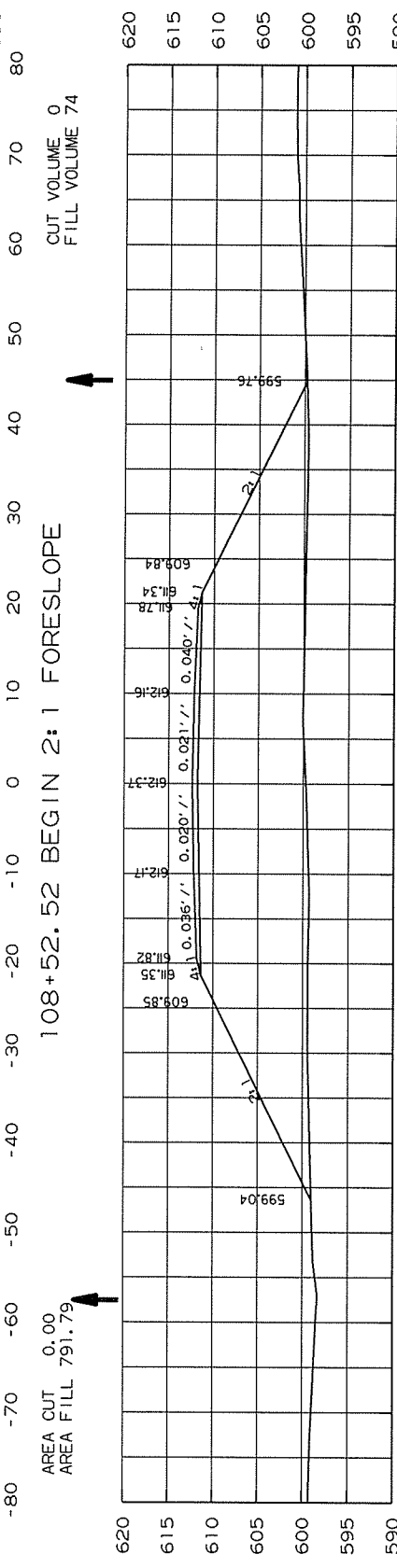
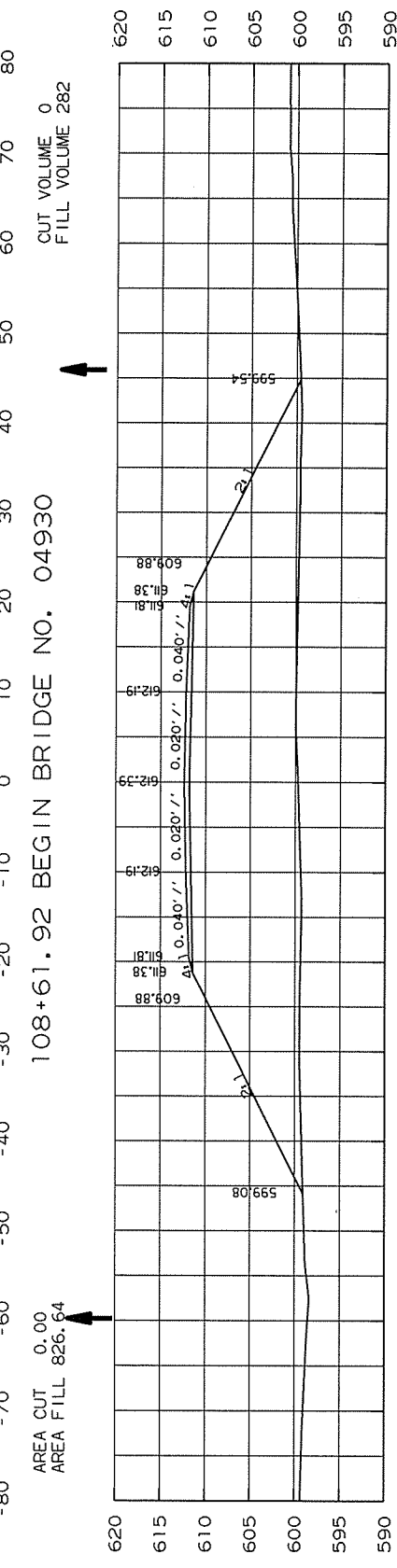
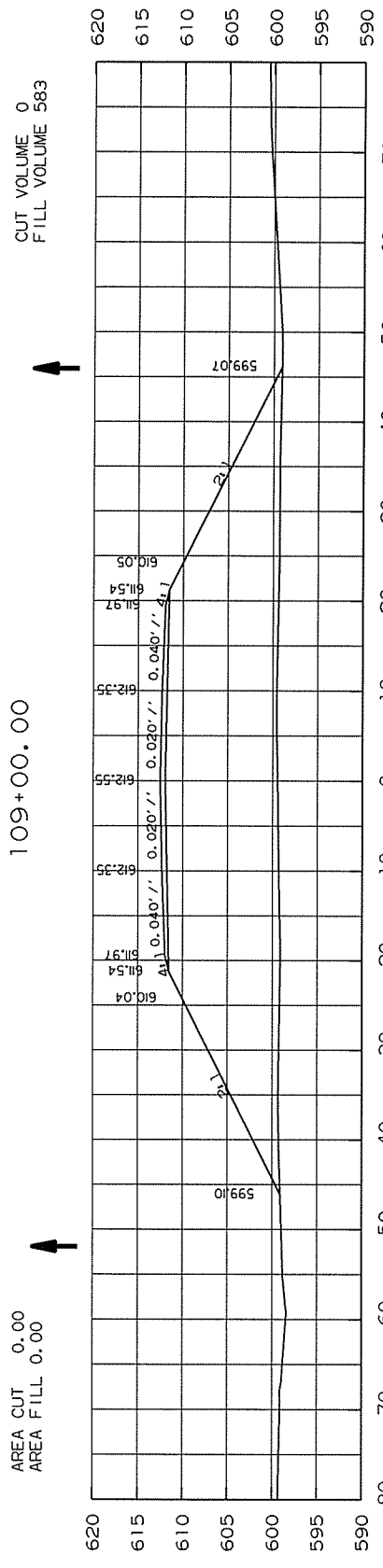
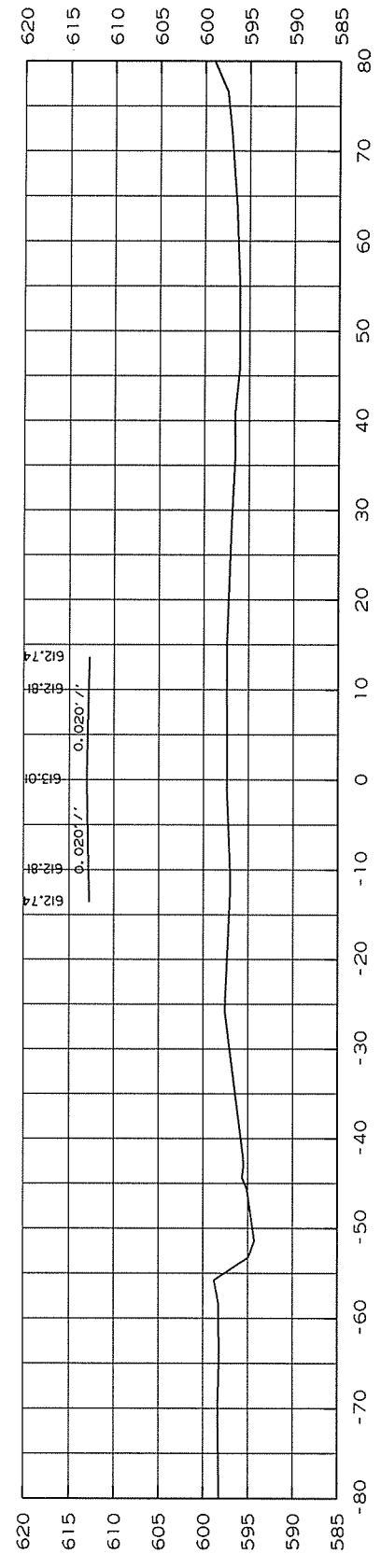
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		66	85

4 CROSS SECTIONS STA. 107+29.49 TO 108+00.00



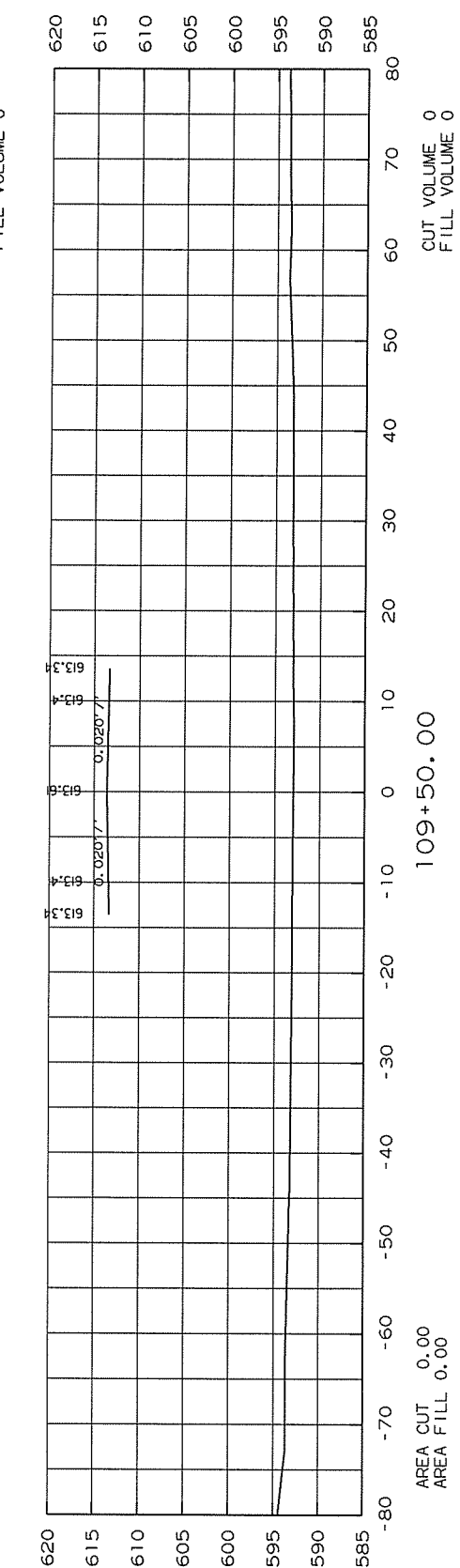
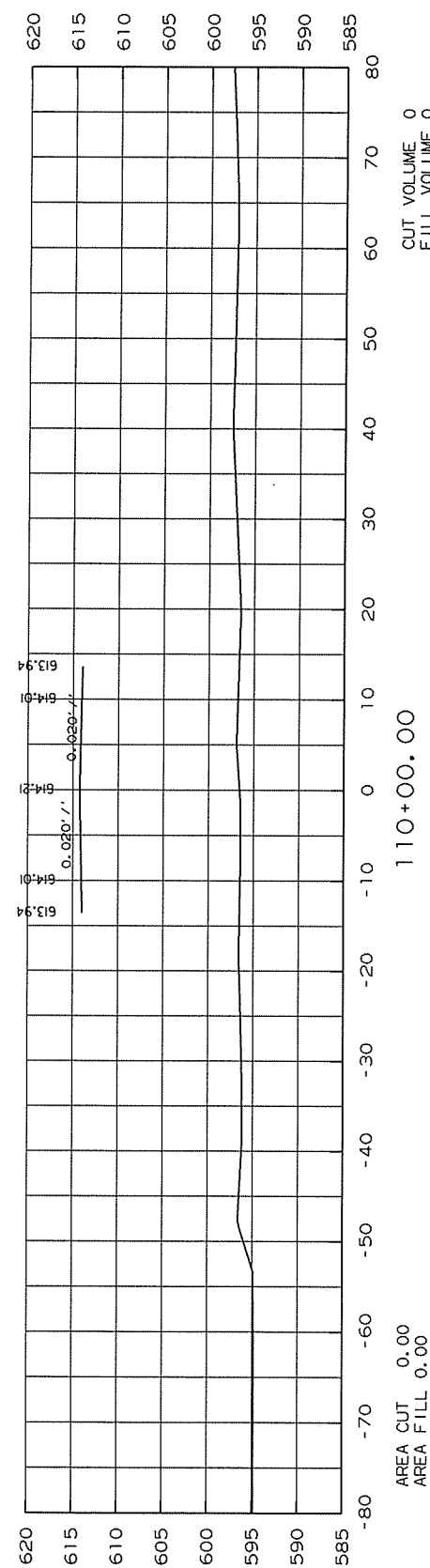
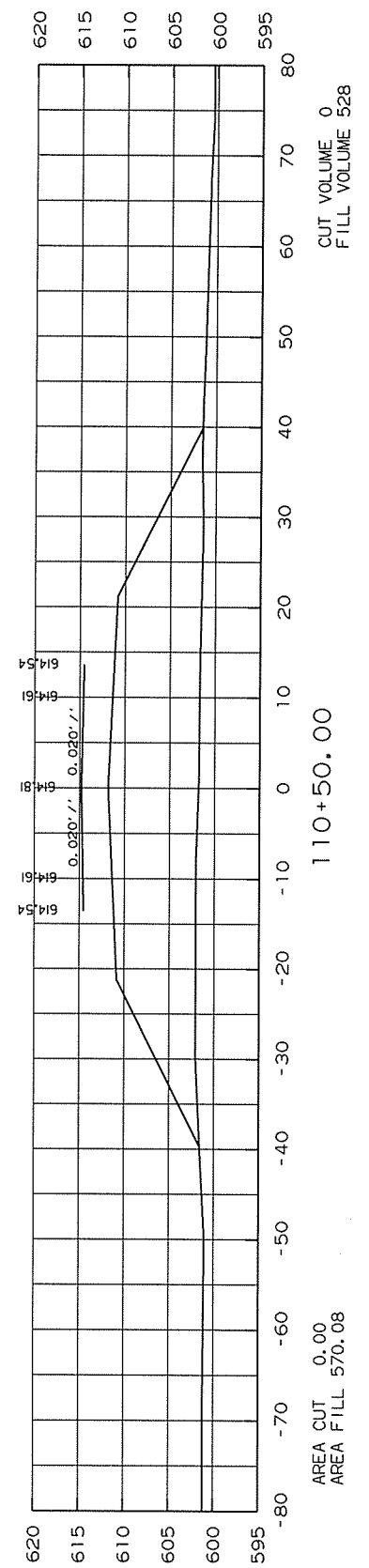
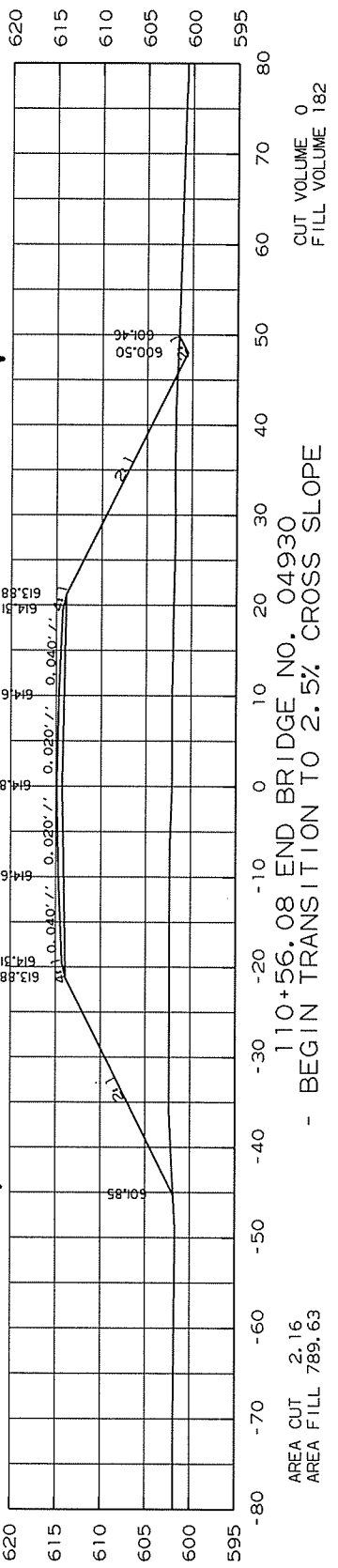
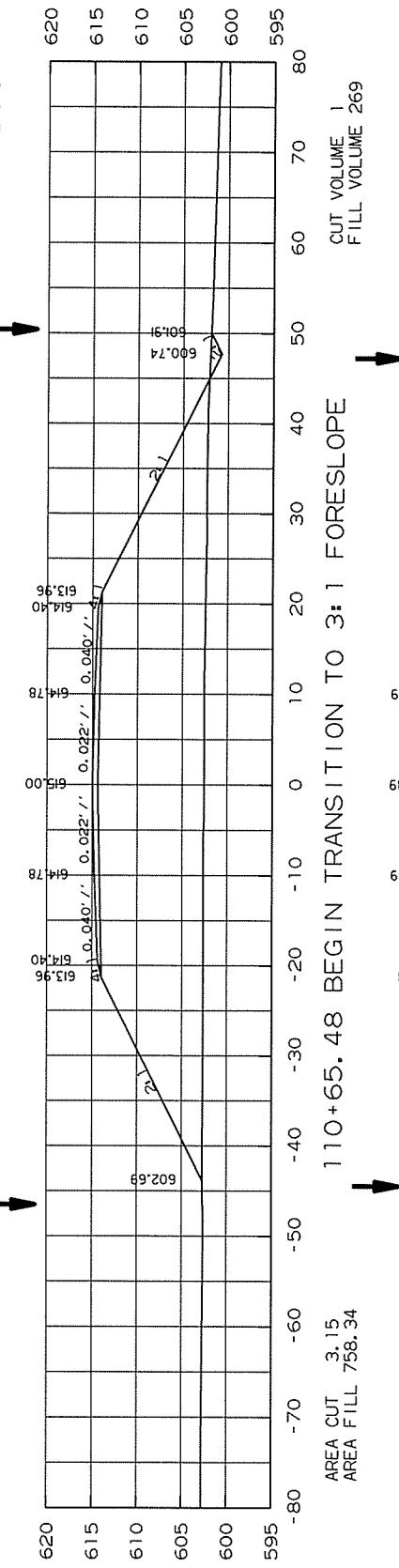
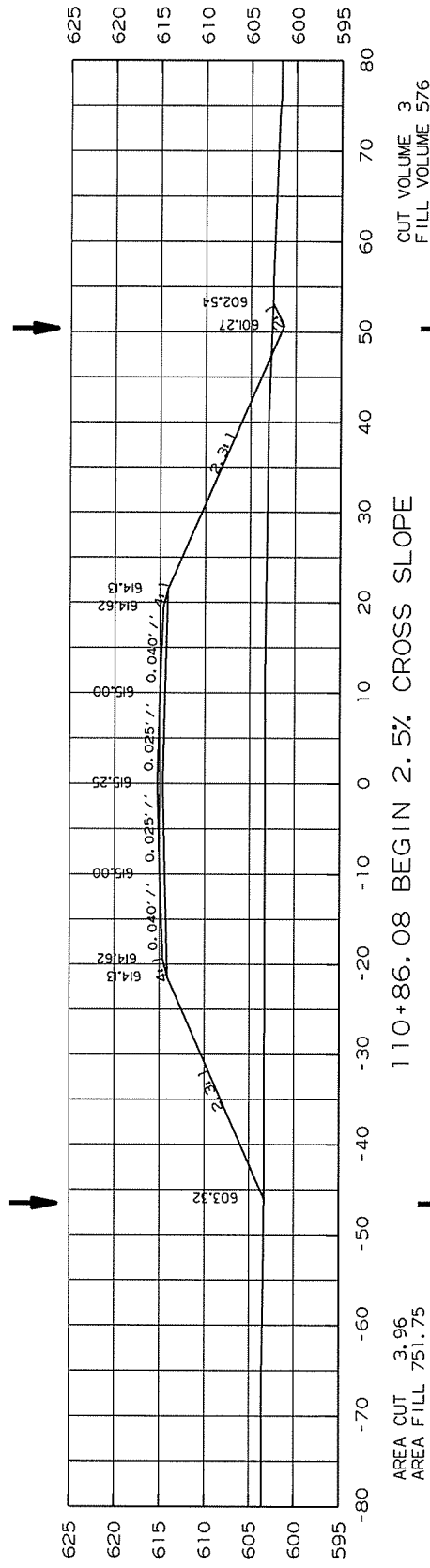
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BR4906	67	85

4 CROSS SECTIONS STA. 108+31.92 TO 109+00.00



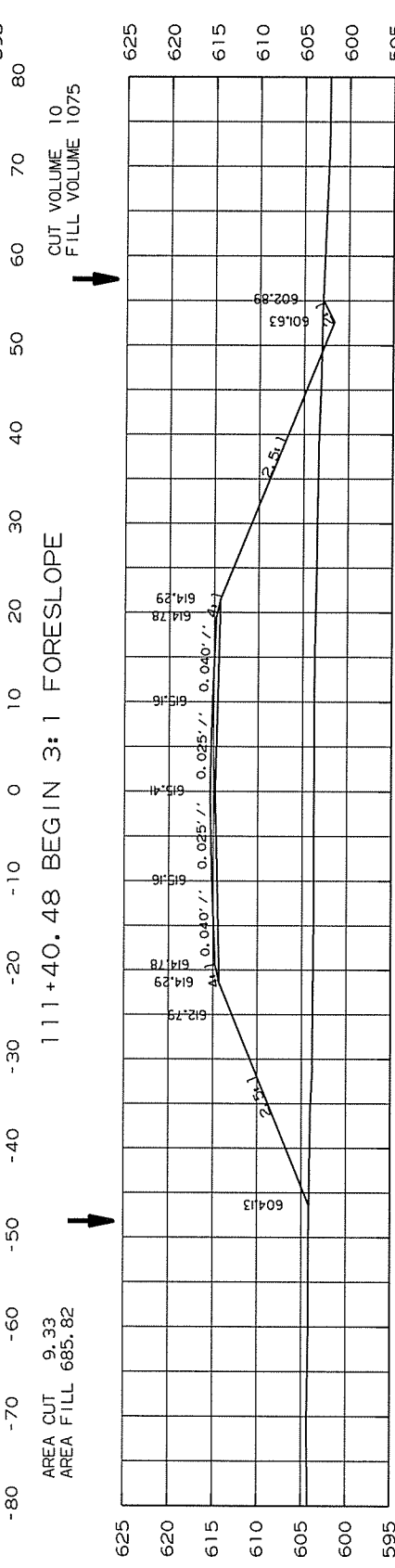
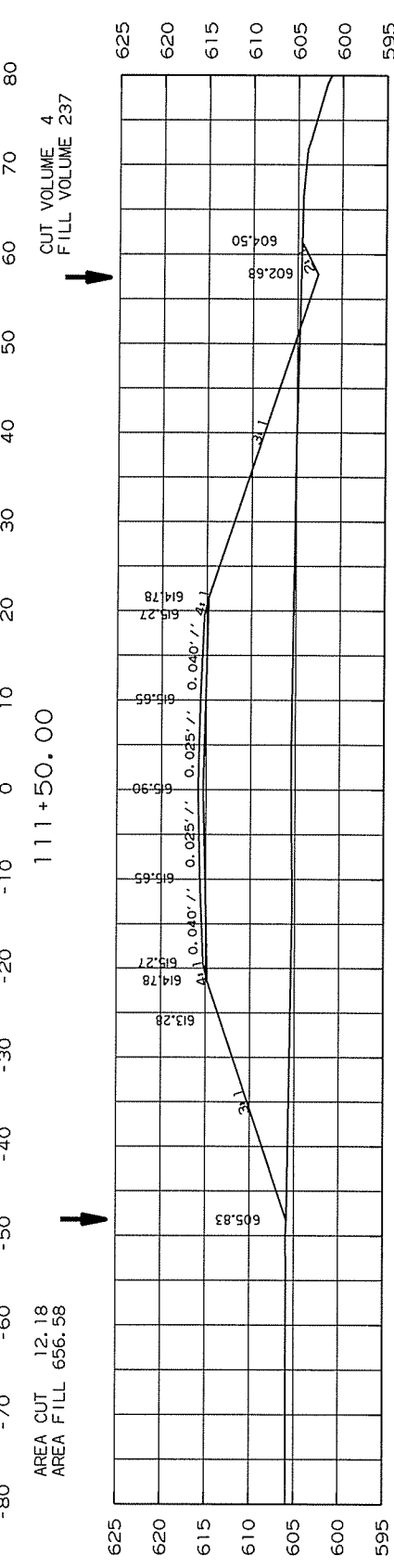
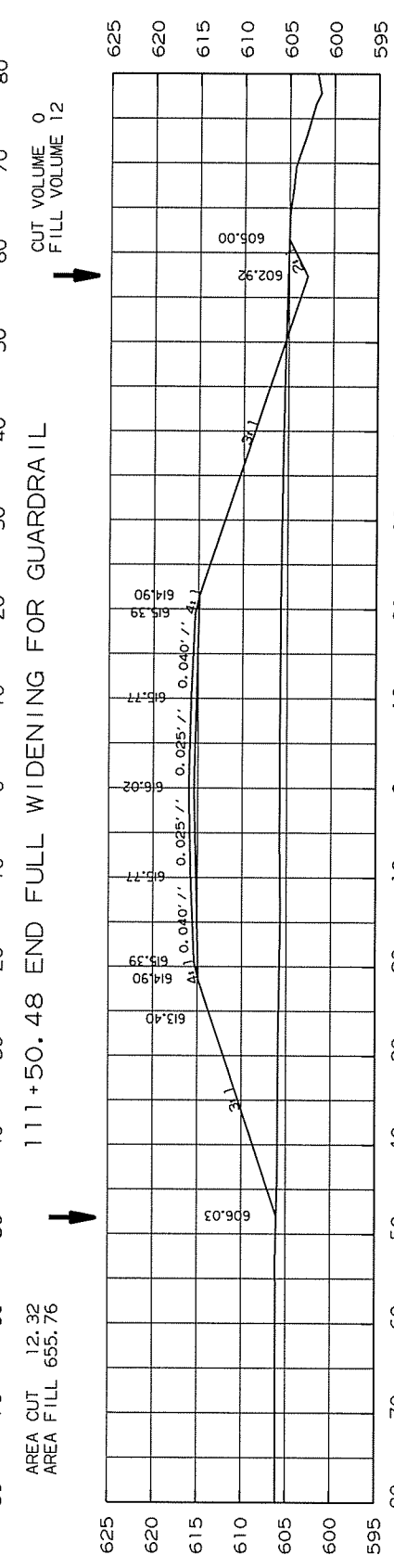
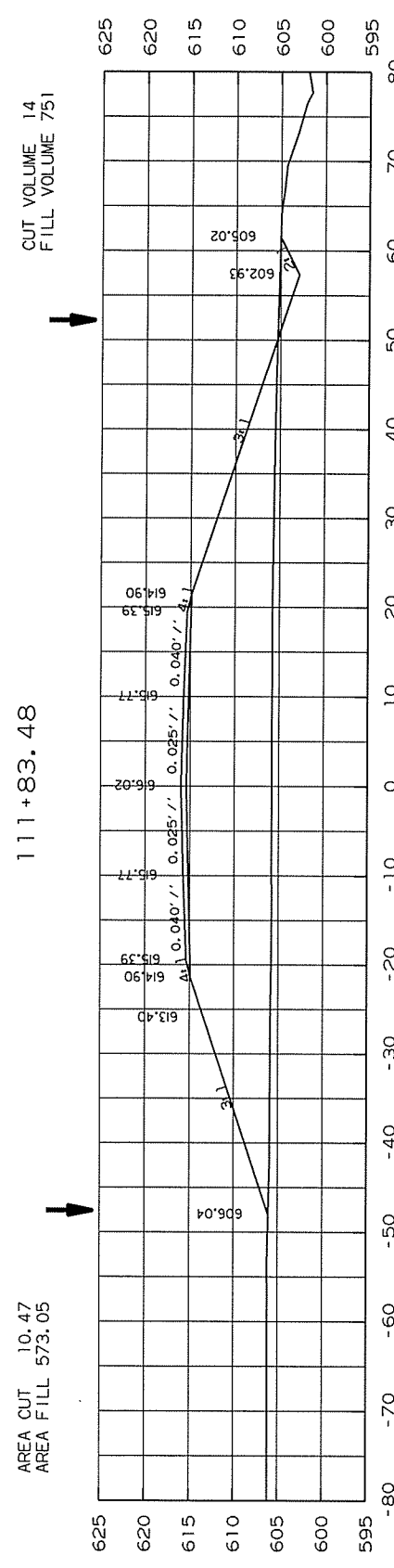
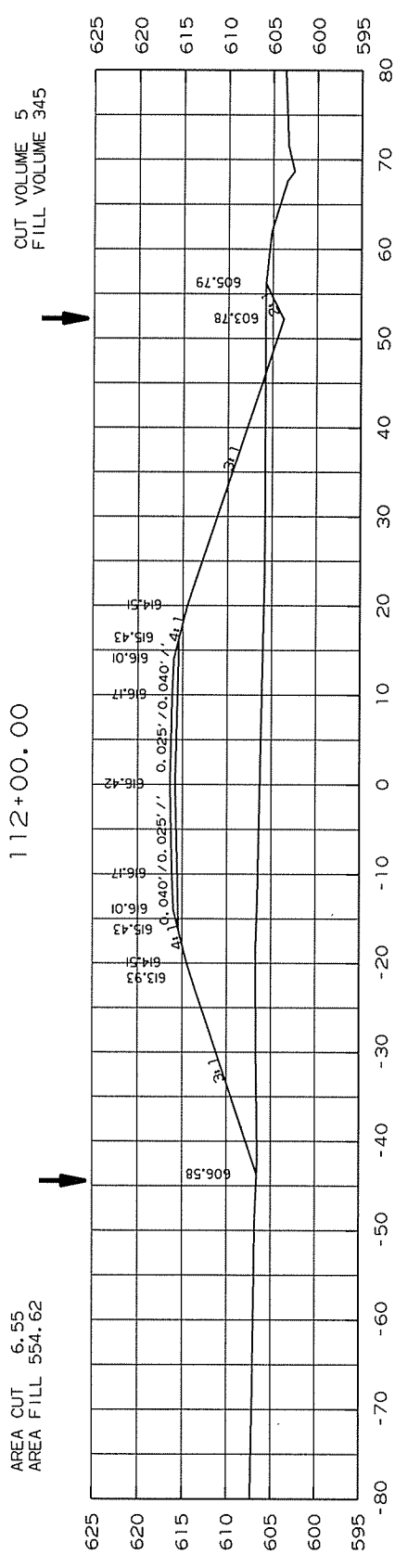
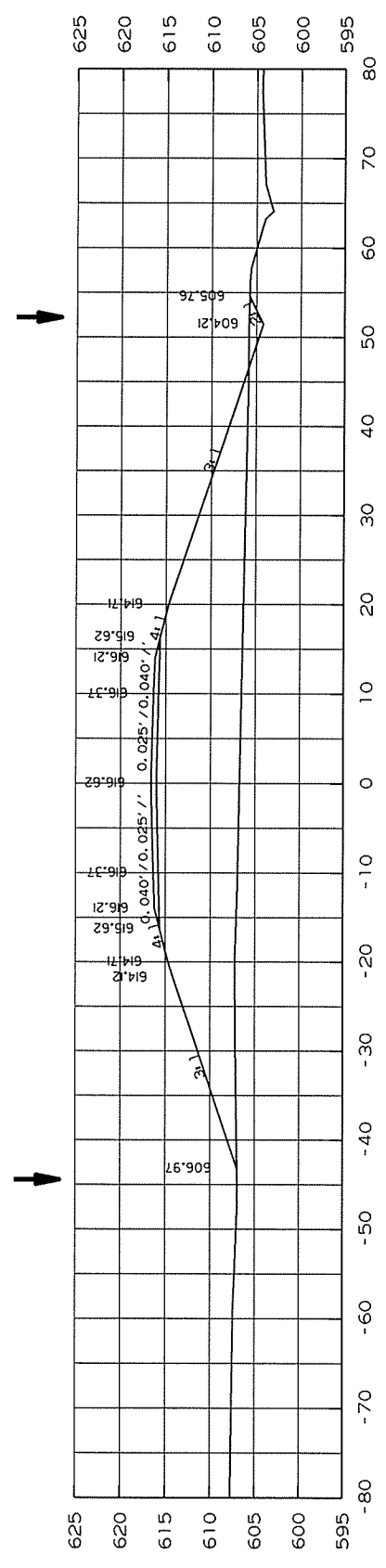
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
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				JOB NO.	BR4906		68	85

4 CROSS SECTIONS STA. 109+50.00 TO 110+86.08



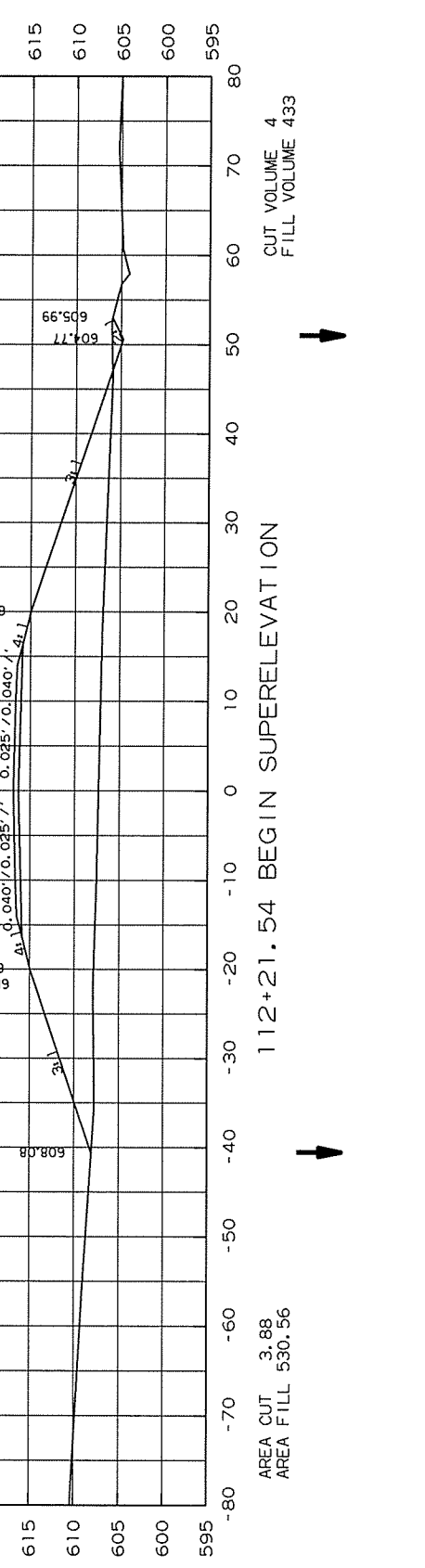
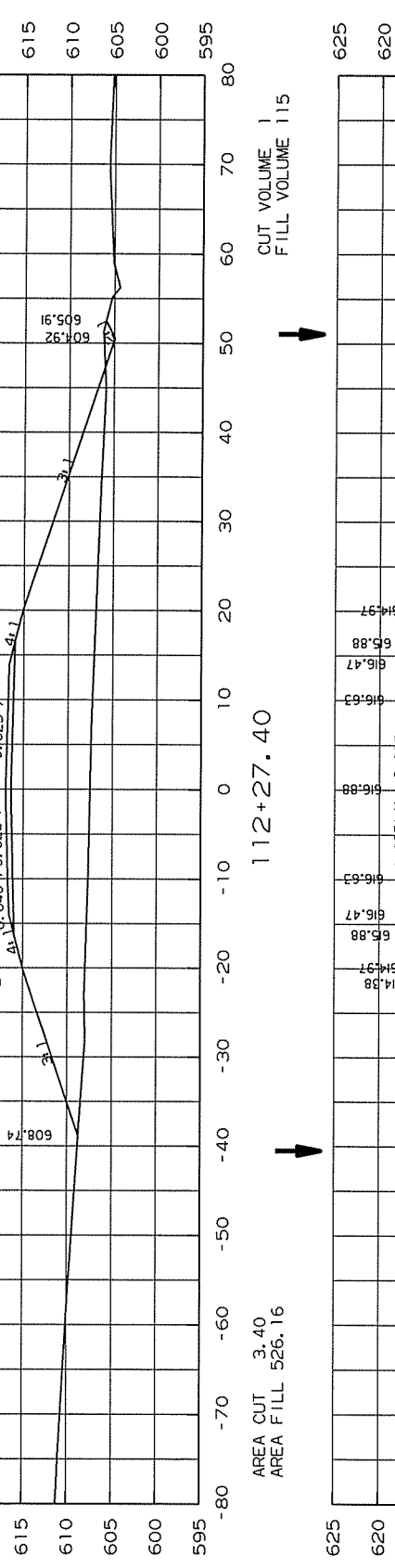
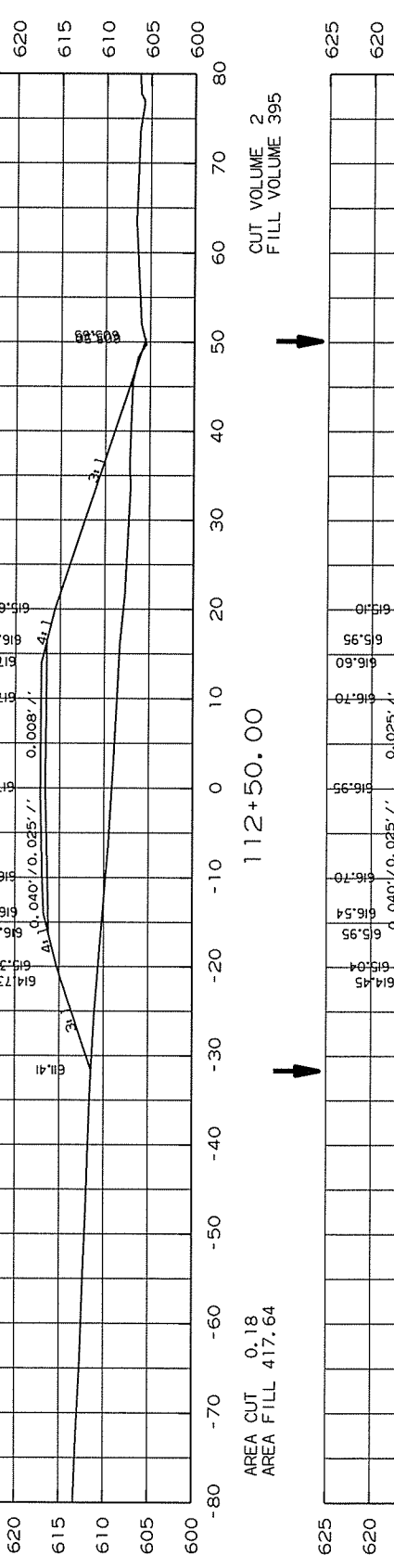
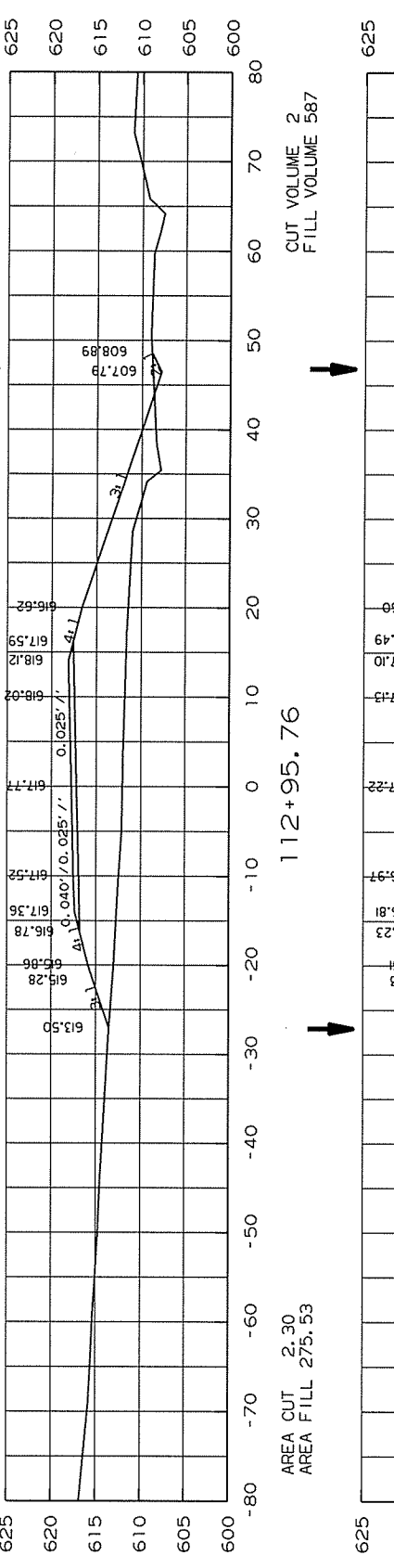
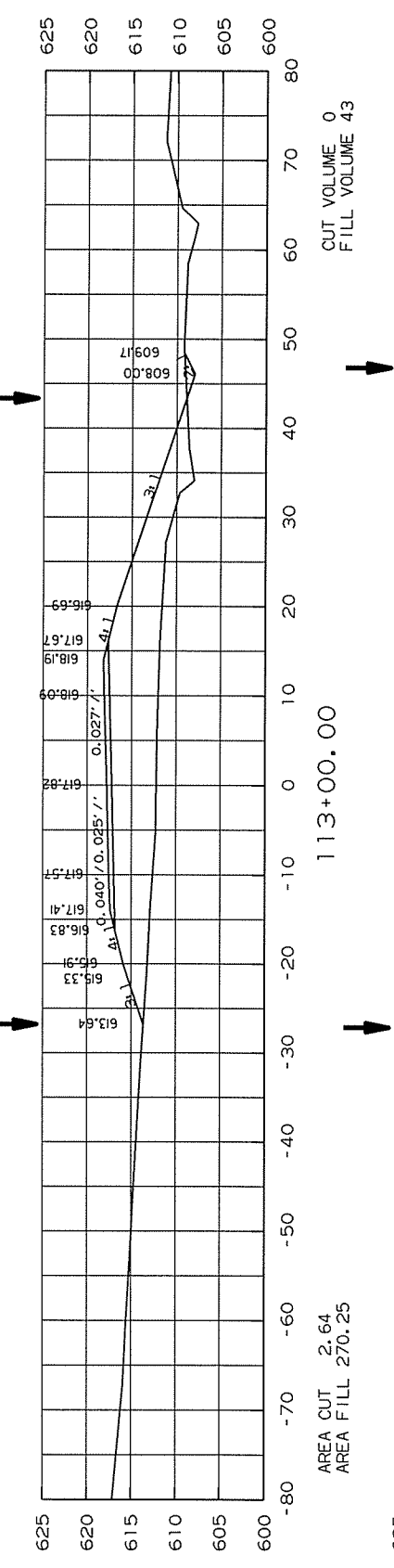
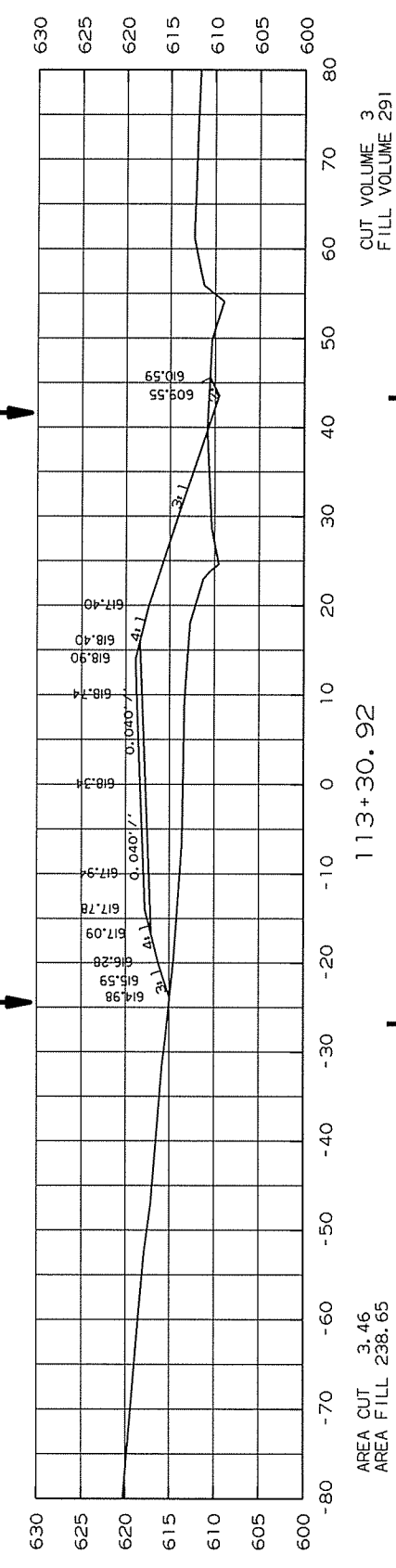
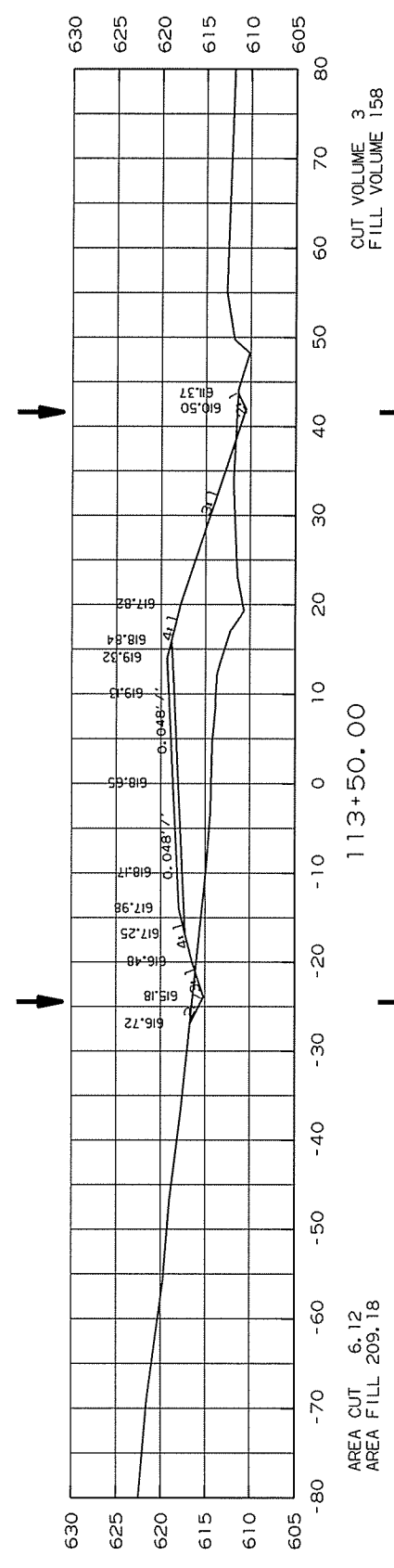
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	69	85	

4 CROSS SECTIONS STA. 111+00.00 TO 112+00.00



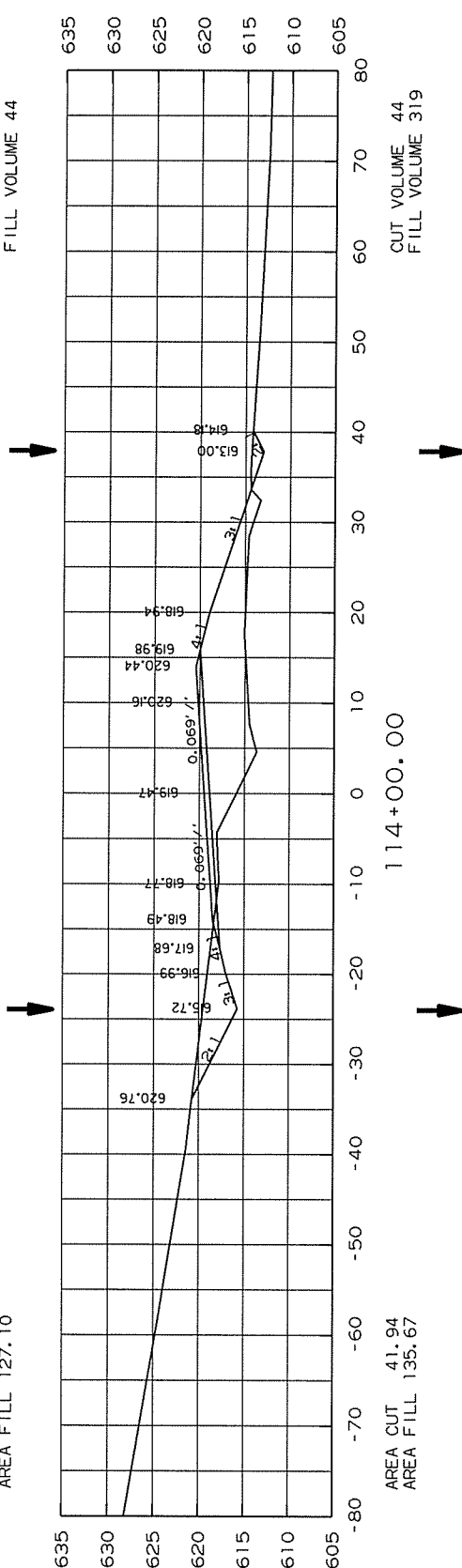
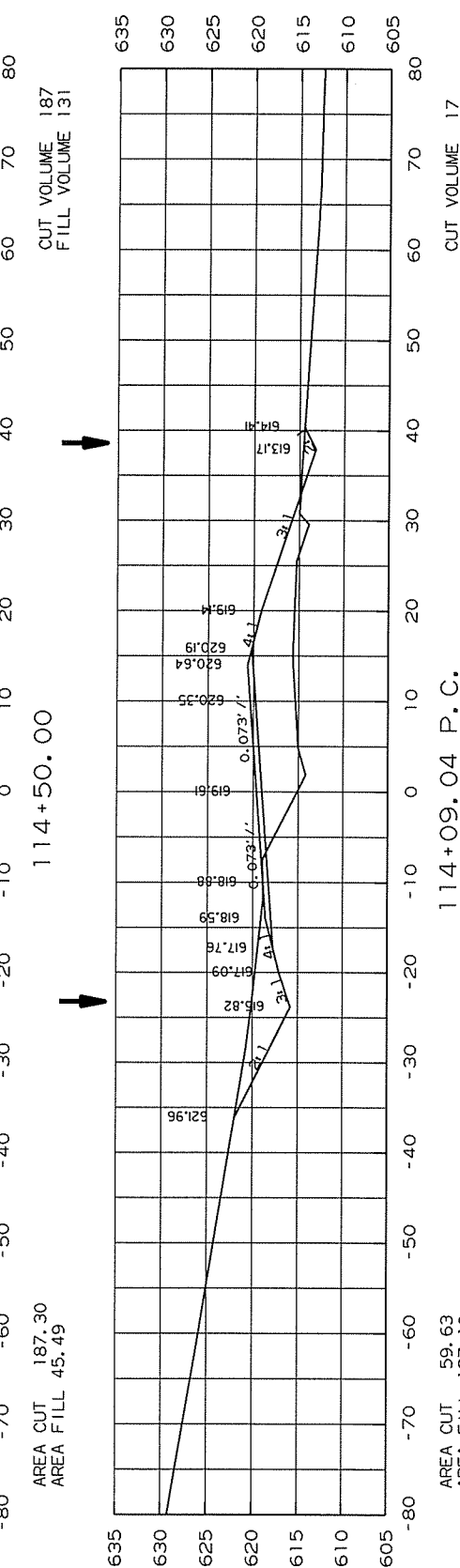
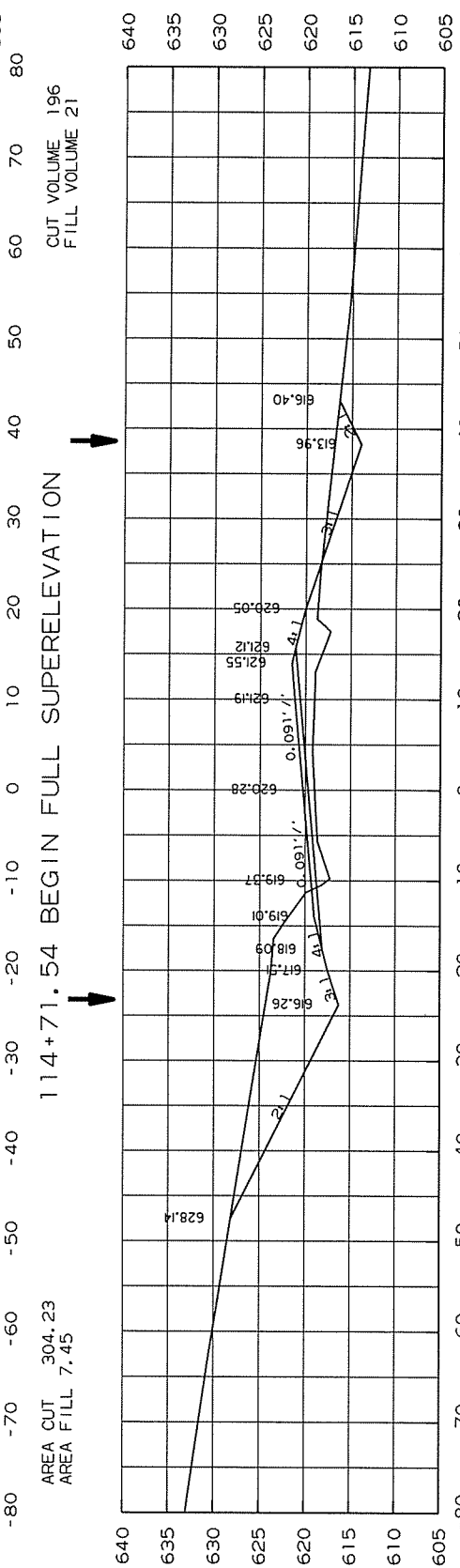
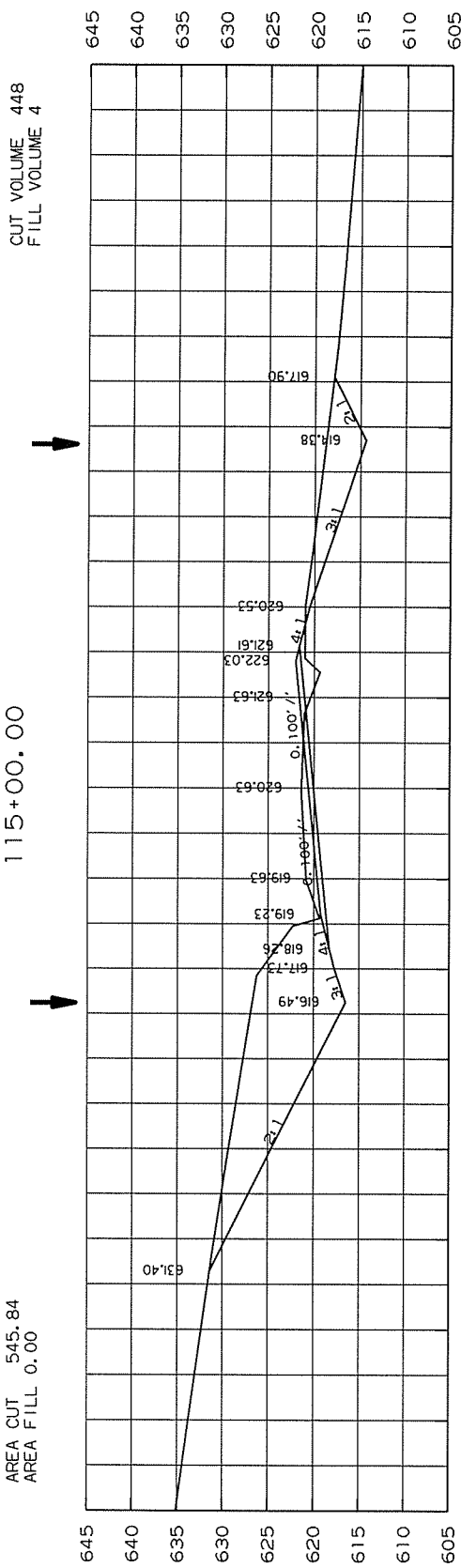
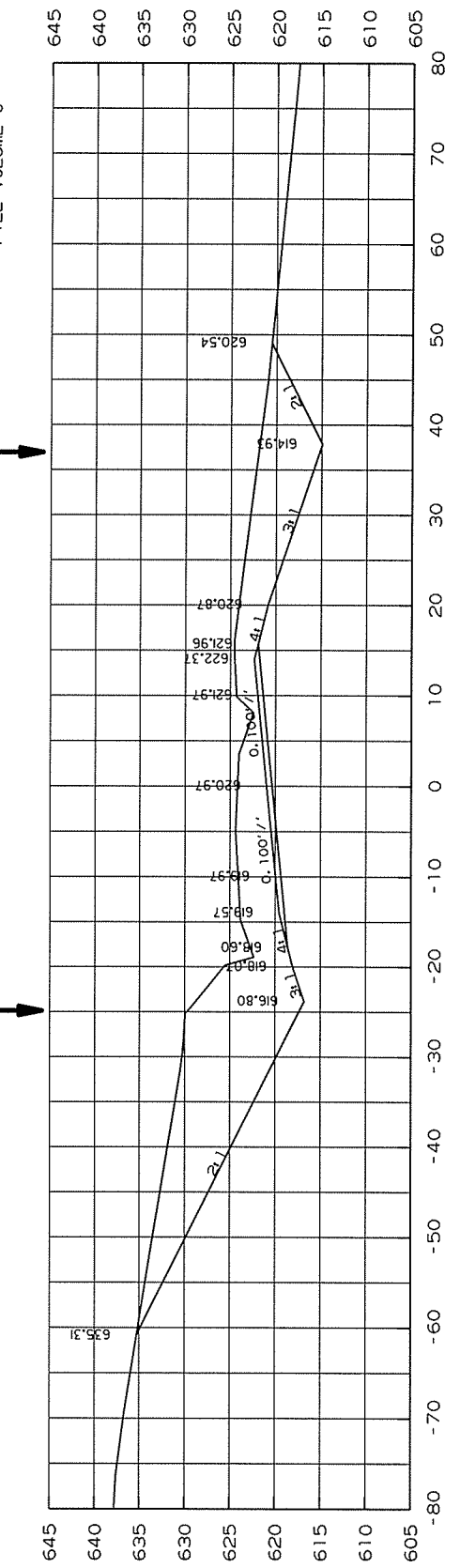
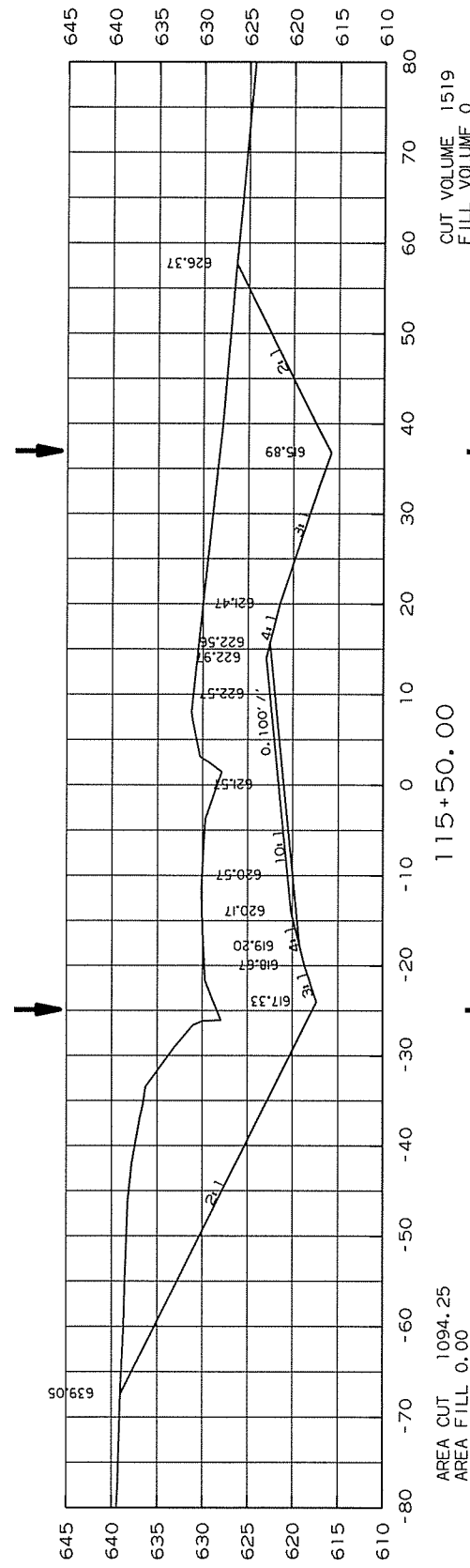
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		70	85

4 CROSS SECTIONS STA. 112+21.54 TO 113+50.00



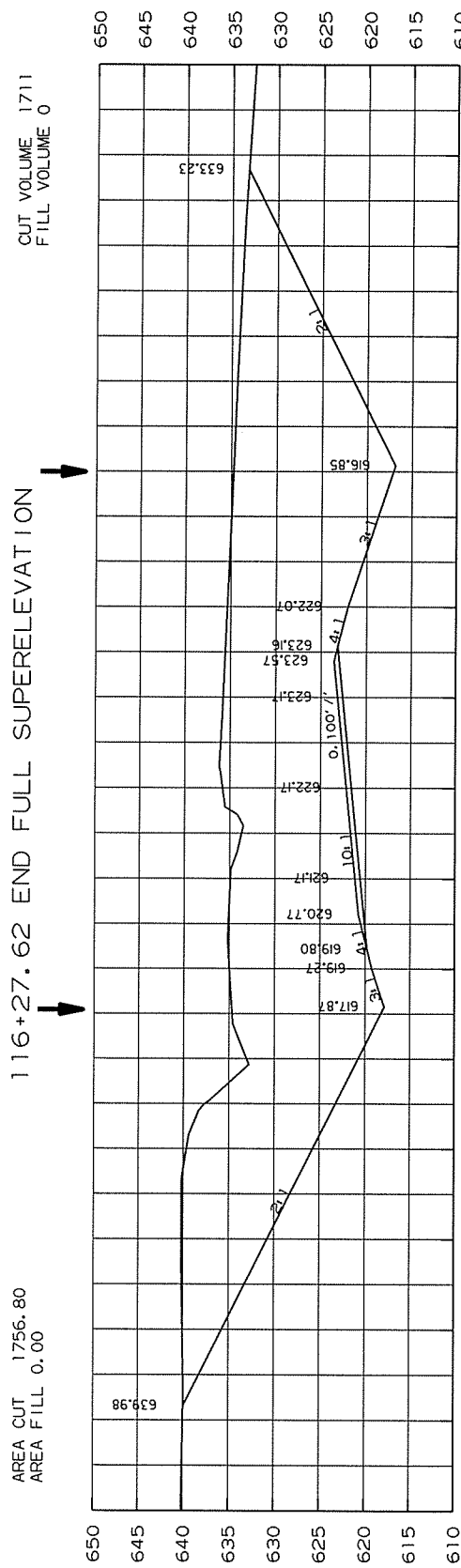
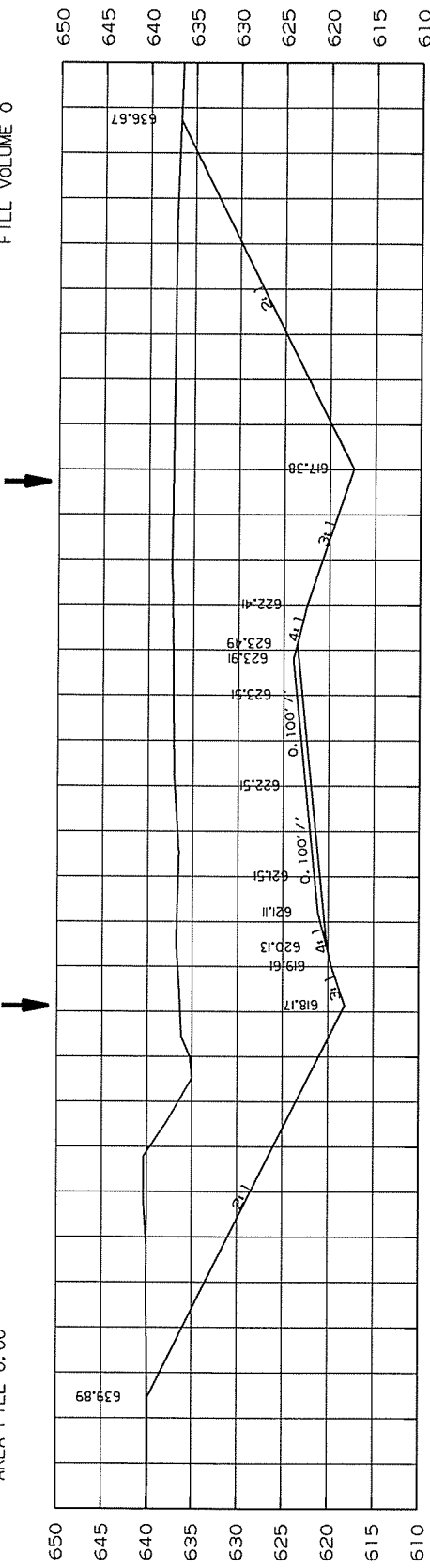
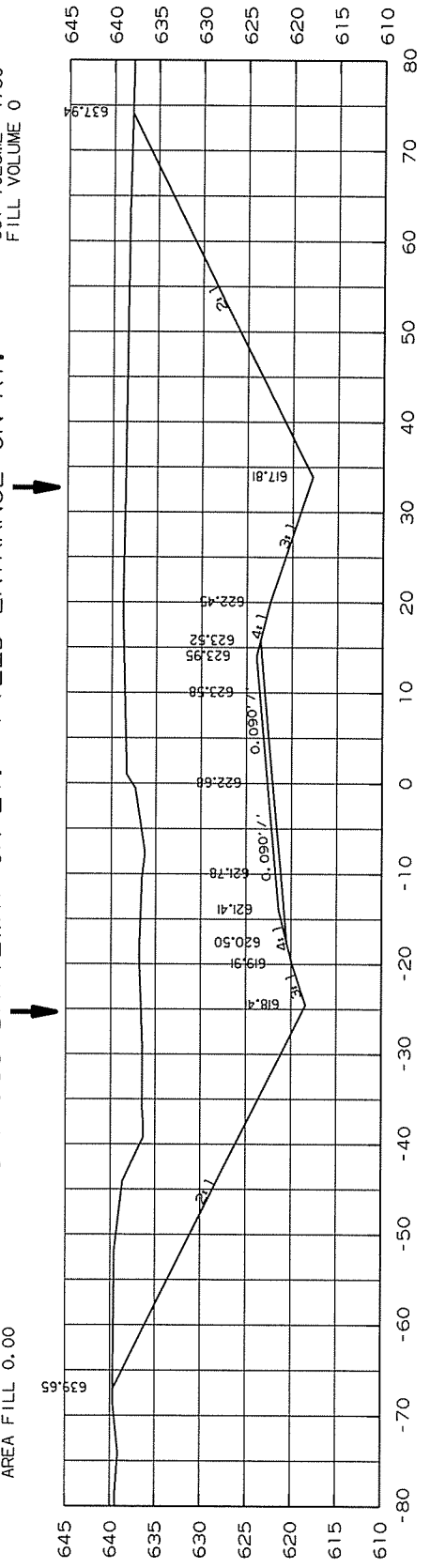
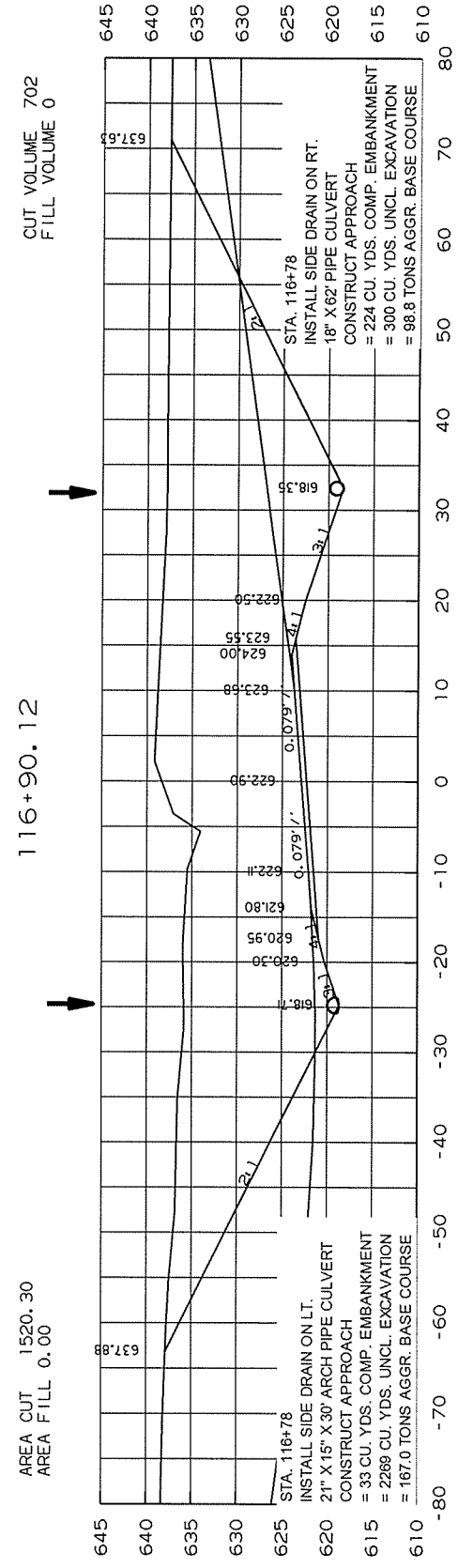
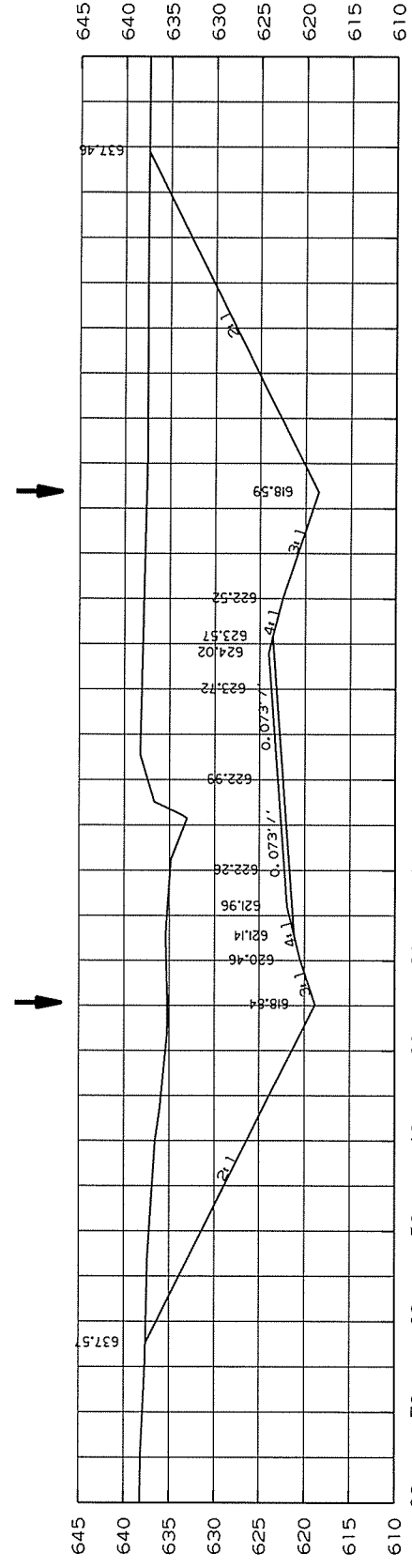
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
						JOB NO. BR4906	71	85

4 CROSS SECTIONS STA. 114+00.00 TO 115+50.00



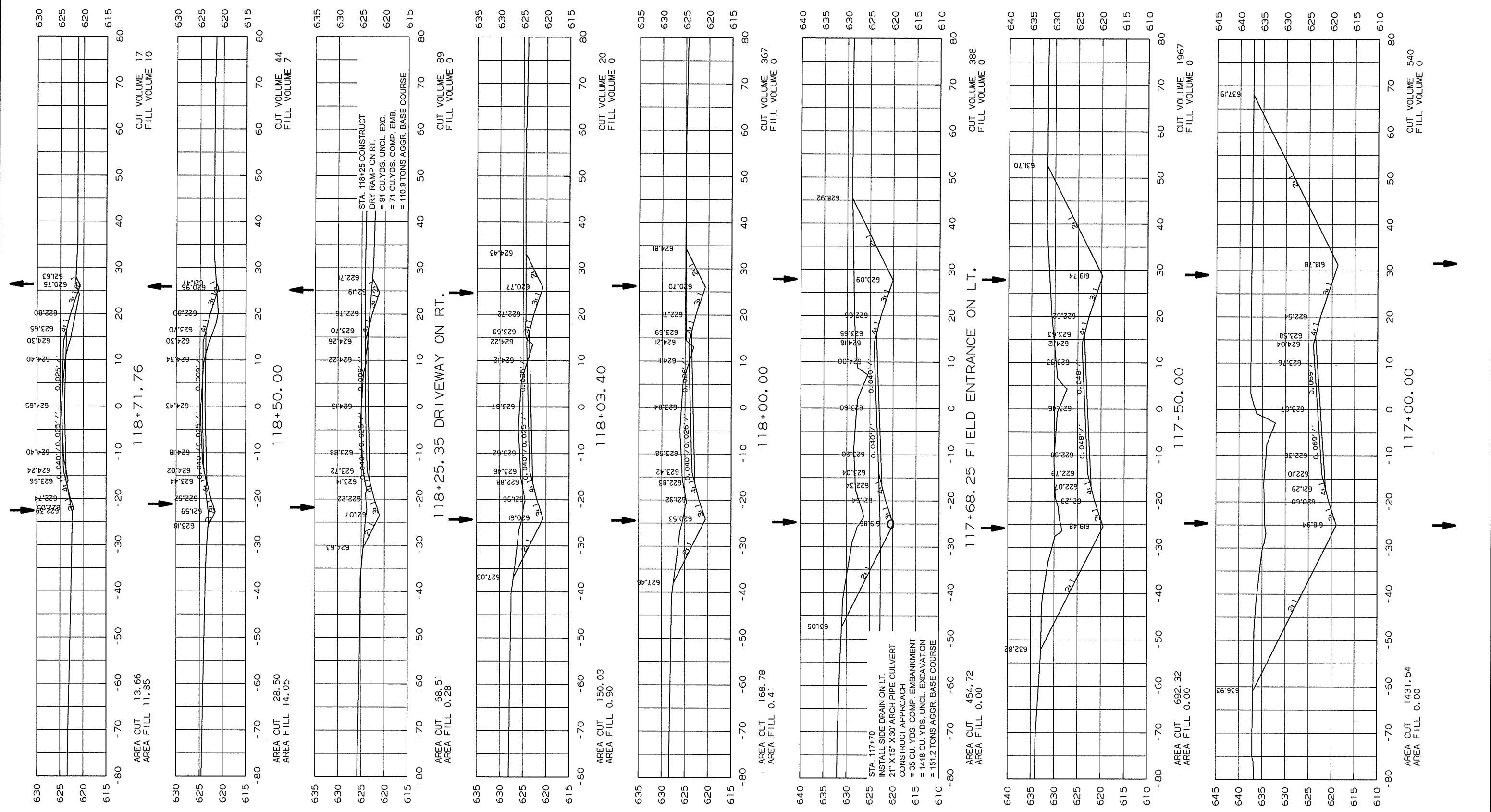
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
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				JOB NO.		BR4906	72	85

4 CROSS SECTIONS STA. 116+00.00 TO 116+90.12



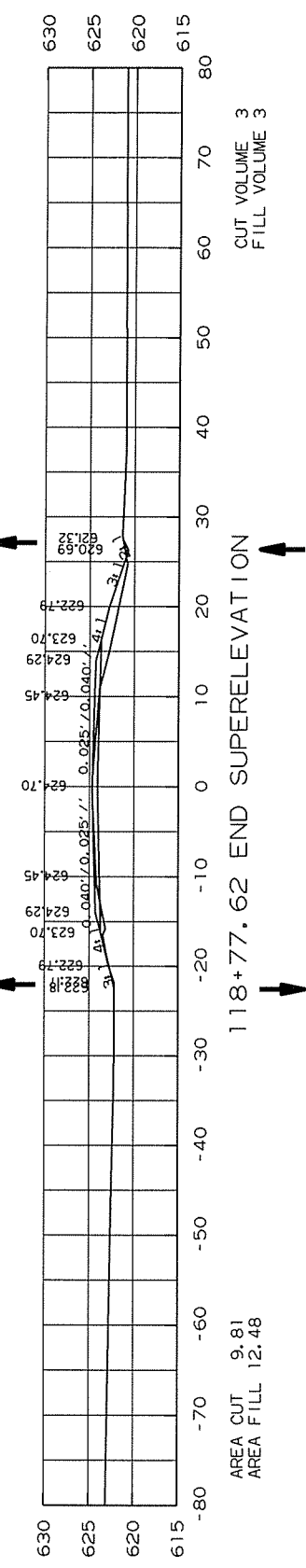
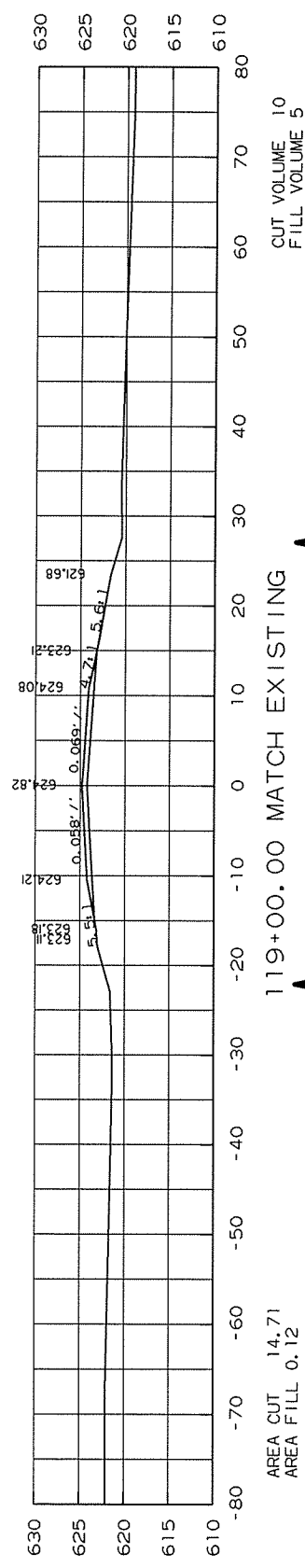
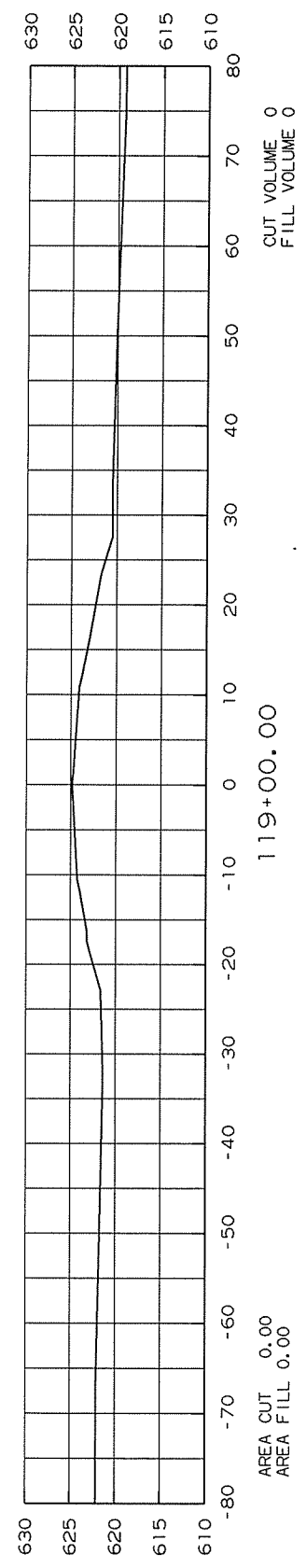
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.		73	85
				JOB NO.		BR4906		

4 CROSS SECTIONS STA. 117+00.00 TO 118+71.76



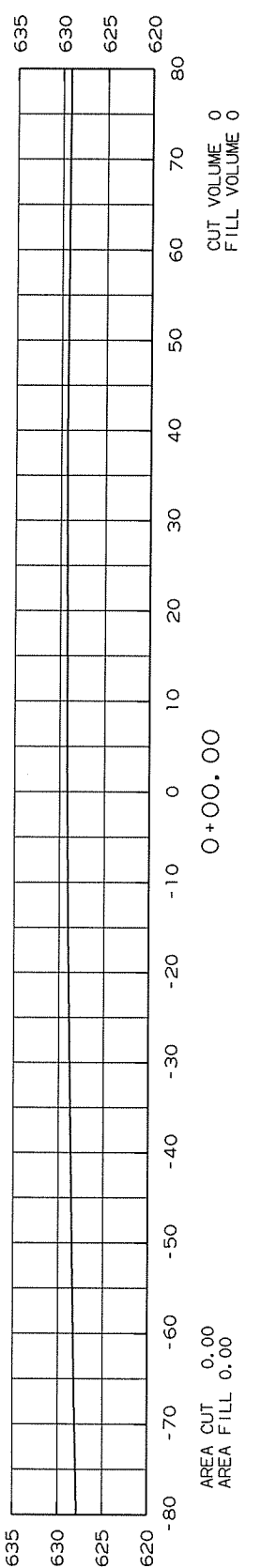
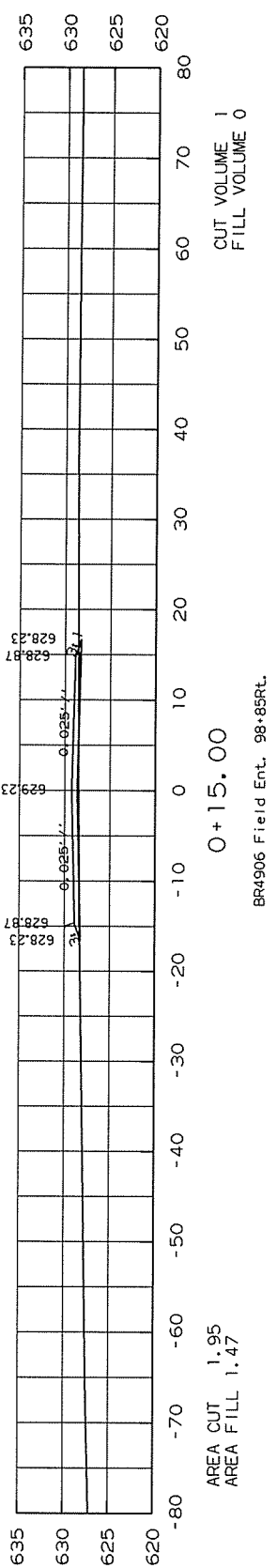
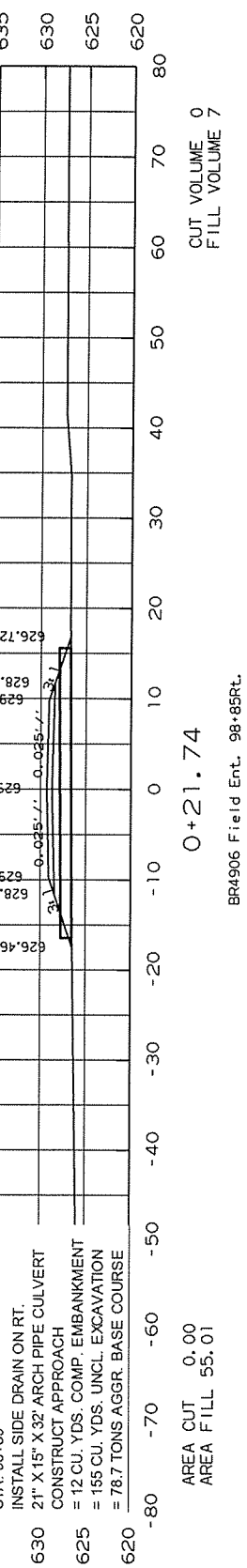
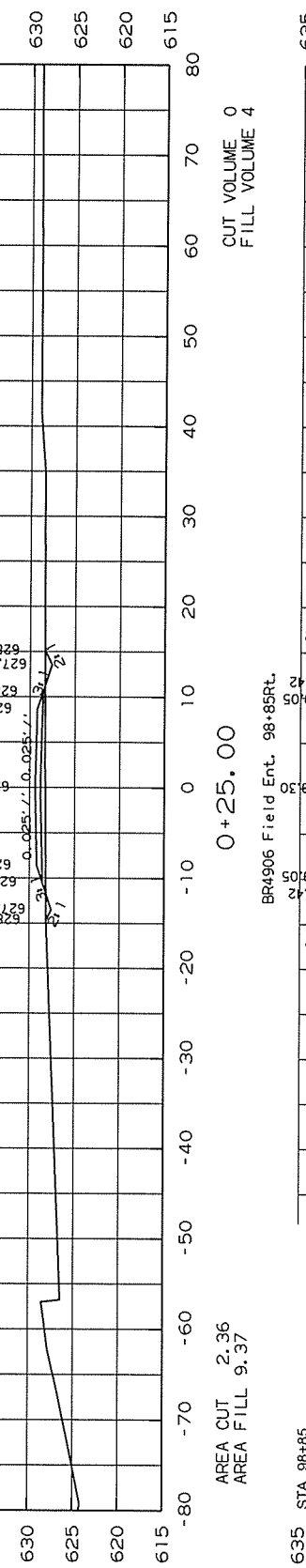
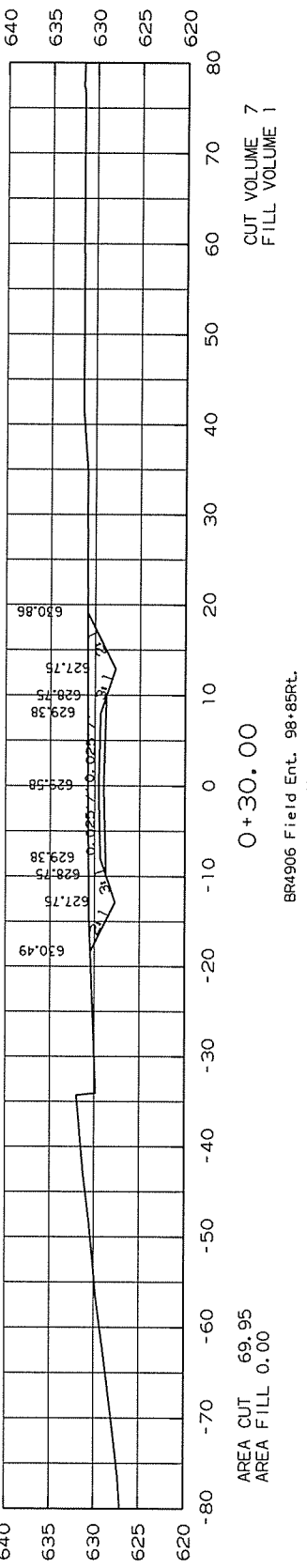
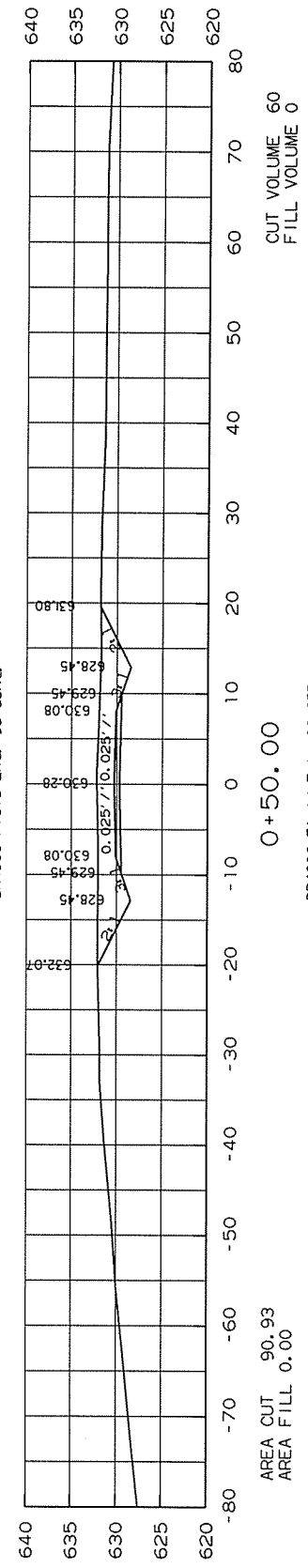
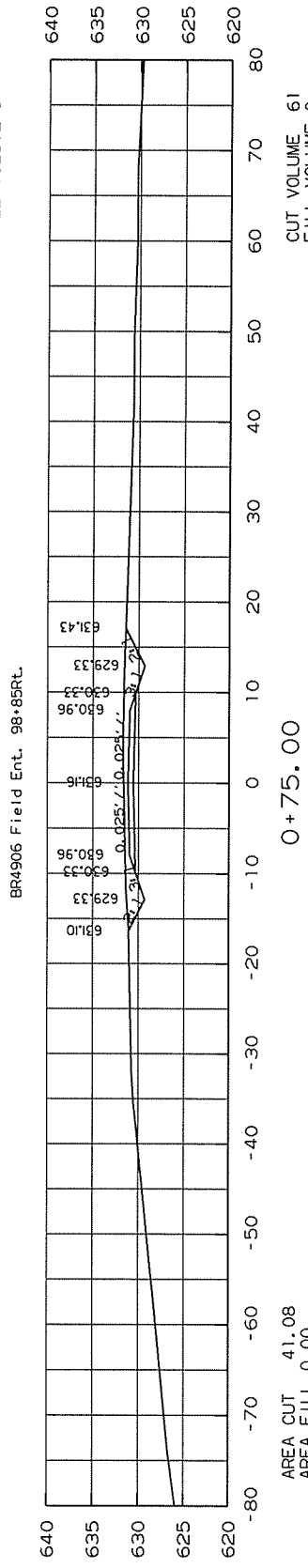
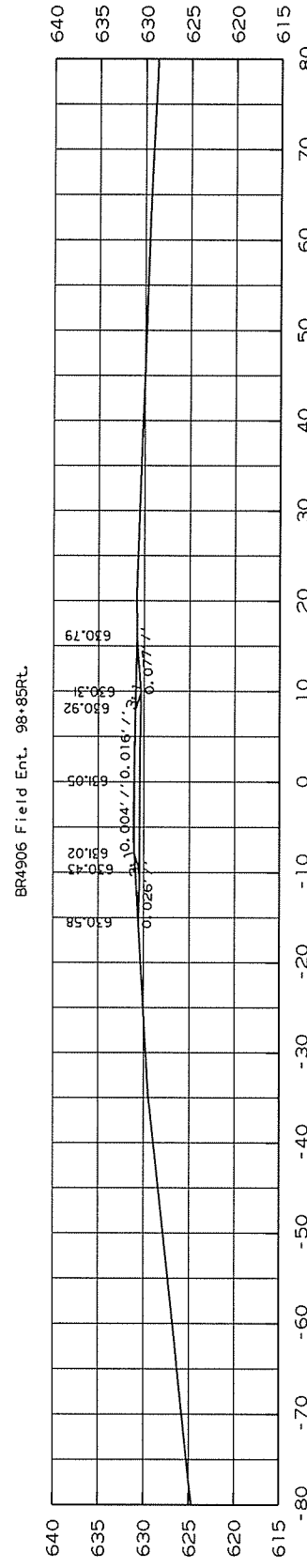
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		74	85

4 CROSS SECTIONS STA. 118+77.62 TO 119+00.00



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	75	85	

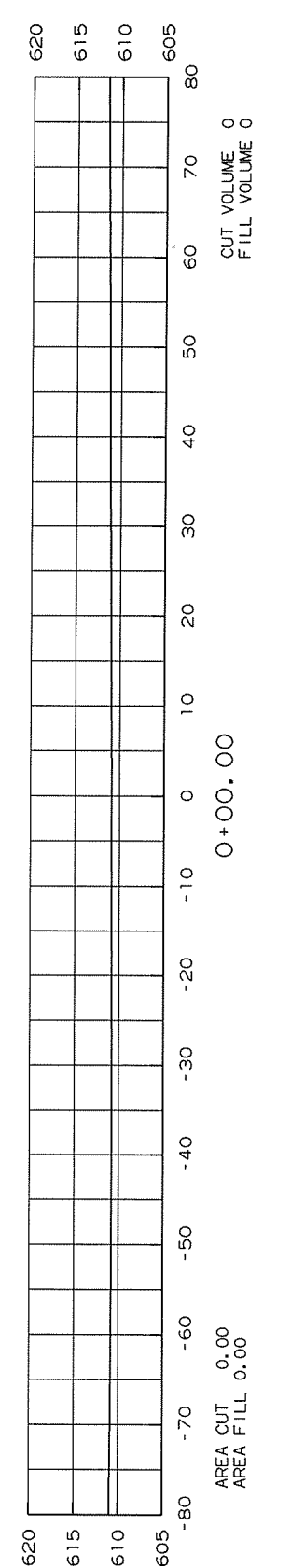
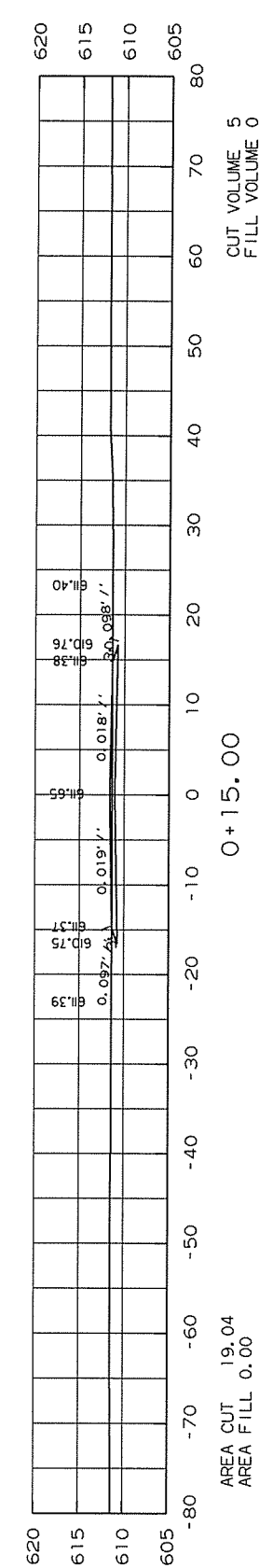
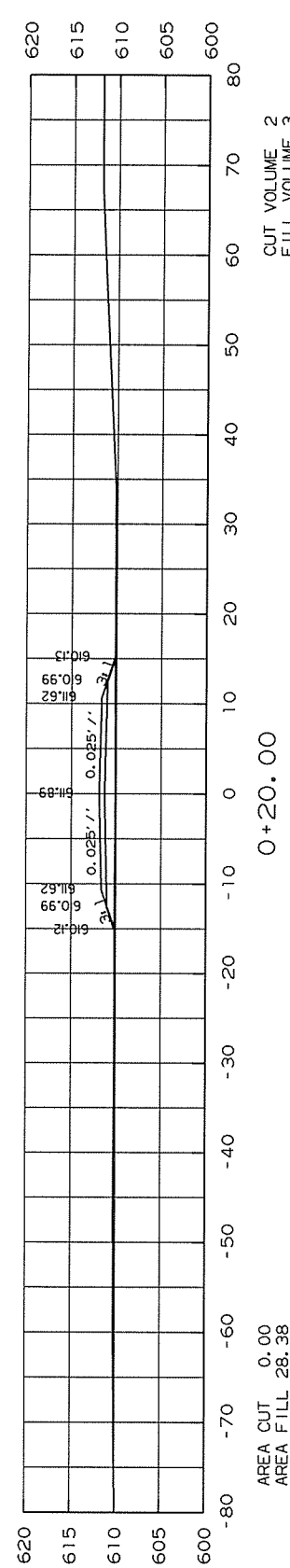
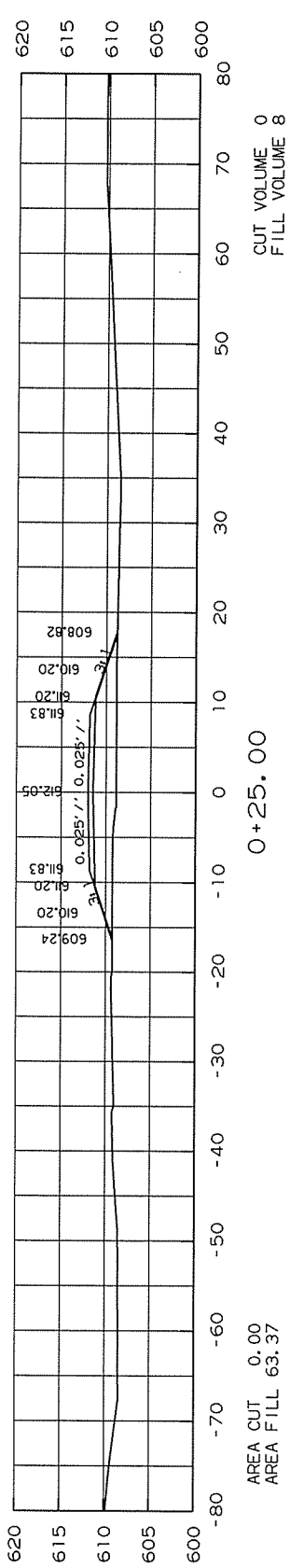
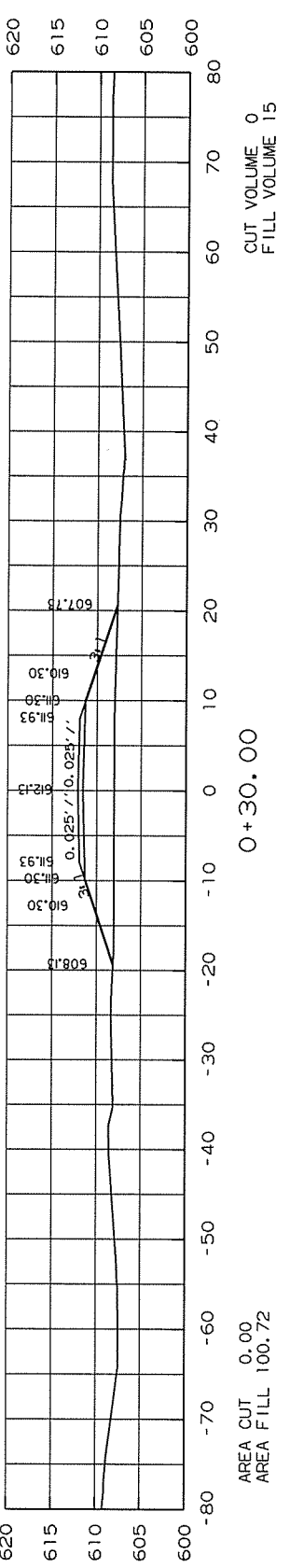
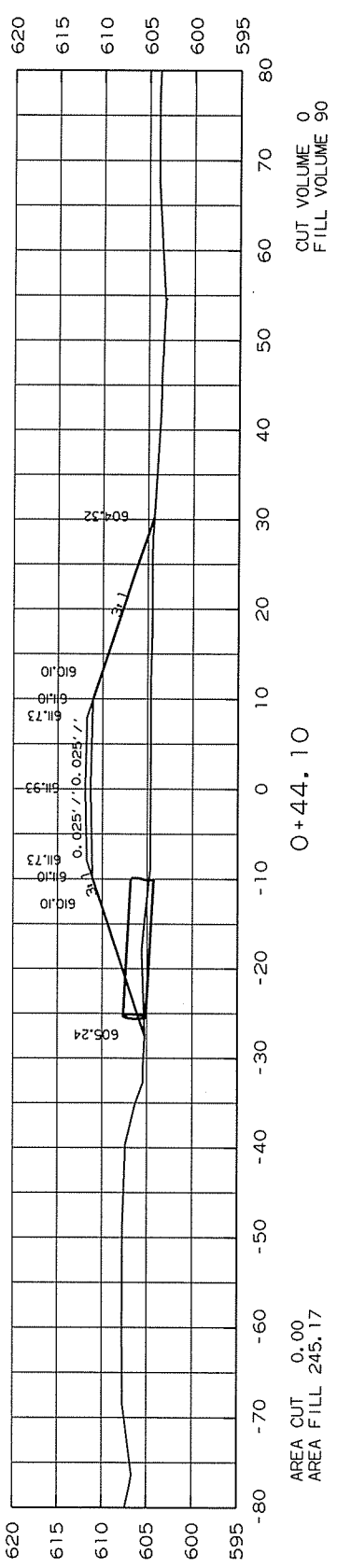
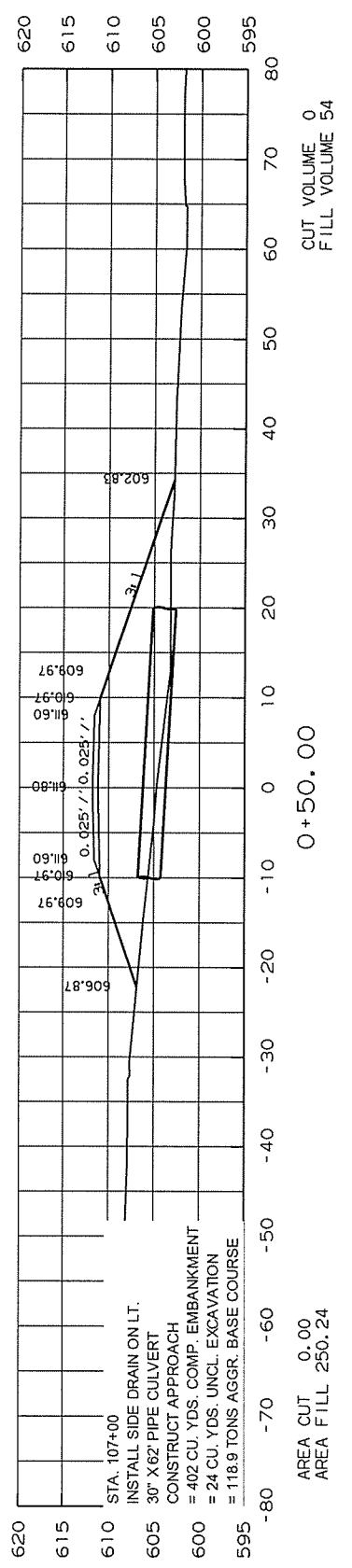
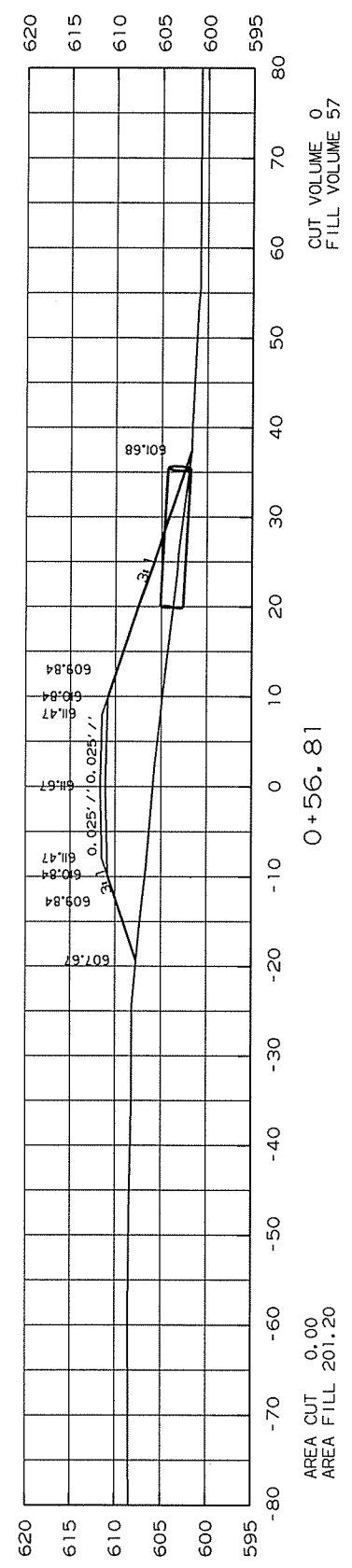
4 CROSS SECTIONS FIELD ENTRANCE 98+85 RT.



635 STA. 98+85
INSTALL SIDE DRAIN ON RT.
21" X 15" X 32' ARCH PIPE CULVERT
CONSTRUCT APPROACH
625 = 12 CU. YDS. COMP. EMBANKMENT
= 155 CU. YDS. UNCL. EXCAVATION
= 78.7 TONS AGGR. BASE COURSE

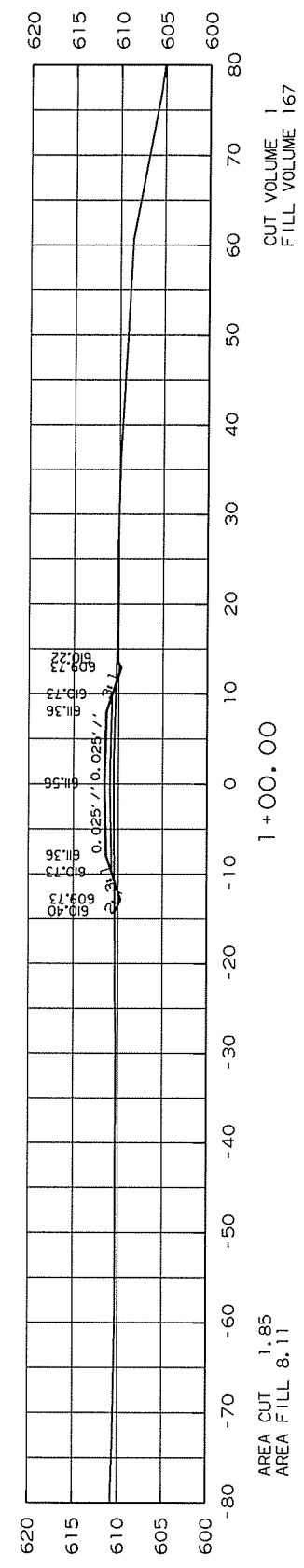
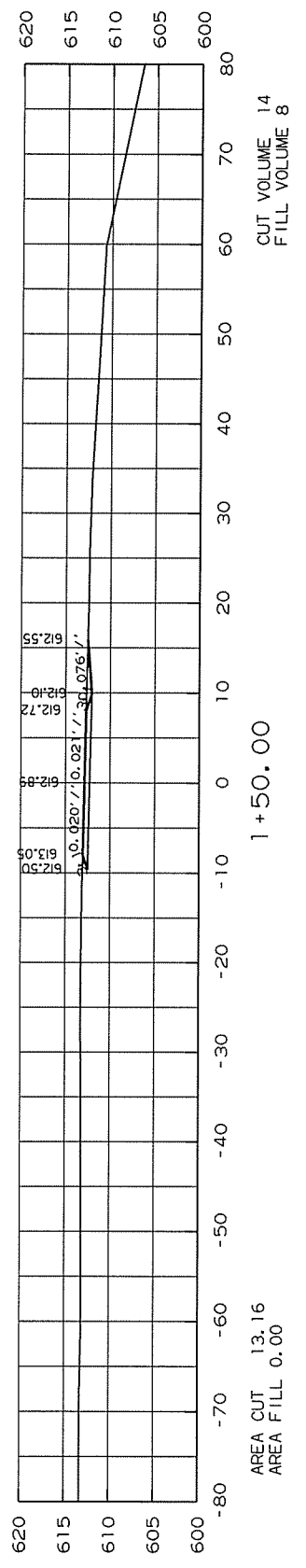
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906	77	85	

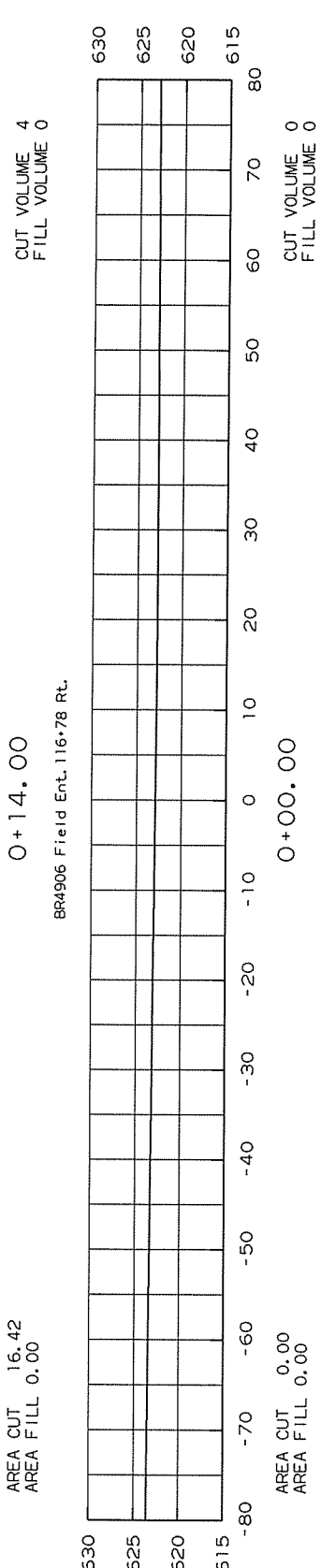
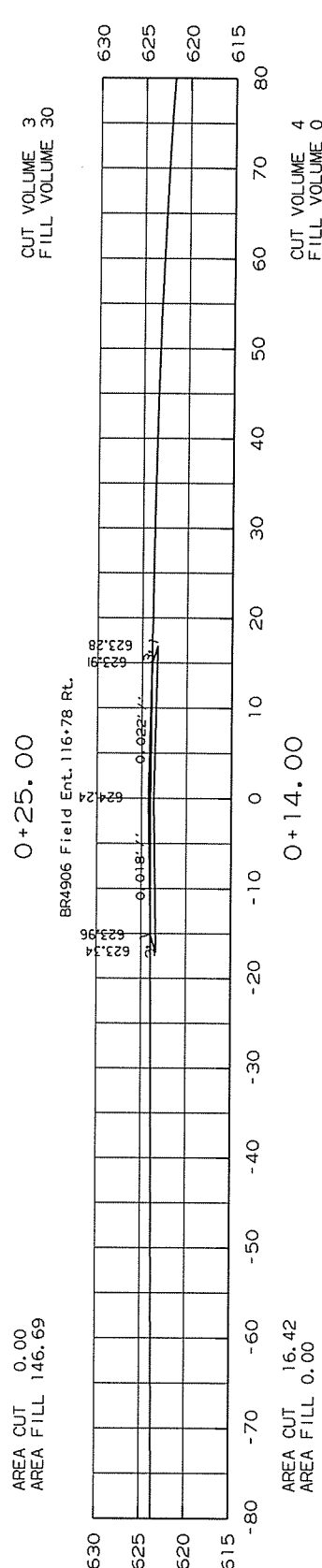
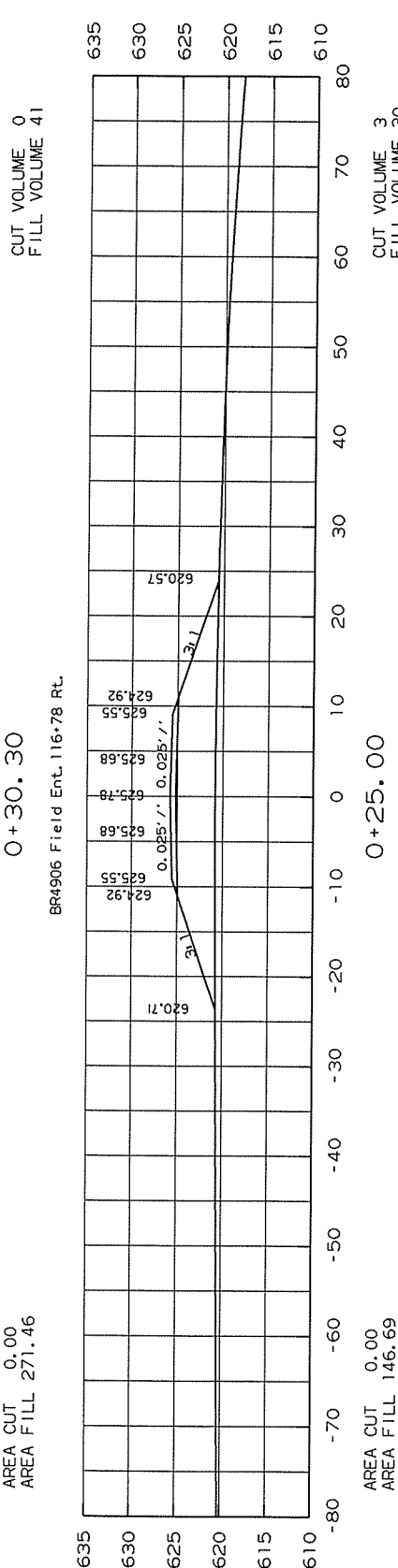
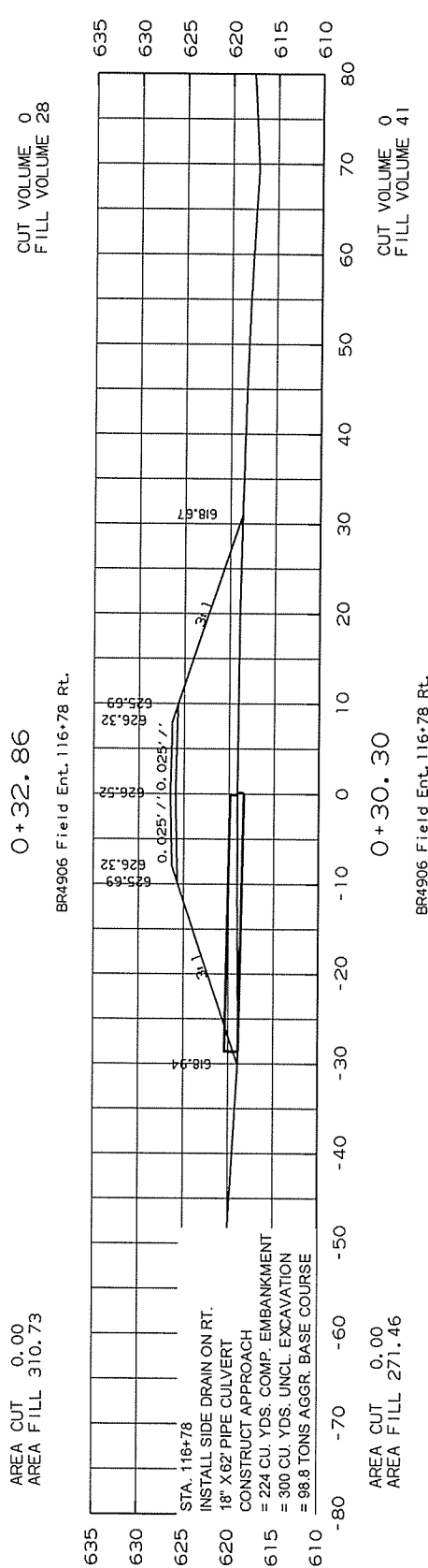
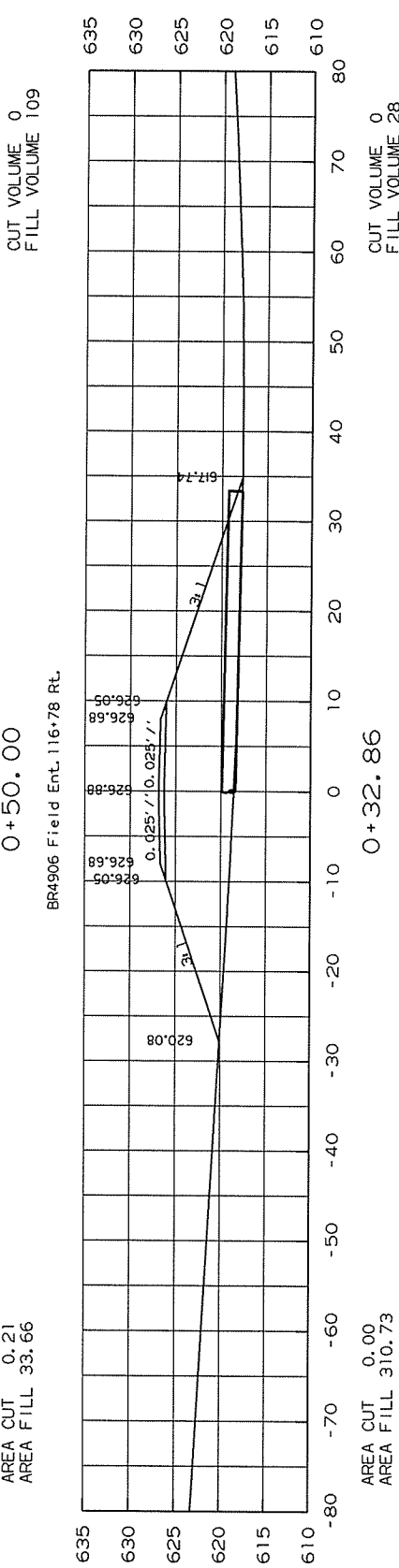
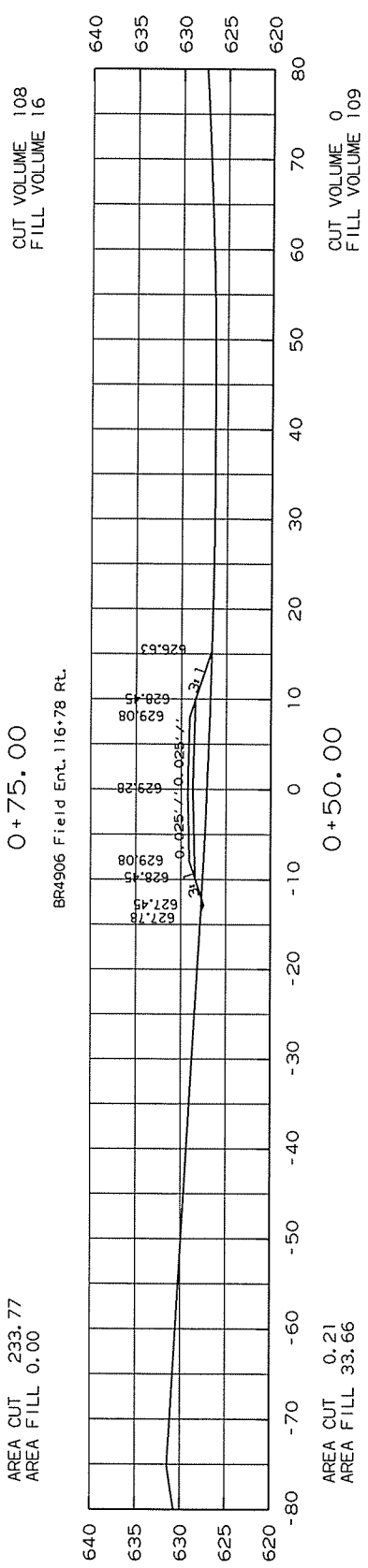
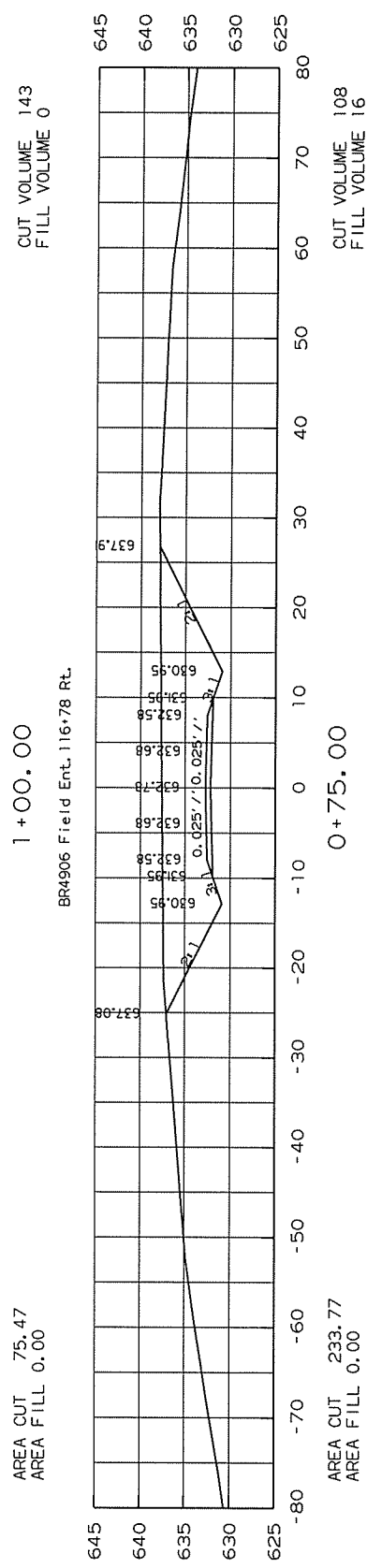
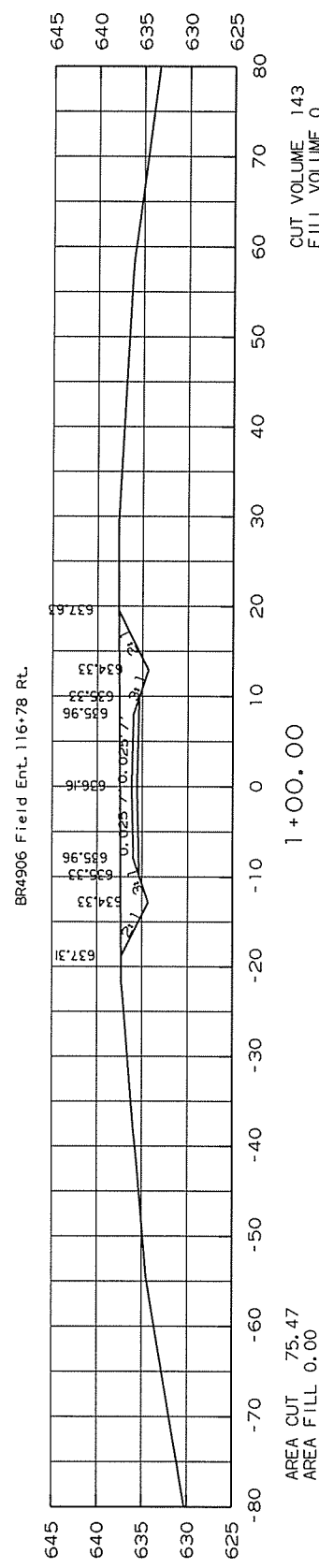
4 CROSS SECTIONS FIELD ENTRANCE 107+00 LT.



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		78	85

4 CROSS SECTIONS FIELD ENTRANCE 107+00 LT.



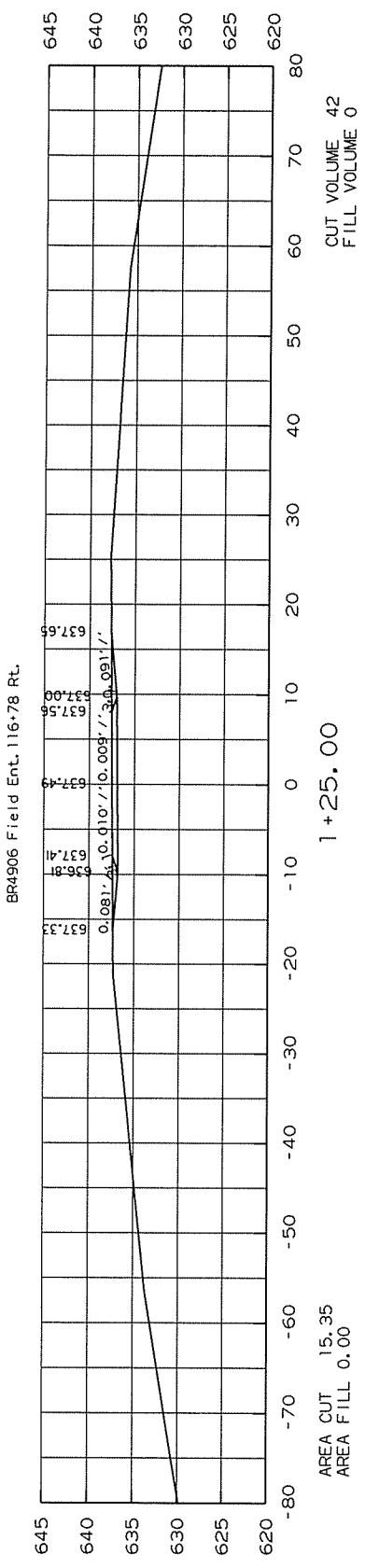


DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		81	85

4 CROSS SECTIONS FIELD ENTRANCE 116+78 RT.

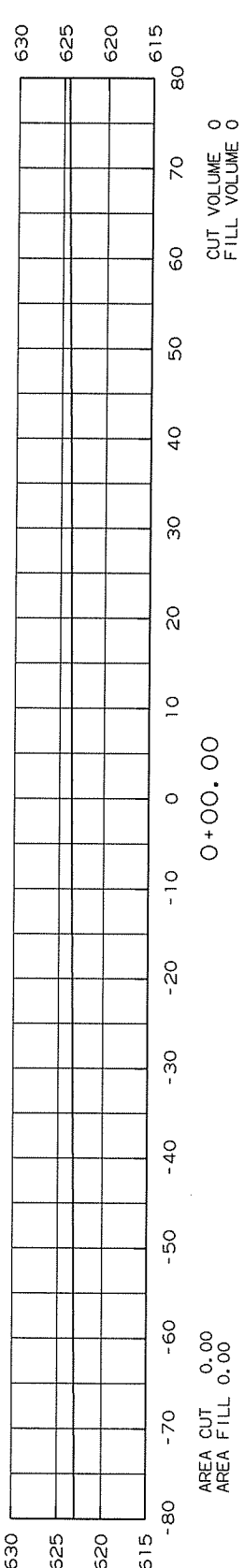
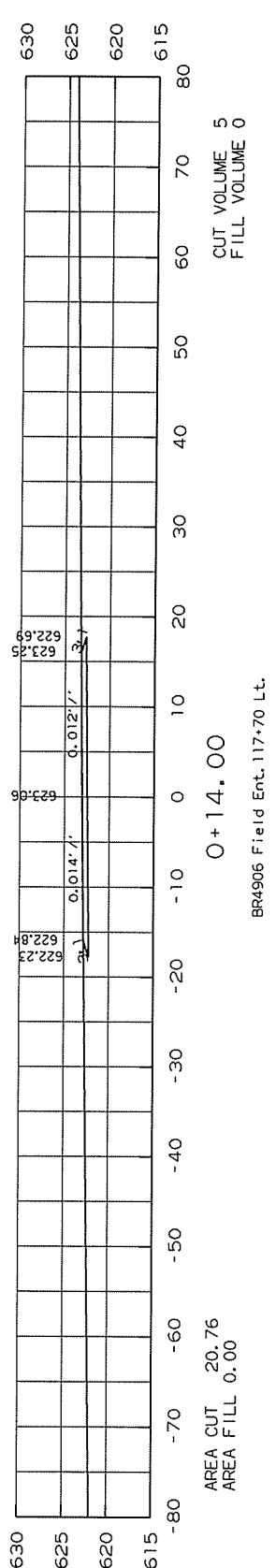
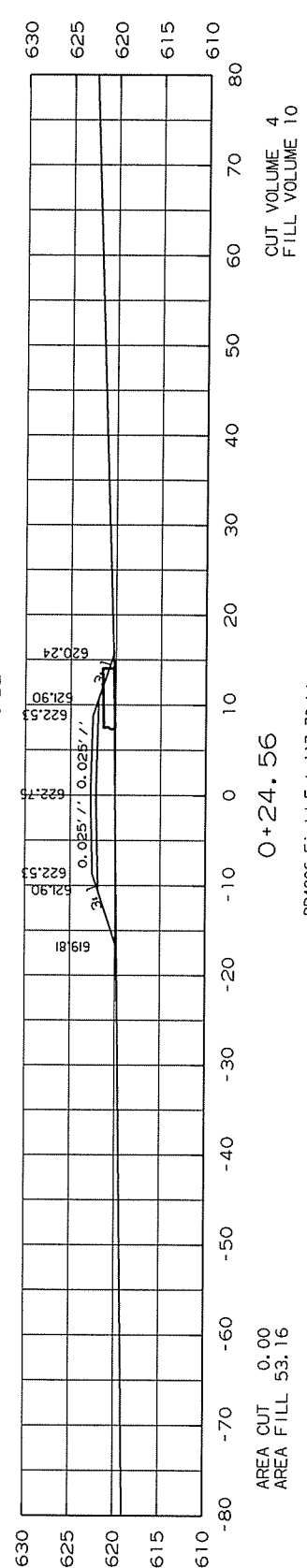
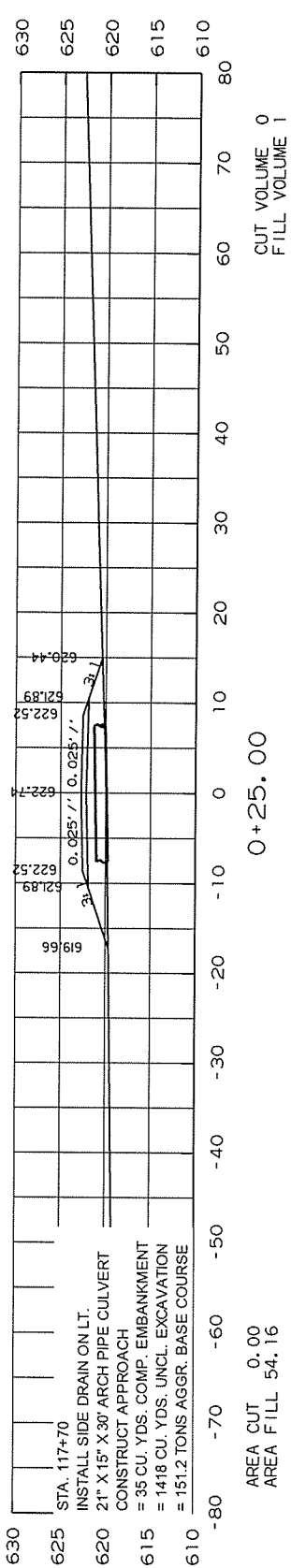
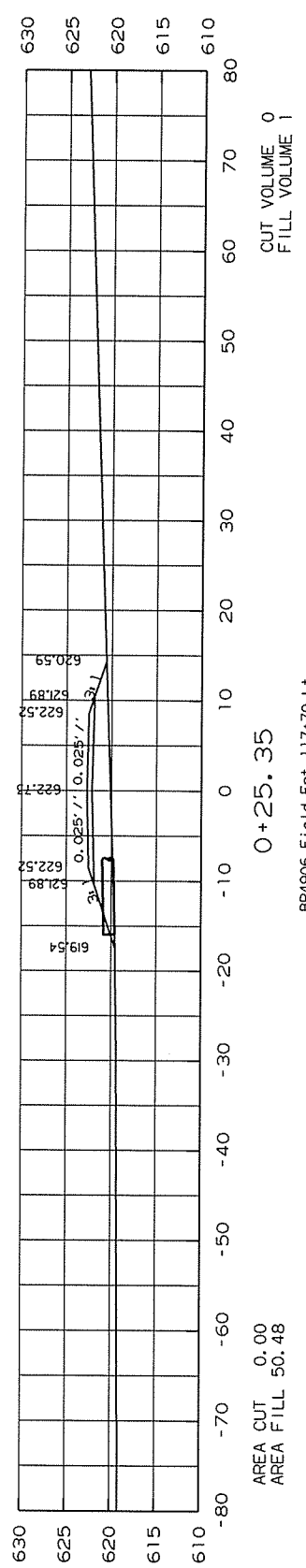
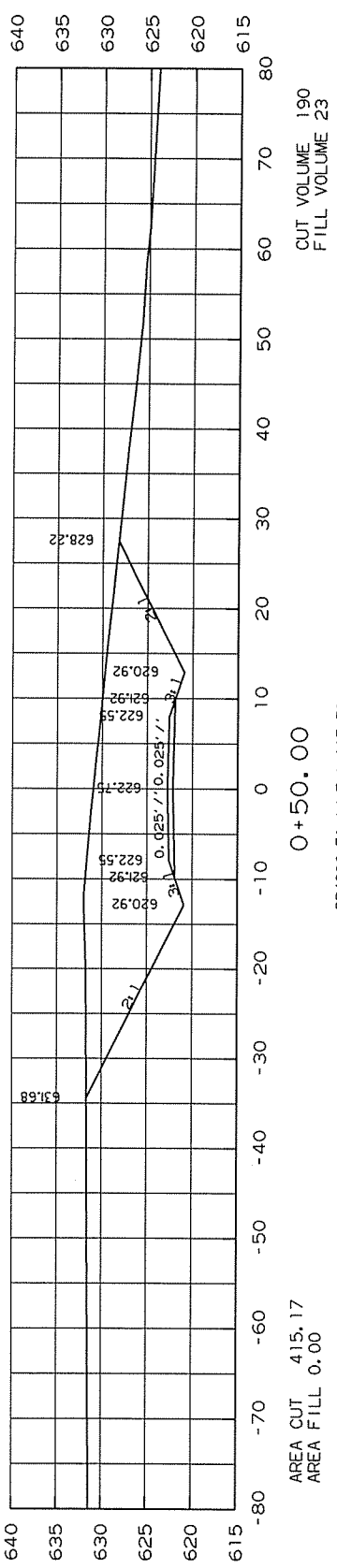
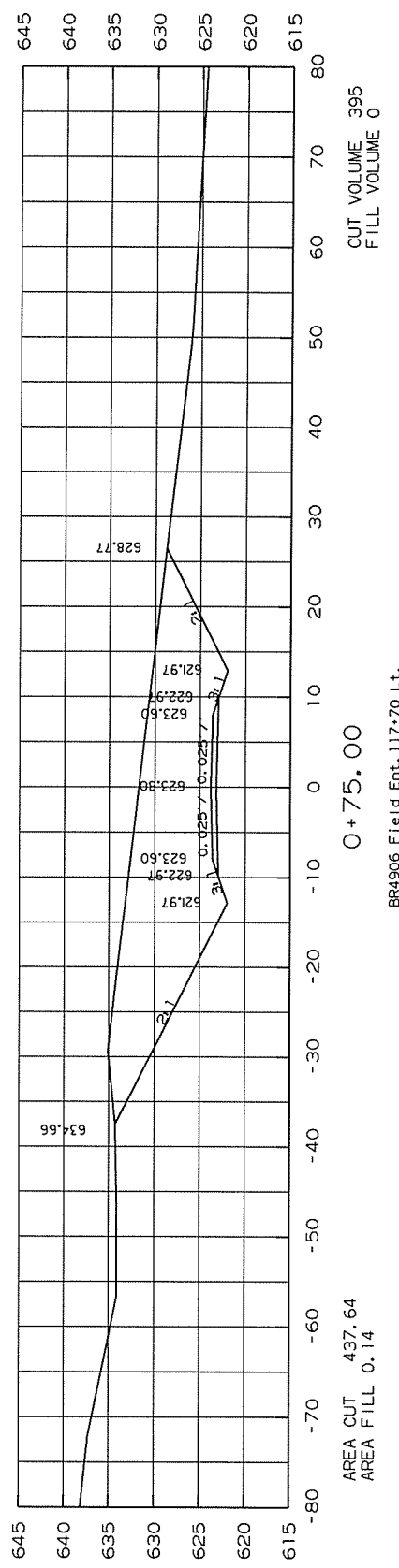
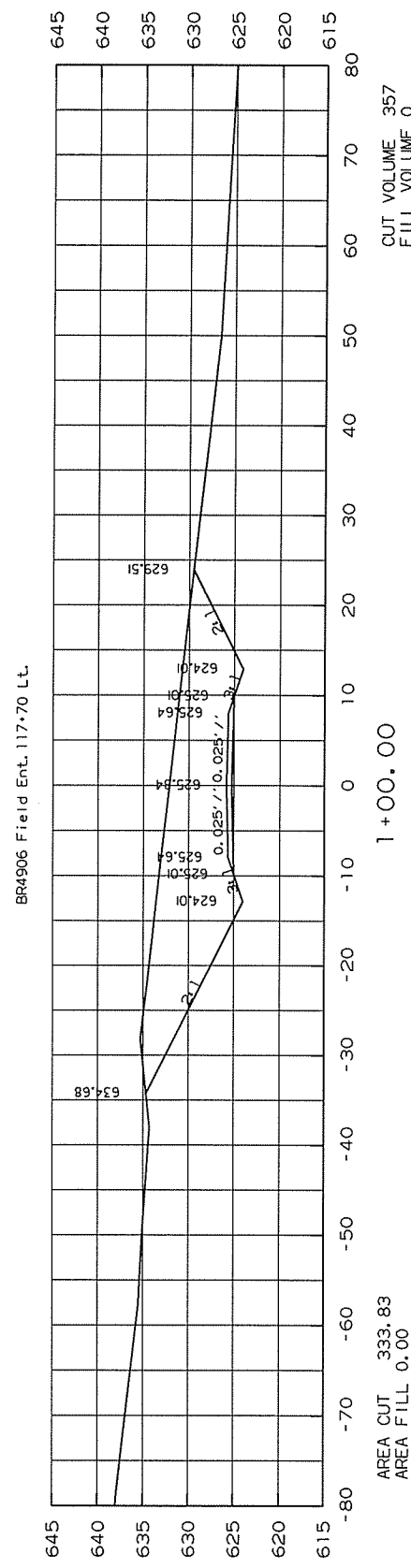
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.		BR4906	82	85

4 CROSS SECTIONS FIELD ENTRANCE 116+78 RT.

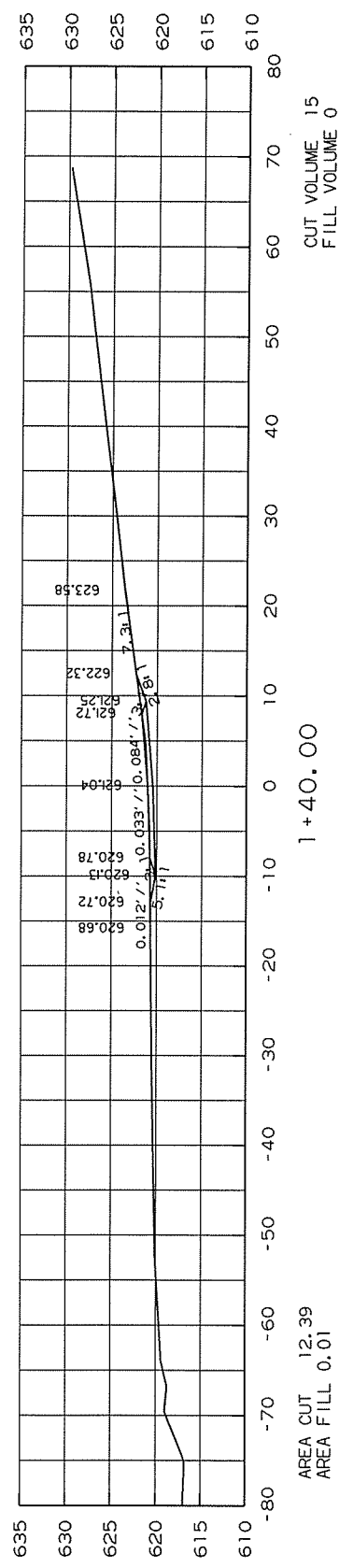


DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		83	85

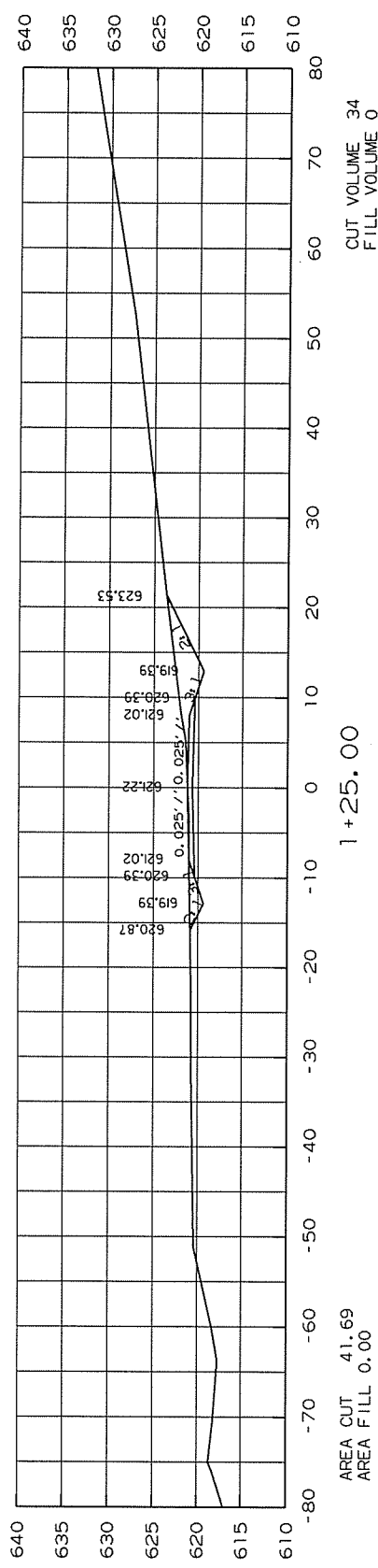
4 CROSS SECTIONS FIELD ENTRANCE 117+70 LT.



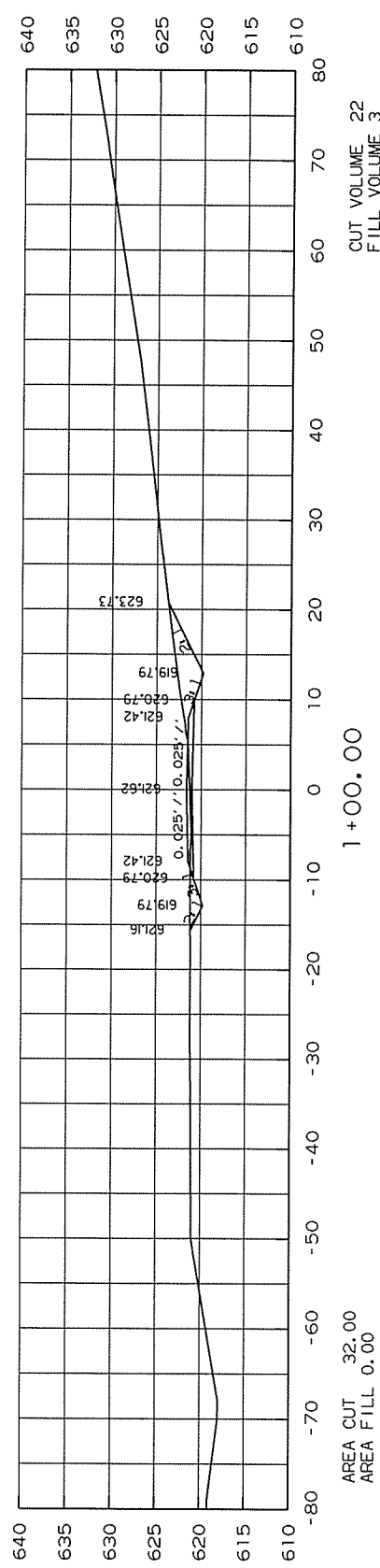
BR4906 Field Ent. 118+25 Rt.



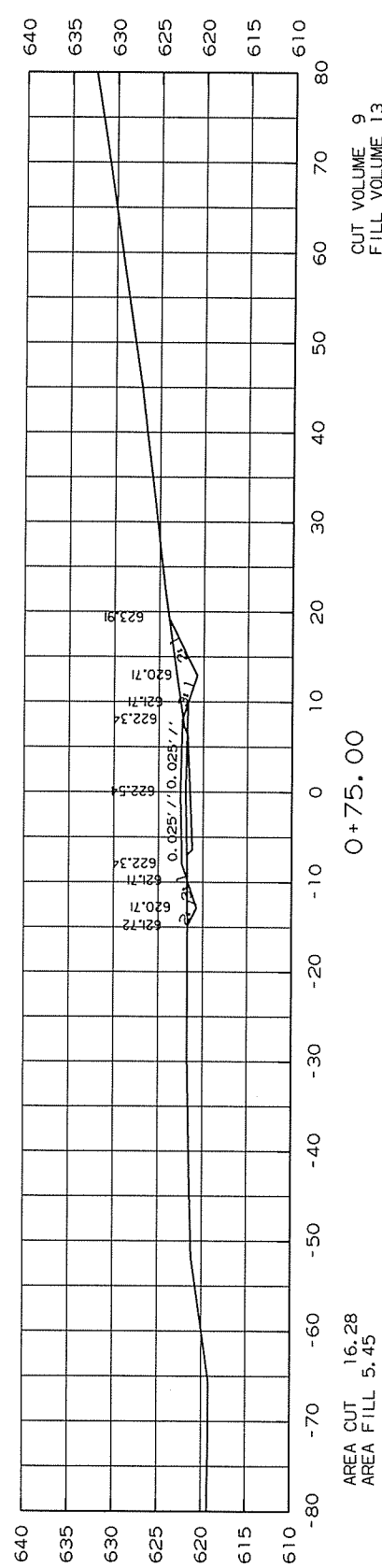
BR4906 Field Ent. 118+25 Rt.



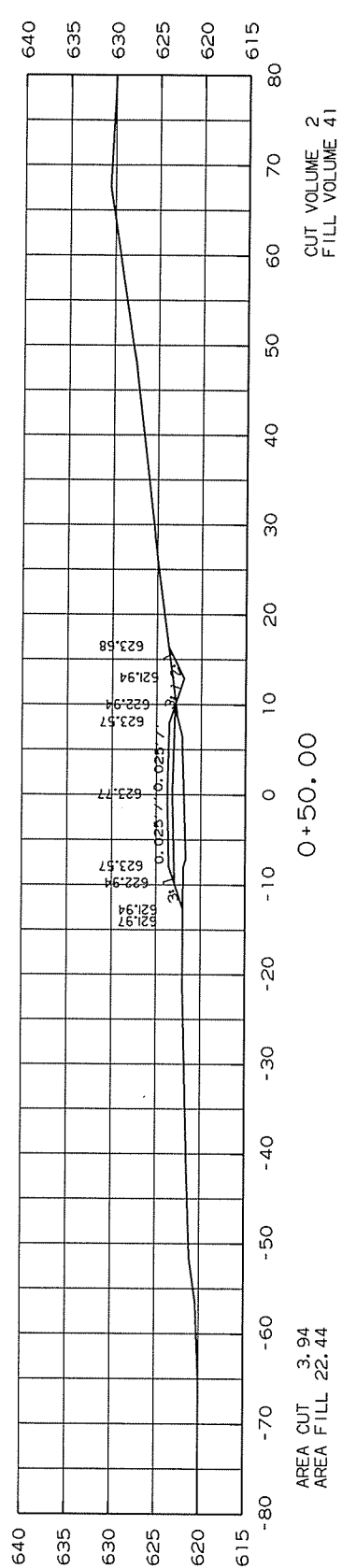
BR4906 Field Ent. 118+25 Rt.



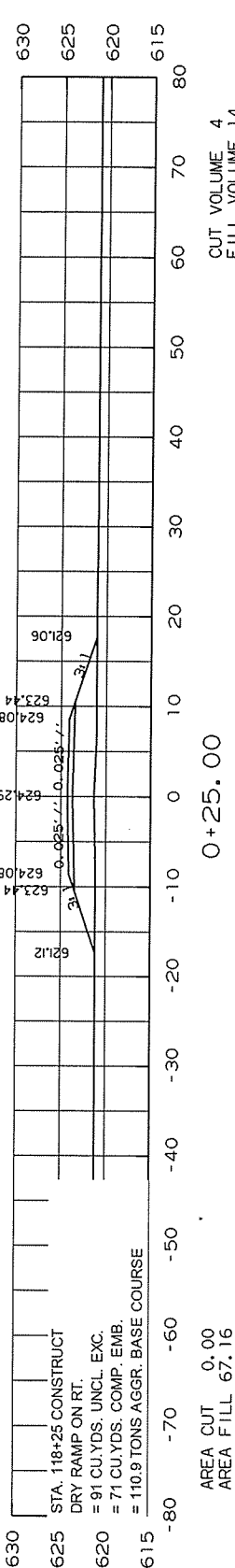
BR4906 Field Ent. 118+25 Rt.



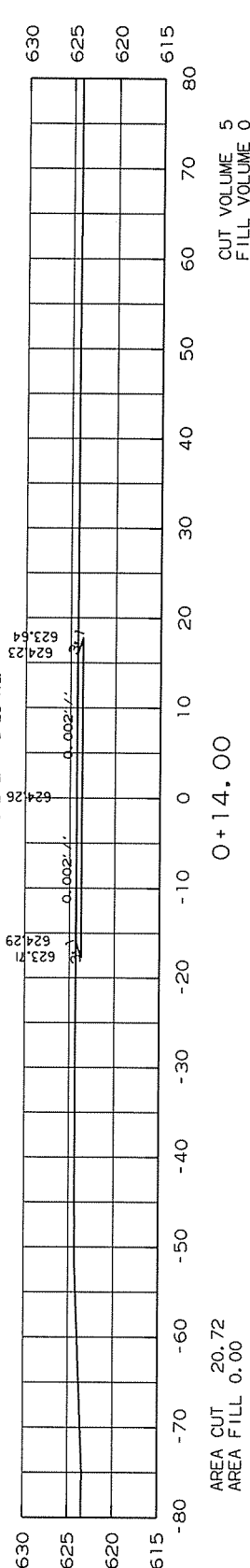
BR4906 Field Ent. 118+25 Rt.



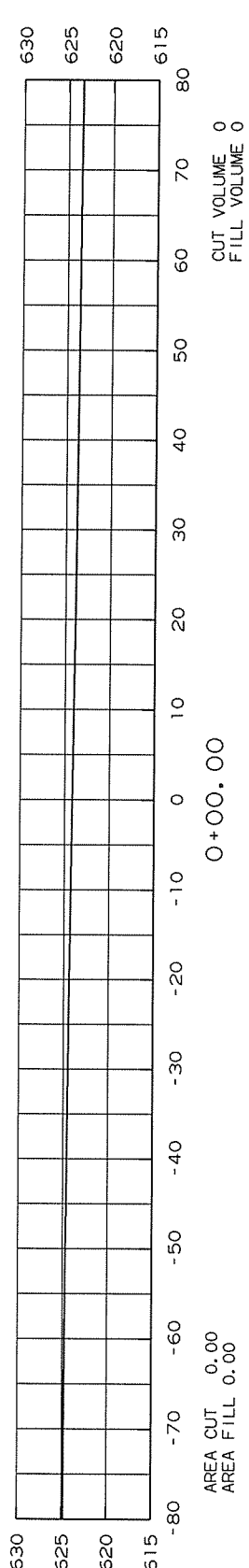
BR4906 Field Ent. 118+25 Rt.



BR4906 Field Ent. 118+25 Rt.



BR4906 Field Ent. 118+25 Rt.



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	FED. RD. DIST. NO.	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
				6	ARK.			
				JOB NO.	BR4906		85	85